

2311 WILSON BOULEVARD 5TH FLOOR ARLINGTON, VA 22201 PHONE 703.525.4000 FAX 703.525.2207 Timothy Dugan Admitted: VA, MD and DC Mobile: 301-922-4504 Office: (703) 526-5583 tdugan@beankinney.com

June 29, 2022 MEMORANDUM

Hand Delivered¹

TO:	Matt Arcieri	Greg Bokan
	Dorothy Baker	Christian Samples
CC:	Christian Engel	Josie Engel
	Maureen Wiechert	Saeed Dornajafi
	Mike Kitchen	Garrett Brown
	Eric Bogumil	Laurie Beth Donnachie
	Chris Lemon	Kevin Sitzman
	Niraja Chandrapu	
FROM:	Timothy Dugan \mathcal{TD}	
RE:	Special Use Permit Appl	ication
	Applicant: Edugenius U	S Real Estate Group, LLC
	Tillett Hall, 9522 Fairvie	w Avenue, City of Manassas, Virginia 20110

On behalf of the Applicant, Edugenius US Real Estate Group, LLC, a Virginia limited liability company, the Applicant, and with the permission of the Owners, we are submitting the following materials in support of the Special Use Permit Application.

¹ Delivered to the temporary address at: 9800 Godwin Drive, Manassas, VA 20110.



Row # Description # hard Copies to Manassas 1) \$500 Application Fee 2) 1 Fee schedule $\mathbf{2}$ 3) Completed Special Use Permit Application Form (Original and one copy) 4) Applicant's Notarized Conflict of Interest Affidavit (Original and one copy) $\mathbf{2}$ Owners' Notarized Conflict of Interest Affidavit (Original and one copy) 5) $\mathbf{2}$ $\mathbf{2}$ 6) Owners' Special Limited Power of Attorney Appointing Timothy Dugan, Esq. (Original and one copy) 7)Architectural renderings providing full-color, four-sided architecture and exterior finishes for any new From **ARB** files construction or alteration, including building elevations, signage, fencing and dumpster enclosures. Note: The Applicant was advised that the City Planning Staff will provide the architectural renderings from the plans submitted in the course of the applications with the Architectural Review Board's considerations of the concept/massing and the materials applications for certificates of appropriateness. Please advise if we may provide any additional information. As for site features, *i.e.*, fencing, dumpster and signs, the Applicant's application for an ARB certificate of 8) appropriateness has not been filed. The SUP Application plans: (1) include a notation about fencing, Sheet C300; (2) refer to the ARB COA for signage shown on the rendered elevations, Sheet C000; and (3) include a notation about the dumpster location to be determined. Sheet C300. Traffic impact analysis, economic impact analysis, and environmental studies. 9) • A traffic impact analysis will be submitted under a separate cover, after this initial application submittal, in early July 2022. The traffic counts have been submitted already. There is no economic impact analysis. Environmental studies associated with tree preservation and stormwater management are included among the Application materials within the GDP plans. Generalized Development Plan (GDP) showing all existing and proposed improvements to demonstrate 10) 15compliance with zoning and DCSM and mitigate negative impacts of the proposed use. C000 Cover Sheet C100 Existing Conditions Plan C200 Tree Preservation Plan C300 Layout C400 Conceptual Landscape Plan C500 Utility Layout 11"x17" reductions of the GDP 1 11)



Page 3 of 3

Row #	Description	# hard
		Copies to
		Manassas
12)	A written narrative Statement of Justification describing: The proposed use as it relates to the Comprehensive	15
	Plan, Operational conditions and special conditions proposed to minimize the impact of the use on surrounding	
	properties. Attachments are also included.	
13)	Thumb drive containing electronic files of all documents included in the submission	1
14)	Requests for waivers or modifications from the City's Design and Construction Standards Manual (DCSM) must	
	be filed, including filing fee, with the Development Services Division concurrent with the submission of a special	
	use permit application and approved prior to the case being scheduled for public hearing. = None noted.	
15)	April 19, 2022 Architectural Review Board Concept/Massing Certificate of Appropriateness and Supporting	15
	Materials	
16)	May 19, 2022 Architectural Review Board Materials Certificate of Appropriateness and Supporting Materials	15

Thank you for your consideration. Please call with your comments, questions and instructions. Thank you.



CITY OF

Ν

SUP #: 2023-0001

ANASSAS, VIRGINIA Community Development · 9800 Godwin Drive, Manassas, VA 20110 · 703-257-8223

DATE: <u>6/30/2022</u> (Completed by City Staff)

SPECIAL USE PERMIT APPLICATION

Site Address: 95	22 Fairview Ave.			Manassas, V	'A 20110
Tax Map No(s):	Street 100-01-00-202				
Site Acreage:			Zoning District: _	R-1	
Comprehensive Pla	n Designation: _				
Proposed Use:Ple	ase see attached.				
□ This is an amend	ment to existing	g SUP #:			
<u>Al</u>	PPLICANT		<u>OWNI</u> (Leave Blank if Sam	E R e as Applicant)	
Please see attached.			Please see attached.		
Name (Please Print)			Name		
Signature			Company		
Address			Address		
City	State	Zip Code	City	State	Zip Code
Phone #	Fax #		Phone #	Fax #	
E-mail Address			E-mail Address		

ATTACHMENT TO SPECIAL USE PERMIT

Description of Proposed Work

The Applicant wishes to develop a child care center in Phase 1 and an elementary school in Phase 2. Tillet Hall would be used for the school's administrative offices.

OWNER:

Kan Phillips Musta

Karen E. Phillips, Trustee of the Kenneth F. Parsons Revocable Trust under agreement dated August 9, 2011, as amended on February 24, 2015 and on January 1, 2021

Tulin J

Karen E. Phillips, Trustee of the Kathleen E. Parsons Revocable Trust under agreement dated August 9, 2011, as amended on February 24, 2015

R. Scott Phillips, Trustee of the Kathleen E. Parsons Revocable Trust under agreement dated August 9, 2011, as amended on February 24, 2015

Please contact the agent for the Owners: Christopher M. Malone, Esq. Thompson McMullan P.C. 100 Shockoe Slip, Third Floor Richmond, Virginia 23219 email: <u>cmalone@t-mlaw.com</u> Office: (804) 698-6231 Fax: (804) 780-1813

ATTACHMENT TO SPECIAL USE PERMIT

Page 2 of 2

APPLICANT:

. . .

EDUGENIUS US REAL ESTATE GROUP, LLC, a Virginia limited liability company

By: Jøsefine Engel-Banks, Manager

Please contact the agent for the Applicant: Timothy Dugan, Esq. Bean, Kinney & Korman, P.C. 2311 Wilson Boulevard, 5th Floor Arlington, VA 22201 email: tdugan@beankinney.com Office: 703-526-5583 Fax: 703-525-2207

County of Arlington Commonwealth of Virginia This foregoing instrument was acknowledged before me this <u>24</u> day of <u>000</u>, 20<u>2</u> by <u>FONCUYO</u> <u>AUCOSTOS</u> Notary Public Commission Exp.<u>07/31/282</u>-Registration # <u>799/68/67</u>





MANASSAS, VIRGINIA

Community Development · 9800 Godwin Drive, Manassas, VA 20110 · 703-257-8223

CONFLICT OF INTEREST AFFIDAVIT

STATE OF		_
of		_, to wit:
Tax Map No	100-01-00-202	
Property Address:	9522 Fairview Avenue,	City of Manassas, Virginia 20110

I hereby certify that no member of the City Council or the Planning Commission of the City of Manassas, Virginia, has any interest in the above property, either individually, by ownership of stock in a corporation owning such land, or partnership, or as a holder of ten percent (10%) or more of the outstanding shares of stock in or as a director or officer of any corporation, owning such land, directly or indirectly by such member or members of his immediate household.

Josefine Engel-Banks Manager	APPLICANT, EDUGENIUS US REAL ESTATE GROU PLLC, a
Name of Representative	Company Name
434-466-2212	2319 S. Joyce Street Arlington, Virginia 22202
Telephone	Address
	Sterature of Representative
STATE OF VIRGINICI	
COUNTY OF ACTINGTION	
1. The Uality AUGUSTUR	, a Notary Public in and for the State and County aforesaid, do
the foregoing, this day personally acknowledged the same.	appeared before me in my State and County aforesaid and
Given under my hand this <u>74</u>	day of <u>Une</u> , 20 <u>72</u> .
	Januilli Augestos

My Commission expires: 07/3/2026



ID: 7496367

Revised December 2021



OWNERS' CONFLICT OF INTEREST AFFIDAVIT

MANASSAS, VIRGINIA

Community Development · 9800 Godwin Drive, Manassas, VA 20110 · 703-257-8223

CONFLICT OF INTEREST AFFIDAVIT

STATE OF _____

_____ of _____, to wit:

Tax Map No. 100-01-00-202

Property Address: ____9522 Fairview Avenue, City of Manassas, Virginia 20110

I hereby certify that no member of the City Council or the Planning Commission of the City of Manassas, Virginia, has any interest in the above property, either individually, by ownership of stock in a corporation owning such land, or partnership, or as a holder of ten percent (10%) or more of the outstanding shares of stock in or as a director or officer of any corporation, owning such land, directly or indirectly by such member or members of his immediate household.

Please see attached.	Trustees of the Kenneth F. Parsons Revocable Trust and the Kathleen F. Parsons Revocable Trust. Please see attached
Name of Representative	Company Name
Telephone	Address
	Signature of Representative
STATE OF	
COUNTY OF	
I,	_, a Notary Public in and for the State and County aforesaid, do whose name is signed to the before me in my State and County aforesaid and acknowledged
Given under my hand this	lay of, 20
	Notary Public
My Commission expires:	ID:

Revised December 2021

ATTACHMENT TO OWNERS' CONFLICT OF INTEREST AFFIDAVIT SPECIAL USE PERMIT OWNERS SIGNATURES Tax Map Number: 100-01-00-202

OWNER:

Karn Mullips, Justa

Karen E. Phillips, Trustee of the Kenneth F. Parsons Revocable Trust under agreement dated August 9, 2011, as amended on February 24, 2015 and on January 1, 2021

STATE OF VIRGINIA CITY COUNTY OF RICHMOND

> I, <u>Patricia Femiani</u>, a Notary Public in and for the State and County aforesaid, do hereby certify that <u>KAREN E. PHILLIPS</u> whose name is signed to the foregoing, this day personally appeared before me in my State and County aforesaid and acknowledged the same.

Given under my hand this 15 day of Jone, 20 22.

atricea tunay

Notary Public

My Commission expires: $\frac{4}{30} \frac{1}{2003}$ ID: $\frac{274950}{2003}$



Willip Musta

Karen E. Phillips, Trustee of the Kathleen E. Parsons Revocable Trust under agreement dated August 9, 2011, as amended on February 24, 2015

CRY COUNTY OF __ RICHMOND

I, <u>Patricia Femiani</u>, a Notary Public in and for the State and County aforesaid, do hereby certify that <u>KAREN 5 PHILUPS</u> whose name is signed to the foregoing, this day personally appeared before me in my State and County aforesaid and acknowledged the same.

Given under my hand this 1546 day of \overline{JUNC} , 20 $\lambda\lambda$

elicatemane

Notary Public

30/2023 ID: 274952 My Commission expires: 4



ATTACHMENT TO OWNERS' CONFLICT OF INTEREST AFFIDAVIT SPECIAL USE PERMIT OWNERS SIGNATURES Tax Map Number: 100-01-00-202 Page 3 of 3

R. Scott Phillips, Trustee of the Kathleen E. Parsons Revocable Trust under agreement dated August 9, 2011, as amended on February 24, 2015

STATE OF VIRGINIA

I, <u>Patricia Femiani</u>, a Notary Public in and for the State and County aforesaid, do hereby certify that <u>R.SCOTT PAILUAS</u> whose name is signed to the foregoing, this day personally appeared before me in my State and County aforesaid and acknowledged the same.

Given under my hand this 15th day of Jone, 20 22

atricea Emiarie

Notary Public

My Commission expires: $\frac{4/30}{a0a3}$ ID: $\frac{274953}{30}$





MANASSAS, VIRGINIA

CASE #___2022-0001___ (Completed by City Staff)

Community Development ' 9800 Godwin Drive, Manassas, VA 20110 ' 703-257-8223

SPECIAL/LIMITED POWER OF ATTORNEY

	THIS	d	ay of	2	.0_22, 1	, Please	e see attach	ed.		, the
owner	of Tax M	lap Nu	unber	100-01-00-	202			, make	e, constitu	te and
appoint	Timo	thy Du	ngan				, my	true and lay	vful attorr	ney-in-
fact,	and	in	my	name,	place	and	stead	giving	unto	said
Tin	othy Du ga	11			ful	l power a	nd authorit	ty to do and	perform a	all acts
and ma	ke all rej	present	ation n	ecessary, wi	thout any	limitation	n whatsoev	ver, to make	applicati	ion for
(circle	one) (Sp	ecial U	se Per	mit), (Rezor	ning), (Ar	chitectur	al Review	Board), (B	oard of Z	Loning
Appeal	ls) in com	nection	with th	e above desc	ribed real	property.				

The right, powers and authority of said attorney-in-fact herein granted shall commence and be in full force and effect on _______, 20_____, and shall remain in full force and effect thereafter until actual notice, by certified mail, return receipt requested is received by the Office of Community Development of the City of Manassas stating that the terms of this power have been revoked or modified.

Owner's Name (Please Print)

	Please	see the attached owner's signature	e pages.
	Owner's	Signature	
STATE OF			
COUNTY OF			
I,	, a Notai	y Public in and for the State and Co	ounty
aforesaid, do hereby certify that		V	vhose
name is signed to the foregoing, this day per-	sonally appear	ed before me in my State and Co	ounty
aforesaid and acknowledged the same.			
Given under my hand this	day of	. 20	
	D	, , .	
	Notary Pu	blic	
My Commission expires:	ID;		
Revised December 2021			

ATTACHMENT TO SPECIAL/LIMITED POWER OF ATTORNEY SPECIAL USE PERMIT **OWNERS SIGNATURES** Tax Map Number: 100-01-00-202

OWNER:

Karen E. Phillips, Trustee of the Kenneth F. Parsons Revocable Trust under agreement dated August 9, 2011, as amended on February 24, 2015 and on January 1, 2021

CITY COUNTY OF __ RICHMOND

I, Patricia Femiani, a Notary Public in and for the State and County aforesaid, do hereby certify that <u>Karen E. Phillips</u> whose name is signed to the foregoing, this day personally appeared before me in my State and County aforesaid and acknowledged the same.

Given under my hand this 15th day of June, 20 22.

Patricia Jamieni Notary Public

My Commission expires: $\frac{1/30/2023}{10: 27495}$ ID: 27495



ATTACHMENT TO SPECIAL/LIMITED POWER OF ATTORNEY SPECIAL USE PERMIT **OWNERS SIGNATURES** Tax Map Number: 100-01-00-202

OWNER:

Karen E. Phillips, Trustee of the Kenneth F. Parsons Revocable Trust under agreement dated August 9, 2011, as amended on February 24, 2015 and on January 1, 2021

CITY COUNTY OF __ RICHMOND

I, Patricia Femiani, a Notary Public in and for the State and County aforesaid, do hereby certify that <u>Karen E. Phillips</u> whose name is signed to the foregoing, this day personally appeared before me in my State and County aforesaid and acknowledged the same.

Given under my hand this 15th day of June, 20 22.

Patricia Jamieni Notary Public

My Commission expires: $\frac{1/30/2023}{10: 27495}$ ID: 27495



land Phillips Stutter

Karen E. Phillips, Trustee of the Kathleen E. Parsons Revocable Trust under agreement dated August 9, 2011, as amended on February 24, 2015

CTY COUNTY OF RICHMOND

I. Patricia Femani, a Notary Public in and for the State and County aforesaid, do hereby certify that <u>Karen E. Phillips</u> whose name is signed to the foregoing, this day personally appeared before me in my State and County aforesaid and acknowledged the same.

Given under my hand this 15th day of JUNE

Patricia Terriarie Notary Public

My Commission expires: 4/30/2023 ID: 274952 -----PUBLIC AMISSIC EXPIRES MEALTH

R. Scott Phillips, Trustee of the Kathleen E. Parsons Revocable Trust under agreement dated August 9, 2011, as amended on February 24, 2015

STATE OF VIRGINIA CITY COUNTY OF RICHMOND

> I, <u>Patricia Femiani</u>, a Notary Public in and for the State and County aforesaid, do hereby certify that <u>R. Scott Phillips</u> whose name is signed to the foregoing, this day personally appeared before me in my State and County aforesaid and acknowledged the same.

Given under my hand this 1546 day of June, $20 \rightarrow 2$.

ricedaniant

Notary Public

My Commission expires: 4/30/2023 ID: 274952



NOTES

THIS IS A SUP FOR A CHILD CARE CENTER AND ELEMENTARY SCHOOL. THE SUBMISSION PACKAGE CONTAINED HEREIN INCLUDES A GENERAL DEVELOPMENT PLAN (GDP).

NOTES

- 1. THE PROPERTIES SHOWN HEREON ARE IDENTIFIED ON THE CITY OF MANASSAS, VIRGINIA GEOGRAPHIC INFORMATION SYSTEM, AS MAP:#100-01-00-202.
- 2. THE PROPERTIES SHOWN HEREON ARE CURRENTLY IN THE NAMES OF KENNETH F PARSONS AND KAREN E PHILLIPS TRUSTEES, RECORDED AT INSTRUMENT #2015052600040422 AMONG THE LAND RECORDS OF PRINCE WILLIAM COUNTY.
- GROSS SITE AREA: 145,055 SQ. FT. OR 3.33 ACRES.
- CURRENT ZONING R-1: LOW DENSITY. THE SUBJECT PROPERTY IS WITHIN THE HISTORIC OVERLAY DISTRICT.
- 5. EXISTING USE: MIXED USES MDL-94
- 6. THE BOUNDARY AND TOPOGRAPHICAL INFORMATION SHOWN HEREON IS BASED ON A FIELD SURVEY BY christopher consultants ON NOVEMBER 23, 2021.
- 7. TO THE BEST KNOWLEDGE OF THE ENGINEER AND DEVELOPER. THERE ARE NO GRAVES OR BURIAL SITES LOCATED ON THE PROPERTY.
- 8. TO THE BEST KNOWLEDGE OF THE ENGINEER AND DEVELOPER. THERE ARE NO HAZARDOUS OR TOXIC SUBSTANCES ON THE PROPERTY.
- 9. THE PROPERTY SHOWN HEREON LIES WITHIN A ZONE 'X', AN AREA TO BE DETERMINED TO BE OUTSIDE A 500 YEAR FLOODPLAIN, AS DELINEATED ON FLOOD INSURANCE RATE MAP NO. 51153C0177D, DATED JANUARY 5, 1995.
- 10. TO THE BEST KNOWLEDGE OF THE ENGINEER AND DEVELOPER, THIS GDP CONFORMS TO APPLICABLE ORDINANCES, REGULATIONS AND ADOPTED STANDARDS, UNLESS OTHERWISE SPECIFICALLY NOTED.
- 11. NO ADVERSE EFFECTS TO NEIGHBORING PROPERTIES ARE ANTICIPATED WITH THIS DEVELOPMENT.
- 12. PROPOSED SIGNAGE WILL CONFORM TO ARTICLE IV OF THE ZONING ORDINANCE AND THE HISTORIC DISTRICT DESIGN GUIDELINES PURSUANT TO THE ARCHITECTURAL REVIEW BOARD'S CERTIFICATE OF APPROPIATENESS FOR SITE FEATURES.
- 13. THE SUBJECT PROPERTY IS WITHIN ONE-HALF MILE OF AN ACTIVE RAILROAD LINE. THIS WILL BE DISCLOSED TO FUTURE PROPERTY OWNERS.
- 14. ALL LIGHTING ALONG PUBLIC STREETS WILL BE IN CONFORMANCE WITH THE DCSM, THE COMPREHENSIVE PLAN AND THE HISTORIC DISTRICT DESIGN GUIDELINES.

GENERAL DEVELOPMENT PLAN CASE #: TBD EDUGENIUS CITY OF MANASSAS, VIRGINIA





GRAPHIC SCALE 1'=500'

	SHEET INDEX
C000	COVER SHEET
C100	EXISTING CONDITIONS
C200	TREE CONSERVATION PLAN
C300	LAYOUT
C400	CONCEPTUAL LANDSCAPE PLAN
C500	UTILITY LAYOUT



OWNER

KENNETH F PARSONS, TRUSTEE AND KAREN E PHILLIPS, TRUSTEES 14319 DUMFRIES ROAD MANASSAS, VA 20112

> APPLICANT EDUGENIUS US REAL ESTATE GROUP LLC 2319 S JOYCE STREET ARLINGTON, VA 22202

> > ATTORNEY BEAN KINNEY & KORMAN C/O TIMOTHY DUGAN 2311 WILSON BLVD., SUITE 500 ARLINGTON, VIRGINIA 22201

CIVIL ENGINEER / LAND PLANNING

christopher consultants, ltd. C/O CHRIS LEMON, P.E. 9301 INNOVATION DRIVE, SUITE 150 MANASSAS, VIRGINIA 20110



5



NOTES:

. SEE TREE PRESERVATION PLAN FOR MORE DETAIL OF SPECIMEN TREES ON SITE.

OWNER	SHIP TABLE	-			
ΤΑΧ ΜΑΡ	OWNER	ADDRESS	INSTRUMENT NUMBER	ZONING	ACREAGE
100-01-00-202	KENNETH AND KAREN PARSONS	9522 FAIRVIEW AVENUE	201505260040422	R1	3.33











HORIZONTAL GRAPHIC SCALE 1" = 30'

	E PR	ESERVATION	SCHEDULE				
TREE	SURVEY			CRITICAL ROOT ZONE RADIUS		CONDITION	
TAG #	TREE #			(FEET)	(INCHES)	RATING	PROCEDURE
	2195 2196	PINUS STROBUS PINUS STROBUS	WHITE PINE	24	24 18	71	REMOVE REMOVE
	2197	PINUS STROBUS	WHITE PINE	16	16	68	REMOVE
	2198	PINUS STROBUS	WHITE PINE	18	18	71 74	
	2199	PINUS STROBUS	WHITE PINE	16	16	68	REMOVE
	2201	PINUS STROBUS		12	12	74	REMOVE
	2202	PINUS STROBUS		18	18	70	REMOVE
	2204	PINUS STROBUS	WHITE PINE	10	10	75	REMOVE
	2205	PINUS STROBUS		24	24	75	
	2200	PINUS STROBUS	WHITE PINE	14	14	73	REMOVE
	2208	PINUS STROBUS		26	26	74	REMOVE
	2228	PINUS STROBUS		20	20	74	REMOVE
	2230	PINUS STROBUS	WHITE PINE	16	16	71	REMOVE
5/99	2231	PICEA ABIES		4	4	75	
5489	2314	ACER SACCHARUM	SUGAR MAPLE	12	12	74	REMOVE
	2319	ILEX OPACA		6	6	72	REMOVE
	2331	PRUNUS SEROTINA		4 8	<u> </u>	70	REMOVE
5487	2333	ACER SACCHARUM	SUGAR MAPLE	20	20	70	REMOVE
104	2362	CEDRUS ATLANTICA		12	12	73	REMOVE
6494	2363	ILEX OPACA		6	<u> </u>	65 75	REMOVE
	2420	PYRUS CALLERYANA	CALLERY PEAR	20	20	65	REMOVE
5495	2426	ACER SACCHARUM		30	30	73	REMOVE
	2403	JUNIPERUS VIRGINIANA	EASTERN REDCEDAR	20	20	71	REMOVE
	2489	JUNIPERUS VIRGINIANA	EASTERN REDCEDAR	22	22	72	REMOVE
	2497	PYRUS CALLERYANA	LALLERY PEAR	20 4	20 4	70 74	
5486	2581	ACER PALMATUM	JAPANESE MAPLE	9	4 9	80	REMOVE
	2601	MAGNOLIA GRANDIFLORA	SOUTHERN MAGNOLIA	14	14	74	REMOVE
	2602	LAGERSTROEMIA INDICA	NUKWAY SPRUCE	12 6	<u>12</u> 6	73 70	
	2686	PYRUS CALLERYANA	CALLERY PEAR	8	8	74	REMOVE
5496	2687	PLATANUS OCCIDENTALIS	SYCAMORE	22 8	22	75	REMOVE
	2689	PLATANUS OCCIDENTALIS	SYCAMORE	14	14	75	REMOVE
	2690	PLATANUS OCCIDENTALIS	SYCAMORE	10	10	73	REMOVE
	2691	PLATANUS OCCIDENTALIS	SYCAMORE SYCAMORE	10	10 4	73 75	
6497	2693	PLATANUS OCCIDENTALIS	SYCAMORE	18	18	76	REMOVE
549	2719	PLATANUS OCCIDENTALIS	SYCAMORE	20	20	85	REMOVE
548 5490	2720	PINUS STROBUS		24	24	67	REMOVE
5491	2807	PINUS STROBUS	WHITE PINE	24	24	74	REMOVE
5493	2808	PINUS STROBUS		24	24	72	
9492	2809	JUGLANS NIGRA	BLACK WALNUT	14		74	REMOVE
	2822	PINUS STROBUS	WHITE PINE	18	18	75	REMOVE
	2828	PRUNUS SEROTINA	BLACK CHERRY	10	10	60 70	
	2833	PRUNUS SEROTINA	BLACK CHERRY	10	10	71	REMOVE
	2840	MORUS ALBA		18	18	65	REMOVE
	2845	PINUS STROBUS	WHITE PINE	12	12	71	REMOVE
	2846	PINUS STROBUS	WHITE PINE	14	14	71	REMOVE
5500	2853 2878	JUGLANS NIGRA	BLACK WALNUT	17	<u> </u>	74	
	2879	PINUS STROBUS	WHITE PINE	8	8	50	REMOVE
	2880	PINUS STROBUS		8	8	72	REMOVE
	2883	JUNIPERUS VIRGINIANA	EASTERN REDCEDAR	20	20	72	PRESERVE
	2885	PINUS STROBUS	WHITE PINE	20	20	73	REMOVE
	2899	ACER PLATANOIDES	NORWAY MAPLE	6	<u>6</u> 28	75 73	
	2938	JUNIPERUS VIRGINIANA	EASTERN REDCEDAR	14	14	75	PRESERVE
	2939	PINUS STROBUS		16	16	75	PRESERVE
	2940 3031	LAGERSTROEMIA INDICA		16	16 4	/5 80	REMOVE
	3034	CORNUS FLORIDA	FLOWERING DOGWOOD	6	6	65	REMOVE
6477	3035	PINUS STROBUS		30	30	80	PRESERVE
	3037	PINUS STROBUS	WHITE PINE	24	24	74	PRESERVE
	3073			18	18	67	REMOVE
5475	3090	ACER SACCHARUM	SUGAR MAPLE	21	21	73	REMOVE
	3114	JUGLANS NIGRA	BLACK WALNUT	22	22	76	REMOVE
	3115	PINUS STROBUS		18	18	72	
5476	3117	QUERCUS ALBA	WHITE OAK	29	29	73	REMOVE
	3119	CORNUS FLORIDA		4	4	75	REMOVE
	3249 3250	ILEX OPACA	AMERICAN HOLLY	6	<u>6</u>	/5 75	
	3251	ILEX OPACA	AMERICAN HOLLY	6	6	75	REMOVE
	3252			4	4	75	
	3253	ILEX OPACA	AMERICAN HOLLY	6	4 6	75	REMOVE
	3273			6	6	75	REMOVE
	3274 3275	ILEX OPACA	AMERICAN HOLLY	6	6	75 75	
5478	3321	ILEX OPACA	AMERICAN HOLLY	15	15	80	PRESERVE
5479	3322			14	14	78	PRESERVE
5480	3327 3328	CERCIS CANADENSIS	EASTERN REDBUD	8	<u> </u>	70 78	PRESERVE
5482	3329	PINUS STROBUS	WHITE PINE	22	22	78	PRESERVE
5481	3330	GINKGO BILOBA		19	19	82	PRESERVE
5483	3350 3351	PICEA ABIES PINUS STROBUS	WHITE PINE	14	14 31	75 74	
	3392	CERCIS CANADENSIS	EASTERN REDBUD	8	8	65	REMOVE
5484	3396		ATLAS CEDAR	15	15	74	REMOVE
	3408	JUNIPERUS VIRGINIANA	EASTERN REDCEDAR	20	20	75 75	REMOVE
	3410	JUNIPERUS VIRGINIANA	EASTERN REDCEDAR	6	6	76	REMOVE
אנ	3450	CARYA ILLINOISNENSIS		29	29	74	
	1 21/11			— I			

NOTE: CONDITION RATINGS ARE BASED ON ISA EVALUATION SYSTEM. EXCELLENT (81-100), GOOD (61-80), FAIR (41-60), POOR (21-40), VERY POOR (6-20), DEAD (0-5).

6



C200



P:\Projects\21361\00100\111720 GDP\4 C300 LAYOUT.dwg, 6/28/2022 12:33:42 PM, DWG To PDF.pc3

PROPERTY BOUNDARY / AREA OF APPLICATION

PROPOSED STANDARD PARKING SPACE COUNT

VEHICLE TRAFFIC CIRCULATION

CHILD CARE CENTER

HEIGHT REQUIREMENTS

PARKING TABULATIONS

REQUIREMENTS: 38 SPACES (1/250 sq.ft. OF CONVENTIONAL CLASSROOM SPACE, 9,500 SQ.FT.)

PROVIDED: 46 SPACES

ELEMENTARY SCHOOL

HEIGHT REQUIREMENTS

PROVIDED: 35' MAXIMUM

PARKING TABULATIONS

REQUIREMENTS: 1/ non-instructional staff 1/20 students PROVIDED: 2 Non-Instructional Staff

120 Students Total: 8 Spaces

1. THE PROPOSED STREETS AND CURBS RADII ARE SUFFICIENT TO HANDLE FIRETRUCK ACCESS.

2. ALL PLAY AREAS WILL BE FENCED IN ACCORDANCE WITH ARTICLE III OF THE ZONING ORDINANCE.



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00	DUT	.001.00 20				CITY OF MANASSAS, VIRGINIA	9301 innovation dr p 703.393.986 suite 150 manassas, va 20110
			MARK D	ATE	DESCRIPTION		engineering • surveying • land planning





1" = 20'



LEGEND PROPERTY BOUNDARY / AREA OF APPLICATION PROPOSED WATER MAIN PROPOSED SEWER MAIN Stisting water line EXISTING SEWER LINE PROPOSED FIRE HYDRANT PROPOSED STORMWATER PIPE EXISTING FIRE HYDRANT NOTES:	Christopher Consultants 9301 innovation dr p 703.393.9887 suite 150 manassas, va 20110 engineering • surveying • land planning
 SWM AND BMP WILL BE HANDLED BY CONTRIBUTING TO THE CITY'S PRO-RATA SHARE OR WILL BE PROVIDED ON SITE GENERALLY AS SHOWN. THE DRAINAGE FOR THIS SITE OUTFALLS INTO THE WINTER'S BRANCH POND. THE PROPOSED WATER AND SEWER SHALL BE CONNECTED TO EXISTING PUBLIC WATER AND SEWER SITH PROPERTY AS CERTIFIED BY christopher consultants ON NOVEMBER 23, 2021. 	EDUGENIUS BUGENUS GENERAL DEVELOPMENT PLAN CITY OF MANASSAS, VIRGINIA
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PROPOSED: 8 SPACES (2 NON-INSTRUCTIONAL STAFF, 120 STUDENTS)

CHILD CARE CENTER

REQUIREMENT: 38 SPACES (1 PER 250 SF OF CLASSROOM SPACE)

PROPOSED: 46 SPACES

christopher	consultants	9301 innovation dr p 703.393.988 suite 150	manassas, va 20110	engineering • surveying • land planning
EDUGENIUS	9522 FAIRVIEW AVENUE		CITY OF MANASSAS, VIRGINIA	
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PROJECT N DRAWING N DATE: 1/27/2 SCALE: 1" = DESIGN: DL DRAWN: DL CHECKED: SHEET TITL CHECKED: SHEET TITL	о.: 2136 ю.: 1114 2022 80' МК Е: NCE	1.001.00 55 PTU LAY	AL OL	MARK

ATTACHMENT TO SOJ

Tillet Hall Parking

Day Care = ITE Parking Generation Based On Square Feet

ITEParkGen Web-based App

Graph Look Up



? Help

ATTACHMENT TO SOJ

Tillet Hall Parking

Day Care = ITE Parking Generation Based On Number of Students

ITEParkGen Web-based App



? Help

SPECIAL USE PERMIT APPLICATION FOR CHILD CARE CENTER¹ AND ELEMENTARY SCHOOL² Applicant: Edugenius US Real Estate Group, LLC³ Tax Map Number: 100-01-00-202 Tillet Hall,⁴ 9522 Fairview Avenue, City of Manassas, Virginia 20110 Zoned: R-1 Land Area: Approx. 145,055 SF or 3.33 acres (the "Property") STATEMENT OF JUSTIFICATION ("SOJ") June 29, 2022

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¹ Definition: Child care center means the use of land for any facility operated for the purpose of providing care, protection, and guidance for two or more children separated from their parents or guardians during a part of a day, but excluding a family day home as defined by this chapter. Source: Section 130-42 Definitions, Manassas Zoning Ordinance. The term "daycare," "day care" and other permutations are sometimes used among the Application materials in lieu of child care, but "daycare," "day care" etc. are intended to mean the same use as child care.

² Definition: For brevity in this SOJ, "Elementary School" means "Educational facility, primary or secondary and means the use of land for a public, private, religious, or parochial school offering instruction for grades prekindergarten through, but not beyond, the 12th grade, and facilities that provide special educational and related services to children with disabilities. Source: Section 130-42 Definitions, Manassas Zoning Ordinance.

³ The Applicant and contract purchaser is Edugenius US Real Estate Group, LLC, a Virginia limited liability company. The fee simple owners are: (1) Karen E. Phillips, Trustee of the Kenneth F. Parsons Revocable Trust under agreement dated August 9, 2011, as amended on February 24, 2015 and on January 1, 2021; (2) Karen E. Phillips, Trustee of the Kathleen E. Parsons Revocable Trust under agreement dated August 9, 2011, as amended on February 24, 2015; and (3) R. Scott Phillips, Trustee of the Kathleen E. Parsons Revocable Trust under agreement dated August 9, 2011, as amended on February 24, 2015; and (3) R. Scott Phillips, Trustee of the Kathleen E. Parsons Revocable Trust under agreement dated August 9, 2011, as amended on February 24, 2015; and (3) R. Scott Phillips, Trustee of the February 24, 2015; and (3) R. Scott Phillips, Trustee of the February 24, 2015; and (3) R. Scott Phillips, Trustee of the February 24, 2015; and (3) R. Scott Phillips, Trustee of the February 24, 2015; and (3) R. Scott Phillips, Trustee of the February 24, 2015; and (3) R. Scott Phillips, Trustee of the Kathleen E. Parsons Revocable Trust under agreement dated August 9, 2011, as amended on February 24, 2015.

⁴ Unless explicitly indicated to the contrary, the term "Tillet Hall" means both Tillet Hall and the Carriage House.

STATEMENT OF JUSTIFICATION SPECIAL USE PERMIT APPLICATION FOR CHILD CARE CENTER AND ELEMENTARY SCHOOL Applicant: Edugenius US Real Estate Group, LLC Tillet Hall, 9522 Fairview Avenue, City of Manassas, Virginia 20110 June 29, 2022 Page 2 of 32

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Introduction/Proposal/Proposed Use 1.

The Special Use Permit Application involves the development of:

(1) Phase 1: a child care center of approximately 125 students and the preservation of the historic Tillet Hall and the Carriage House;⁵ and

(2) Phase 2: an elementary school of approximately 120 students.

The development is collectively referred to as the "Edugenius Project."

The Property is generally oriented north to south. Fairview Avenue borders the Property's east side. Wesley Avenue borders the Property's south side. Immediately to the north is the City-owned property, currently used as Police Headquarters. Immediately to the west of the Property is the Manassas Arms L.L.C. apartments.

Phase 1, the child care center, will be developed on the Property's south side. The building footprint will run east to west near and parallel to Wesley Avenue.

Phase 2, the elementary school, will be developed on the Property's north side. Its building footprint will run east to west near and parallel to the Property's northern common property line with the Police Headquarters.

The Special Use Permit Application is required because the two uses are allowed in the R-1 zone but are subject to a Special Use Permit approval. Please see Section 130-241 of the Zoning Ordinance.

⁵ By way of emphasis, it is repeated that unless explicitly indicated to the contrary, the term "Tillet Hall" means both Tillet Hall and the Carriage House. Thus, "Carriage House" will no longer be repeated throughout this SOJ.

STATEMENT OF JUSTIFICATION SPECIAL USE PERMIT APPLICATION FOR CHILD CARE CENTER AND ELEMENTARY SCHOOL Applicant: Edugenius US Real Estate Group, LLC Tillet Hall, 9522 Fairview Avenue, City of Manassas, Virginia 20110 June 29, 2022 Page 4 of 32

The Edugenius Project will complement the character and development of the Downtown's traditional neighborhood. It will be in harmony with the purposes of the City of Manassas' plan of land use.⁶

2. About Edugenius Child Care Center and Elementary School

2.1 <u>Phase 1 Child Care Center "miniGenius"</u>

The child care center program, also known as "miniGenius," is a Montessori self-directed learning concept that is integrated with additional pedagogical approaches. Edugenius considers every child as unique and that each child should be its own teacher. Special developmental phases of a child are perceived. Edugenius corresponds to these phases of development in which education takes place with a special "prepared environment" designed according to scientific standards. Each child's development is documented. If necessary, a child is helped to "make it on his own." Clear rules are developed. Good values are conveyed, especially through credible examples. The physical educational environment includes a diverse set of learning spaces and tools available inside the classrooms and outdoors on the playgrounds, where they play and learn through hands-on tasks such as growing plants. Edugenius also will offer German-language immersion to first expose the child care children to different languages and cultures.

2.2 <u>Phase 2 Elementary School "poliGenius"</u>

Similarly, the elementary school program, also known as "poliGenius," is a Montessori self-directed learning concept that is integrated with additional pedagogical approaches. Parents and caregivers are attracted to the education because of its comprehensive promotion of the children's uniqueness and individual abilities. The goal is to provide an extended general education that enables individual focus and enables children and young people to continue their education according to their talents and abilities. In addition, the goal emphasizes the

⁶ Please see Article IX, Division 1, Section 130-501 Special Use Permits of the City of Manassas Zoning Ordinance.

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importance of facilitating the broadest possible promotion of the children's diverse talents and creative powers. The physical educational environment includes a diverse set of learning spaces and tools available inside the classrooms and outdoors on the playgrounds where the play and learn. Edugenius will offer German-language immersion to first expose the elementary school children to different languages and cultures.

3. Architectural Review Board Certificates of Appropriateness

The Property is located within the City of Manassas Historic Overlay District. The Architectural Review Board ("ARB") approved the Edugenius Project's concept/massing and materials and issued Certificates of Appropriateness. The Certificates of Appropriateness and the supporting illustrations and pictures of the concept/massing and materials are included among the SUP Application materials. The ARB's approvals are evidence of the Application meeting the City's Historic District Guidelines and the Comprehensive Plan objectives.

4. <u>Compliance with the Manassas 2040 Comprehensive Plan⁷</u>

At Chapter 1, the Manassas 2040 Comprehensive Plan begins with a heading that reads, "Manassas 2040 - Historic Heart. Modern Beat." The Edugenius Project proposes the preservation of Tillet Hall and the development of a modern and compatible facility, offering child care and elementary school education. The Edugenius Project's respect for the City's history and harkening to the City's future will implement the Plan's aspirations.

4.1 <u>Chapter 2, Community Overview, Forces and Trends Shaping</u> <u>Manassas City</u>

Providing a new project offering child care and elementary school education responds to the City's recent population growth. More children will be in need of quality educational facilities. The project is an example of balancing: (1) the need to preserve the community's history, with respect to Tillet Hall; and (2) the need to adapt and to serve the needs of a growing population.

⁷ The Comprehensive Plan means the Adopted 2040 Comprehensive Plan. Subsequently, for brevity, it is referred to as the "Plan."

STATEMENT OF JUSTIFICATION SPECIAL USE PERMIT APPLICATION FOR CHILD CARE CENTER AND ELEMENTARY SCHOOL Applicant: Edugenius US Real Estate Group, LLC Tillet Hall, 9522 Fairview Avenue, City of Manassas, Virginia 20110 June 29, 2022 Page 6 of 32

The Edugenius Project will be a compact, urban infill and revitalization project. It will provide a new education facility. By preserving Tillet Hall, the project upholds the neighborhood's integrity and character. The proposed steps are consistent with the Housing and Neighborhoods objectives at Chapter 2, Page 17.

Please also see the discussion about how the Edugenius Project meets the objectives of Chapter 4, Housing & Neighborhoods, of the Plan at Section 4.3, beginning at Page 11 of this SOJ.

The Edugenius Project will foster economic development directly and indirectly. The physical site development and construction of the Edugenius Project will directly generate work for those responsible for developing the site and constructing the school buildings. The operations of the schools will directly provide jobs for teachers and administrators. Indirectly, the availability of the Edugenius Project's high quality education will reinforce the decisions of the City's educated workforce to live, work, play, and raise a family in Manassas. It will contribute to attracting others living outside the City to join the ranks of the City's educated workforce. The intended outcomes are consistent with the Plan's discussions of both: (1) Housing and Neighborhoods objectives at Chapter 2, Page 18; and (2) Economic Development objectives at Chapter 5, Page 75.

If there is a legal nexus and it is proportional to its impact on public facilities,⁸ the Edugenius Project will undertake to install public infrastructure, which would contribute to enhancing mobility and other community facilities and infrastructure, as applicable, consistent with the Housing and Neighborhoods objectives at Chapter 2, Pages 19-20.

The Edugenius Project is consistent with the overarching themes expressed at Chapter 2, Pages 22-23.

The Edugenius Project will reinvigorate the neighborhood while preserving Tillet Hall, a part of the City's history.

⁸ Please see generally, *Dolan v. City of Tigard*, 512 U.S. 374 (1994) and *Nollan v. California Coastal Commission*, 483 U.S. 825 (1987) and their progeny.

4.2 <u>Chapter 3, Land Use</u>

4.2.1 <u>Chapter 3 Goal, Objectives and Strategies</u>

Page 27 of the Plan identifies the "balanced land use" objective.

The most pertinent strategy concerns "Preserving and enhancing existing residential areas while promoting infill development that is consistent with and contributes to that neighborhood's character" as L.U. 3.1.3. The Architectural Review Board ("ARB") analyzed the Edugenius Project as an infill development. Most particularly, the ARB considered the compatibility of its massing and materials with the neighborhood and with the buildings and setting of Tillet Hall. As evidenced by the ARB's approvals, the Edugenius Project meets such objective of retaining the neighborhood's character. It will preserve Tillet Hall. The new buildings are designed to be compatible with Tillet Hall.

At Page 28, the Plan addresses "Redevelopment." The Edugenius Project will be one step toward accomplishing L.U. 3.2.2, by elevating the community's appearance and contributing to the area's character by virtue of its investment in the long standing established neighborhood.

At Page 29, the Plan encourages an update of the Architectural Review and Historic Overlay regulations, L.U.3.3.1., which undertaking was concluded in time for the filing of this Statement of Justification. The Applicant explains how the Edugenius Project has followed the newly adopted "Historic District Design Guidelines" through its application with the Architectural Review Board.

4.2.2 Chapter 3 Character Areas and Design

With respect to the discussions in the Plan concerning: (1) the Downtown Area's design principles for residential areas, beginning at Page 46 of the Plan; (2) the Downtown Opportunity Areas, beginning at Page 51 of the Plan; and (3) the cross-reference to the Traditional Neighborhoods design principles, beginning at Page 37 of the Plan, the Applicant first STATEMENT OF JUSTIFICATION SPECIAL USE PERMIT APPLICATION FOR CHILD CARE CENTER AND ELEMENTARY SCHOOL Applicant: Edugenius US Real Estate Group, LLC Tillet Hall, 9522 Fairview Avenue, City of Manassas, Virginia 20110 June 29, 2022 Page 8 of 32

incorporates by reference the deliberations of the ARB involving the Certificates of Appropriateness as the most pertinent evidence and explanation of how the Application meets the Plan recommendations. By way of emphasis, this SOJ includes a discussion about the Downtown Character Area in Section 4.2.3, which also references the Traditional Neighborhoods and the Tillet Hall property along Fairview Avenue.

4.2.3 Chapter 3 Downtown Character Area

4.2.3.1 The Project meets the Appropriate Land Use Recommendations, Focus Priorities, Downtown Neighborhood Design Principles, and the overall Objective for Design Principles.

At Page 46 of the Plan, the Downtown Character area identifies institutional uses such as child care and elementary school as appropriate land uses.

At Page 46-47 of the Plan, under "Focus Priorities," and "Objective," the applicable language reads as follows:

(From "Focus Priorities")

Encourage historic patterns of development to retain historic charm.

Maintain harmony and compatibility by evaluating the appropriateness of architectural features, materials, scale, size, height, and placement of new structures in relationship to existing structures and to the setting.

Support the existing historic district and explore the expansion or creation of new historic districts with additional structures.

• • •

STATEMENT OF JUSTIFICATION SPECIAL USE PERMIT APPLICATION FOR CHILD CARE CENTER AND ELEMENTARY SCHOOL Applicant: Edugenius US Real Estate Group, LLC Tillet Hall, 9522 Fairview Avenue, City of Manassas, Virginia 20110 June 29, 2022 Page 9 of 32

Preserve, protect, and enhance residential neighborhoods within and around the commercial core.

(From "Objective")

. . .

Existing residential neighborhoods should be preserved and protected, while allowing new residential infill and supportive retail services that enhance the character and quality of the district.

At Page 47-49, of the Plan, under Design Principles, the applicable language for "Downtown Neighborhoods" at Page 49, reads as follows:

Development and redevelopment within the Downtown Neighborhoods should follow the design principles of the Traditional Neighborhood character area. [Note: Please see the discussion about how the Application meets the design principles of the Traditional Neighborhood character area at Section 4.2.4, beginning at Page 11.] New neighborhood development should be connected to and integrated with the Downtown Core in accordance with the recommendations of the Mobility chapter. Note: Please see the discussion about how the Application addresses the recommendations of Chapter 6, the Plan's Mobility Chapter at Section 4.4, beginning at Page 12.]

New development should respect the Downtown's historic context through the use of historic Manassas materials and architectural styles, consistent streetscape STATEMENT OF JUSTIFICATION SPECIAL USE PERMIT APPLICATION FOR CHILD CARE CENTER AND ELEMENTARY SCHOOL Applicant: Edugenius US Real Estate Group, LLC Tillet Hall, 9522 Fairview Avenue, City of Manassas, Virginia 20110 June 29, 2022 Page 10 of 32

treatments, and pedestrian amenities. New development may incorporate a mix of modern and historic components with context-sensitive review, per the Infill Development Guidelines, 2006, as amended.

The Applicant notes that the Certificates of Appropriateness are evidence that the above recommendations have been addressed satisfactorily.

4.2.3.2 Chapter 3 Tillet Hall Is One of the Opportunity Areas

Tillet Hall is among the 12 properties listed as "Downtown Opportunity Areas" on Map 3.2, at page 50 of the Plan, concerning Chapter 3, Land Use. The Property is included in "Circle 8" of the Opportunity Areas. The comment reads as follows:

The relocation of the Police Headquarters, the existence of underutilized parcels along Fairview Avenue, and redevelopment potential of the Head Trauma site provide an opportunity to undertake a small master plan of this area, preserving the existing historic structures, and incorporating a range of residential densities that transition to the surrounding neighborhoods and provide high-quality amenities and open space. *The viewshed and historic site features surrounding Tillett Hall should be protected*.

(Emphasis added.)

The ARB granted Certificate of Appropriateness for concept/massing and materials. The Applicant refers to the ARB's decisions as evidence that the Application STATEMENT OF JUSTIFICATION SPECIAL USE PERMIT APPLICATION FOR CHILD CARE CENTER AND ELEMENTARY SCHOOL Applicant: Edugenius US Real Estate Group, LLC Tillet Hall, 9522 Fairview Avenue, City of Manassas, Virginia 20110 June 29, 2022 Page 11 of 32

adequately protects the historic site features surrounding Tillet Hall.

4.2.4 <u>Chapter 3 Design Principles Of The Traditional</u> <u>Neighborhood</u>

The Plan's section concerning the Character Area "Traditional Neighborhood" begins at Page 37. The appropriate land uses include "institutional." The relevant focus priorities read as follows:

Preservation of urban form, historic structures, and landmarks.

Preservation of traditional scale and architectural styles.

Maintenance of a balanced mix of housing types.

Maintenance and upkeep of existing property.

Compatibility of infill development and redevelopment.

Walkability and connectivity for pedestrians and cyclists.

Because the above design related objectives are so similar to the ones in the Downtown Area of the Plan, we avoid redundancy by referring to the earlier discussion at Section 4.2.3 beginning at Page 8 of this SOJ.

4.3 <u>Chapter 4 Housing & Neighborhoods</u>

The Edugenius Project meets important goals addressed in Chapter 4, by being sensitive to the preservation of Tillet Hall, which in turn aids in the preservation of the caliber of the neighborhood. Even though housing is not being provided on site child care and elementary schools support vibrant housing. Because the Edugenius Project is architecturally compatible with Tillet Hall and operationally compatible with the neighborhood, it enhances the long term quality and character of the
STATEMENT OF JUSTIFICATION SPECIAL USE PERMIT APPLICATION FOR CHILD CARE CENTER AND ELEMENTARY SCHOOL Applicant: Edugenius US Real Estate Group, LLC Tillet Hall, 9522 Fairview Avenue, City of Manassas, Virginia 20110 June 29, 2022 Page 12 of 32

neighborhood, consistent with the language at Chapter 4, Page 68 of the Plan. The Edugenius Project supports and reinforces the neighborhood through its revitalization of the Tillet Hall campus. See Chapter 4, Page 69.

Other objectives:

Chapter 4, Page 69, HOU 4.1.2 Improve pedestrian and bike mobility within and across neighborhoods by providing safe and comfortable connections to key activity areas, such as Downtown, schools, and parks.

Please see the discussion about public facilities at Section 4.6 beginning at Page 14 of this SOJ.

Chapter 4, Page 71, HOU 4.3.5 Implement character area design principles to ensure that infill, rebuilds, and expansions are compatible with, and enhance, neighborhood character. Use guidelines to encourage the use of contextual building materials, establish uniform build-to lines for consistent setbacks, ensure that building heights and upper floor step backs are compatible with existing structures, discourage dead-end streets and cul-desacs to facilitate neighborhood connectivity, encourage short blocks for improved walkability, allow on-street parking, and locate parking lots behind buildings to improve aesthetics and walkability.

Please see the discussion about public facilities at Section 4.6 beginning at Page 14 of this SOJ. Please also see the discussion concerning the Downtown Area of the Plan at Section 4.2.3 beginning at Page 8 of this SOJ.

4.4 <u>Chapter 5 Economic Development</u>

Chapter 5 begins with the following observation that well-suited for the Edugenius Project:

A strong local economy is intrinsically tied to a high quality of life. Businesses make location STATEMENT OF JUSTIFICATION SPECIAL USE PERMIT APPLICATION FOR CHILD CARE CENTER AND ELEMENTARY SCHOOL Applicant: Edugenius US Real Estate Group, LLC Tillet Hall, 9522 Fairview Avenue, City of Manassas, Virginia 20110 June 29, 2022 Page 13 of 32

and expansion decisions based not only on the local workforce and business environment, but also on the community's ability to offer employees the lifestyle they desire. In addition to workforce availability and the costs of doing business in a community, employers are increasingly interested in the quality of schools, range of housing options, and availability of recreation and cultural amenities.

The quality of the child care and elementary school education to be offered at the Edugenius Project will contribute to the City's inventory of schools and foster the City being attractive to an educated workforce and to employers interested in locating in a community that helps attract a talented workforce.

The Economic Development chapter describes steps that can be accomplished by the City. The Edugenius Project will be a type of redevelopment project that advances the City's goal to preserve an existing historic asset, Tillet Hall, and to revitalize the property by introducing a new child care center and elementary school. See Pages 76-78 of Chapter 5 of the Plan. Also, although the Edugenius Project will not be a part of the public school system, it will contribute to "providing an educated, skilled workforce to fill the community's needs" by affording quality child care and elementary school instruction which will be a foundation or stepping stone for students excelling in academic pursuits at their current level and at even higher levels of education.

4.5 Chapter 6 Mobility, Comprehensive Plan

Recommended Transportation Projects are included in Chapter 6, Mobility, beginning at Page 85 of the Plan. On Page 86, Map 6.1 and Table 6.1 identify Fairview Avenue as an area for improving pedestrian safety. Note 16, of Table 6.1, Page 87, reads as follows:

Improve pedestrian safety and access along the Fairview Ave. corridor.

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The stretch of Fairview Avenue along the Property is improved with a sidewalk on both sides and street lighting. The posted speed limit is 25 MPH.

Map 6.2 at Page 9 of Chapter 6 of the Plan appears to list Fairview Avenue as a "Neighborhood Connector." The Transportation and Mobility Master Plan appears to classify Fairview Avenue as a "Neighborhood Connector" and Wesley Avenue as a "Local Road."

The cross section narrative for the "neighborhood connector street," in the Transportation and Mobility Master Plan reads as follows:

DESCRIPTION These are the more heavilytrafficked streets within neighborhoods, which provide important connections to major streets. These streets typically are wider than neighborhood streets, with a marked center line. While speeds are relatively low, they require some additional protection for cyclists (bike lanes or sharrows⁹).

VEHICLE PROVISION: 10' Travel Lanes 7' Parking Lanes BICYCLE PROVISIONS: 5' Bike Lane PEDESTRIAN PROVISION: 5' Minimum Sidewalk, both sides of street EXAMPLE STREETS: Stonewall Road Cloverhill Road.

The Comprehensive Plan rights of way recommendations are part of the Application's considerations. Please refer to Section 4.6 beginning at Page 14.

4.6 Chapter 7 Community Facilities and Infrastructure

At Page 100, Chapter 7, Community Facilities and Infrastructure, the Plan acknowledges the City's responsibility to provide services but also the responsibility of applicants to

⁹ A "sharrow" is a roadway marker indicating a lane area to be shared by autos and bicycles.

STATEMENT OF JUSTIFICATION SPECIAL USE PERMIT APPLICATION FOR CHILD CARE CENTER AND ELEMENTARY SCHOOL Applicant: Edugenius US Real Estate Group, LLC Tillet Hall, 9522 Fairview Avenue, City of Manassas, Virginia 20110 June 29, 2022 Page 15 of 32

mitigate its impact on capital costs, if applicable. It reads as follows:

It is Manassas' responsibility to provide services in conformance with the adopted standard. In addition, applications for land use decisions are expected to provide reasonable mitigation to offset the capital cost associated with additional demand, as allowed by the Code of Virginia.

The Applicant will consider what, if any, capital costs might be associated with the Edugenius Project.

It should be noted that the Edugenius Project will contribute toward the City having more available capacity in its public schools to the extent that the students at the Edugenius Project will not be using capacity in the public schools. At Page 109, Table 7.1, Level of Service, the Plan lists a school level of service goal of having no more than 100% utilization of facilities for pre-kindergarten, elementary, intermediate, middle and high school. The Edugenius Project is planned to have a child care center of approximately 125 students and an elementary school of approximately 120 students.

4.7 <u>Chapter 8 Environmental Sustainability and Health</u>

- 4.7.1 <u>Objectives and Strategies</u>
 - 4.7.1.1 ESH 8.1, Adopt a Broad-Based Sustainability Ethic That Incorporates Best Practices and Modern Approaches

At Page 113 of the Plan, Objective EHS 8.1.1 recommends establishing a holistic sustainability plan and consideration of implementing Leadership in Energy and Environmental Design (LEED).

The Edugenius Project is expected to meet the appropriate green building standards customarily applied for similar forms of developments and uses.

STATEMENT OF JUSTIFICATION SPECIAL USE PERMIT APPLICATION FOR CHILD CARE CENTER AND ELEMENTARY SCHOOL Applicant: Edugenius US Real Estate Group, LLC Tillet Hall, 9522 Fairview Avenue, City of Manassas, Virginia 20110 June 29, 2022 Page 16 of 32

4.7.1.2 ESH 8.2, Vegetation and Tree Canopy. Support diverse native vegetation as a core function of the urban landscape to boost property values, cut energy consumption, reduce costs for stormwater management and erosion control, and make the City a more beautiful place.

Page 114 of the Plan includes multiple objectives that are implemented and reflected in the Edugenius Project.

The Application includes plans to preserve trees including those growing near Tillet Hall, through the layout of the improvements, as shown on the Plans.

4.7.1.3 ESH 8.3, Green Infrastructure, Create a green infrastructure network (see Map 8.1) of interconnected natural areas, parks, and green streets, recognizing the value of nature for a healthy, resilient, and prosperous community.

The Applicant's plans reflect how the Edugenius Project can contribute to fostering such objective.

4.7.1.4 ESH 8.4, Water Resources, Protect, enhance, and restore the integrity of the City's water resources.

Objective/Strategy ESH 8.4.4 reads:

Encourage developers to incorporate environmentally-sensitive approaches to stormwater management into site planning, including low impact development techniques and preservation and restoration of natural land forms.

As noted above, the Applicant's stormwater management plans reflect how the Edugenius Project can contribute to the establishment of such objective. Stormwater quality and quantity controls will be provided on-site to reduce runoff volume and nonpoint source pollution. STATEMENT OF JUSTIFICATION SPECIAL USE PERMIT APPLICATION FOR CHILD CARE CENTER AND ELEMENTARY SCHOOL Applicant: Edugenius US Real Estate Group, LLC Tillet Hall, 9522 Fairview Avenue, City of Manassas, Virginia 20110 June 29, 2022 Page 17 of 32

4.7.1.5 Air Quality and Emissions. ESH 8.5 Create a more safe and secure future by encouraging the reduction of fossil fuel consumption and emissions that are harmful to human health and the environment.

As noted above, the Applicant can consider the implementation of green building standards to design, construct, and operate the project with a reduced emissions footprint.

4.7.1.6 Noise and Light Pollution. ESH 8.6 Promote efforts to reduce the impact of noise and light pollution.

ESH 8.6.1 Apply special use permit conditions and accept rezoning proffers that limit lighting and noise impacts on adjacent properties and the City as a whole. Support development standards that address lighting and noise pollution.

When filed in the course of the site plan application following the Special Use Permit Application, the Applicant's lighting plan would demonstrate that the proposed lighting would cut off significant glare onto adjacent properties.

The only significant sounds will be from children playing outdoors.

4.8 Chapter 9 Parks, Culture, & Recreation

In addition to the resources offered on campus at the Edugenius Project, the City's parks, culture, and recreation resources will be available to the children. The Edugenius Project school would determine which parks and activities are suitable for the different ages of the children. As examples, Oakenshaw Park is located less than 1 mile away. It offers playgrounds, basketball and a ("baseball" "softball" "t-ball") diamond field. Kinsley Mill Park is located slightly over 1 mile away. It offers picnic tables STATEMENT OF JUSTIFICATION SPECIAL USE PERMIT APPLICATION FOR CHILD CARE CENTER AND ELEMENTARY SCHOOL Applicant: Edugenius US Real Estate Group, LLC Tillet Hall, 9522 Fairview Avenue, City of Manassas, Virginia 20110 June 29, 2022 Page 18 of 32

and grills, playground, climbing wall, and a roller hockey and basketball court. The Manassas Museum renovation will likely be completed before the child care center is operating and certainly before the phase 2 elementary school is operating. The Museum has programs and events for all ages and likely will be one of the places where the children will visit.

As noted in numerous sections of this Statement of Justification, historic preservation is also noted as an important goal in Chapter 9, which reads as follows:

PCR 9.2 Conserve and enhance the City's historically-significant sites and structures for the cultural, educational, and economic benefits they provide to City residents and visitors.

Further, PCR 9.2.4 acknowledges the contribution of private historic preservation. It reads:

PCR 9.2.4 Continue to work with, support, and provide incentives for private-sector investment and use of historic properties

The Edugenius Project will not be public open space. Tillet Hall will remain visible from the perimeter. Tillet Hall has been private property for most, if not all, of its existence. It will continue to be viewed and enjoyed by the public from such vantage point.

5. **Operational Conditions**

- 5.1 Expected Numbers of Students, Faculty and Administrative Staff
 - 5.1.1 Child Care Center

As noted, the Applicant will develop the child care center as the Edugenius Project's first phase. The child care center's population is expected to be comprised generally as follows:

Description	Number
	of Individuals
Students	125

STATEMENT OF JUSTIFICATION SPECIAL USE PERMIT APPLICATION FOR CHILD CARE CENTER AND ELEMENTARY SCHOOL Applicant: Edugenius US Real Estate Group, LLC Tillet Hall, 9522 Fairview Avenue, City of Manassas, Virginia 20110 June 29, 2022

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Description	Number
	of Individuals
Instructional Faculty	22
Non-instructional Staff (Examples:	6
Executive Director, Educational	
Director, Administrative Support	
Staff, Nurse)	

5.1.2 <u>Elementary School</u>

The Applicant will develop the elementary school as the Edugenius Project's second phase. The elementary school's population is expected to be comprised generally as follows:

Description	Number
	of Individuals
Students	120
Instructional Faculty	12
Non-instructional Staff (Examples: Executive Director, Educational Director, Administrative Support Staff, Nurse)	3

5.2 <u>Hours and Days of Operations</u>

5.2.1 <u>Child Care Center</u>

Child Care Center Description	Hours
Drop off	6:30AM-9AM
Drop off: A receptionist and the inst	ructional faculty
will be on campus at 6:30AM in orde	r to check in and
care for the students as they arrive.	Every student is
expected to have been dropped off by	9AM, when the
day's educational activities are sched	luled to begin.
Pick Up	12:00PM (latest
	6PM)

STATEMENT OF JUSTIFICATION SPECIAL USE PERMIT APPLICATION FOR CHILD CARE CENTER AND ELEMENTARY SCHOOL Applicant: Edugenius US Real Estate Group, LLC Tillet Hall, 9522 Fairview Avenue, City of Manassas, Virginia 20110 June 29, 2022

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Child Care Center Description	Hours
Pick Up: Pick up's are expected to be	egin at 12:00PM,
with care givers arriving over the cou	arse of the
afternoon. The latest pick up is expe	ected to be 6PM.

Summer operations are expected to be programmed as well.

Days of Operations: The child care center generally will be open and operating weekdays during the hours indicated above. Other events, such as open houses, parent-teacher conferences, holiday parties, plays, and possibly graduation ceremonies, may occur on weekday or weekends during the day or evenings. As for evening events, they are expected to generally conclude no later than 10 PM.

5.2.2 <u>Elementary School</u>

Elementary School Description Hou		Hours
Drop off 7AM-8A		7AM-8AM
Drop off: A receptionist and so	ome of	the instructional
faculty will be on campus at 7AM in order to check in		order to check in
and care for the students as they arrive. Every		ive. Every
student is expected to have been dropped off by 8AM,		
when the day's educational activities are scheduled to		
begin.		
Pick Up Day Program 3PM		
Pick Up: Pick up's are expected to occur at 3PM.		
Pick Up After School		3PM-6PM
Pick Up After School: The elementary school will have		
post 3PM afternoon programs for curriculum		
instruction and to accommodate later arrivals for pick		
up by caregivers.		

Summer operations are expected to be programmed as well.

Days of Operations: Similarly, the elementary school generally will be open and operating weekdays during the hours indicated above. Similarly, other events, such as open houses, parentteacher conferences, holiday parties, plays, and possibly graduation ceremonies, may occur on weekday or weekends STATEMENT OF JUSTIFICATION SPECIAL USE PERMIT APPLICATION FOR CHILD CARE CENTER AND ELEMENTARY SCHOOL Applicant: Edugenius US Real Estate Group, LLC Tillet Hall, 9522 Fairview Avenue, City of Manassas, Virginia 20110 June 29, 2022 Page 21 of 32

during the day or evenings. As for evening events, they are expected to generally conclude no later than 10 PM.

5.3 Parking

Elementary School

The Applicant bases the elementary school parking spaces based upon the Zoning Ordinance's standard based on the number of elementary school children, as reflected in the Application plans and materials.

Child Care Center

Below, the Applicant explains the reasons underlying the determination to calculate the number of parking spaces by using conventional classroom space and excluding common areas such as movement rooms and "Kinderkitchen," in lieu of simply multiplying the Zoning Ordinance's parking rate by the child care center's gross floor area.

5.3.1 <u>Planning and Community Development Department's</u> <u>February 2022 Determination:</u>

During the winter of 2021-2022, before submitting the concept/massing and materials Applications to the Architectural Review Board, and before preparing and filing the Special Use Permit Application, the Applicant corresponded with the Planning and Community Development Department. The purpose was to determine what child care center space should be used to calculate the number of parking spaces. As noted, above, it was determined that the number of parking spaces is to be calculated by using conventional classroom space and excluding common areas such as movement rooms and "Kinderkitchen." Included among the Application materials is the February 28, 2022 Planning and Community Development Department's determination letter. A copy of the letter also is attached to this SOJ as Attachment 1). For convenience, please also read a quotation of such determination letter immediately below.

STATEMENT OF JUSTIFICATION SPECIAL USE PERMIT APPLICATION FOR CHILD CARE CENTER AND ELEMENTARY SCHOOL Applicant: Edugenius US Real Estate Group, LLC Tillet Hall, 9522 Fairview Avenue, City of Manassas, Virginia 20110 June 29, 2022 Page 22 of 32

Per our conversation on February 14, 2022, you have requested written determination to a question on the property listed above (hereinafter the "Property"). The following determination was based on the review of the information in your correspondence dated, February 9, 2022. In response, I provide the following information:

1. The parking requirement for an Educational Facility, Primary or Secondary, pursuant to section 130-204 of the City of Manassas Zoning Ordinance is: 1 space for each non-instructional staff and 1 space for every 20 students.

2. Based on the proposal for an Elementary School on the property with 2 non-instructional staff and 120 students a total 8 parking spaces would be required.

3. The parking requirement for a Day Care, pursuant to section 130-204 of the City of Manassas Zoning Ordinance is: 1 space per 250 Square Feet of Gross Building area.

4. Based on the proposal for a 17,898 square foot Day Care on the property a total of 72 parking spaces would be required.

5. However, the proposed Day Care includes several non-traditional spaces (larger corridors, movement rooms, and a "Kinderkitchen") which overlap with instructional space typically found in a conventional educational facility classroom.

6. Considering the parking standard applied to a Day Care use is typically applied to a traditional Day Care which includes predominantly conventional classroom space, it would be reasonable to evaluate the parking for STATEMENT OF JUSTIFICATION SPECIAL USE PERMIT APPLICATION FOR CHILD CARE CENTER AND ELEMENTARY SCHOOL Applicant: Edugenius US Real Estate Group, LLC Tillet Hall, 9522 Fairview Avenue, City of Manassas, Virginia 20110 June 29, 2022 Page 23 of 32

the proposed Day Care based on the amount of conventional classroom space, excluding common areas such as movement rooms and "Kinderkitchen."

7. Based on the information submitted in your February 9, 2022 correspondence a Day Care with 9,500 square feet of conventional classroom space would require a total of 38 parking spaces. This calculation should be reflected as part of the SUP plan required for approval by City Council of a Day Care and Educational Facility in this zoning district.

(End of quotation.)

The Applicant is following the above determination of what child care center space should be used to calculate the number of parking spaces; that is, calculate the number of parking spaces by using conventional classroom space and excluding common areas such as movement rooms and "Kinderkitchen."

5.3.2 <u>The Applicant's Explanation In Support of the Planning</u> <u>and Community Development Determination</u>

As the Planning and Community Development Department recommended, this Statement of Justification includes the Applicant's explanation in support of using the square footage of the conventional classroom space.

Parking is one of the project's threshold considerations because parking horizontal demands cause a significant ripple effect through, among other considerations: (1) the project's layout; (2) the design of the new buildings; (3) the planned educational programming for the children; (4) the preservation of Tillet Hall and the Carriage House; and (5) tree preservation.

The project is expected to be developed in two phases. The child care center would be the first phase.

A parking concept layout is included among the Application materials. It shows the proposed new school buildings and the STATEMENT OF JUSTIFICATION SPECIAL USE PERMIT APPLICATION FOR CHILD CARE CENTER AND ELEMENTARY SCHOOL Applicant: Edugenius US Real Estate Group, LLC Tillet Hall, 9522 Fairview Avenue, City of Manassas, Virginia 20110 June 29, 2022 Page 24 of 32

locations of the parking spaces. As also shown on the plan, the historic Tillet Hall and Carriage House are intended to be preserved. Tillet Hall is to be used by the non-educational elementary school administration staff. No plans exist for the use of the Carriage House, other than its preservation.

The parking spaces calculations are based on two land use categories from Sec. 130-204, Table 1, under "Assembly and Institutional Uses." The land uses are "Educational Facility, Primary or Secondary" for the elementary school and "Child Care Center" for the child care center.

Elementary School

The elementary school requirements are:

- (1) one (1) space for every non-instructional staff; and
- (2) one (1) space for every twenty (20) students.

The elementary school will include two (2) administrative, non-instructional, staff members. There will be 120 elementary school students. Eight (8) spaces are planned for the elementary school as follows:

Description	Rate	Spaces
Two (2)	one (1) space	2
non-educational	for each	
staff		
120 students	one (1) space	6
	for every	
	twenty (20)	
	students	
Total		8

The Application elementary school parking layout plan shows eight (8) spaces, including two (2) handicapped spaces, located along the north side of the elementary school. STATEMENT OF JUSTIFICATION SPECIAL USE PERMIT APPLICATION FOR CHILD CARE CENTER AND ELEMENTARY SCHOOL Applicant: Edugenius US Real Estate Group, LLC Tillet Hall, 9522 Fairview Avenue, City of Manassas, Virginia 20110 June 29, 2022 Page 25 of 32

Child Care Center

As the City's determination letter notes, the parking rate requirement for a conventional child care center is one (1) space for every 250 square feet of building gross square feet. The Planning and Community Development Department letter observed that the proposed child care center will provide significant additional space not found in a conventional child care center. The additional space causes the building to be larger than a typical child care center. Consequently, the zoning ordinance's standard parking space rate for a child care center yield would be excessive, because it would be based upon the child care center's gross square feet. It was determined that the Applicant calculate the parking spaces should be based only on the gross square feet of the conventional classroom space. Accordingly, the number of parking spaces is calculated as shown below:

Description	Rate	Spaces
9,500 SF of conventional classroom space	one (1) space for every 250SF	38
Additional parking spaces		8
Total Child Care Center (including 2 handicapped spaces)		46

The Applicant's parking plan spaces located near the child care center area equals a total of 46 spaces which includes eight (8) extra spaces, based upon the above calculation.

ITE-Calculated Parking Spaces Based on the 2.45 Spaces Per 1,000 SF GFA

The proposed provision of 46 spaces is consistent with the empirical data collected nationwide by the Institute of Transportation Engineers ("ITE") and published in its manual.

STATEMENT OF JUSTIFICATION SPECIAL USE PERMIT APPLICATION FOR CHILD CARE CENTER AND ELEMENTARY SCHOOL Applicant: Edugenius US Real Estate Group, LLC Tillet Hall, 9522 Fairview Avenue, City of Manassas, Virginia 20110 June 29, 2022 Page 26 of 32

The ITE Manual reports that an average of only 2.45 spaces for every 1,000SF of gross floor area are provided for day care centers (also known as "child care"). Please see the excerpt from the ITE Manual attached to this Statement of Justification as Attachment 2). For the Edugenius child care center, the ITE Manual's rate would generate about 44 parking spaces, approximately the same number of spaces as the 46 spaces calculated above, as follows:

Description	Current Estimate	ITE Manual Rate	Spaces
	of SF of Gross	("day care")	
	Floor Area		
Child Care	17,898 SF	2.45 spaces for every	43.85
Center's		1,000SF of gross floor	
Current		area	
Estimate of			
Gross Floor			
Area for the			
Entire Child			
Care Center			

<u>Parking Spaces Based on the Number of Students in the Child</u> <u>Care ("Day Care") Center</u>

Also, the ITE Manual includes an empirically-derived parking space rate based on the number of students in a day care center ("child care"), in addition to its rate based on gross floor area. The student-based rate yields an even lower number of parking spaces than the rate based on gross square footage. The student-based ITE rate is 0.24 spaces for every day care student. In the case of Edugenius, the ITE rate would generate only 30 spaces. (0.24 spaces/student times 125 students). Please see the excerpt from the ITE Manual attached to this Statement of Justification as Attachment 3).

5.3.3 <u>Request the Mayor and City Council To Exercise Its</u> <u>Authority Under Zoning Code Section 130-501</u>

Avoiding the unnecessary imposition of excessive parking spaces requires relief through the City Council's exercise of its authority STATEMENT OF JUSTIFICATION SPECIAL USE PERMIT APPLICATION FOR CHILD CARE CENTER AND ELEMENTARY SCHOOL Applicant: Edugenius US Real Estate Group, LLC Tillet Hall, 9522 Fairview Avenue, City of Manassas, Virginia 20110 June 29, 2022 Page 27 of 32

to mitigate impacts under Section 130-504(c)(3), "Review Procedure," which reads as follows:

If the city council approves the special use permit request, it may impose reasonable conditions to mitigate the impacts of the proposed use, as permitted by applicable law. All such conditions shall be reasonably related both in nature and extent to the impact of the proposed development. At least one week prior to the city council public hearing, the city shall provide in writing to the applicant the proposed conditions. The applicant shall advise the city, in writing prior to the public hearing, that (i) the applicant agrees to the conditions and stipulates that they are reasonable under applicable law, including a statement that each condition is reasonably related both in nature and extent to the impact of the proposed development, or (ii) that the applicant objects to one or more proposed condition and the reason for said objection.

Without the City Council's exercise of its authority to mitigate impacts arising from a strict reading of the Zoning Ordinance, the number of parking spaces for the Edugenius Project would far exceed the appropriate number for a well-functioning child care center and impose deleterious consequences.

The Applicant asks the City Council to exercise its authority to establish a reasonable condition. That is, to calculate the number of parking spaces by using conventional classroom space and excluding common areas such as movement rooms and "Kinderkitchen."

It is reasonable to exclude the Edugenius educational space that is *not* found in typical child care centers. The ITE Manual provides national empirical evidence that supports a finding that STATEMENT OF JUSTIFICATION SPECIAL USE PERMIT APPLICATION FOR CHILD CARE CENTER AND ELEMENTARY SCHOOL Applicant: Edugenius US Real Estate Group, LLC Tillet Hall, 9522 Fairview Avenue, City of Manassas, Virginia 20110 June 29, 2022 Page 28 of 32

it is reasonable to impose a number of spaces based on the dedicated conventional classroom space.

Such a condition, child care parking based on the conventional classroom space, would not only mitigate but also avoid the negative impacts of an unnecessary sea of asphalt parking.

Based upon a strict application of the zoning ordinance, the estimated number of child care spaces would be close to double the above-calculated amount (Currently estimated: 17,898SF GFA/250SF = 72 spaces). The excessive spaces are unnecessary to accommodate the new project, because the same number of child care students could fit within a smaller building, but at the expense of forfeiting the educational benefits and related development programming that can be provided with the greater common area space and a higher caliber educational experience.

The proposed large corridors, movement room and "Kinderkitchen" are important spaces and integral to the curriculum. Areas for movement are fundamental to the Montessori school of thought. Although not present in conventional educational settings, likely due to cost and other considerations, research has shown that such areas foster social development. They afford opportunities for children from different ages and classrooms to interact. Children learn through play and movement, for example, by walking around and counting objects, dancing, listening to and playing instruments, and touching different materials, all of which activities are valued and better cultivated within the extra area provided for the children's development.

The proposed child care building also includes extra square footage for the "Kinderkitchen" - another space grounded in Montessori principles. Children learn through activities from everyday life. Through small kitchen tasks such as peeling vegetables, cleaning dishes, measuring and transferring liquids or food, children learn, first of all, fine motor skills and being focused, and, second of all, how to take care of objects and spaces. STATEMENT OF JUSTIFICATION SPECIAL USE PERMIT APPLICATION FOR CHILD CARE CENTER AND ELEMENTARY SCHOOL Applicant: Edugenius US Real Estate Group, LLC Tillet Hall, 9522 Fairview Avenue, City of Manassas, Virginia 20110 June 29, 2022 Page 29 of 32

These are examples of how providing the atypical additional square footage is planned for the benefit of the children at the child care center.

Fundamentally, the child care children are afforded greater indoor space than the usual child care center, but the greater indoor space does not generate the need for additional parking spaces.

Again, the number of parking spaces generated by the empirical information from the Institute of Traffic Engineering is very close to the proposed number of Edugenius parking spaces. ITE's findings support the reasonableness of basing the parking spaces only on the conventional classroom space.

Other grounds support the City Council exercising its authority to impose reasonable conditions and mitigate the impact of excessive parking. Excessive parking spaces would encroach physically upon important objectives of the project's design: (1) tree preservation; (2) stormwater management; (3) heat island effect; (4) massing and spacing; and (5) the aesthetic and historic preservation of the Tillet Hall and the Carriage House setting.

The City Council can impose a reasonable condition, basing the parking space calculation on conventional classroom space. The condition will mitigate the impact of what would otherwise be an unnecessary sea of parking, less space for outdoor recreation, less space for tree preservation, and less space for stormwater management facilities, as examples. The City Council's condition would be reasonably related both in nature and extent to lessening the impact of excess surface parking on the proposed development and on the nearby neighborhood, for the reasons noted.

5.3.4 No Permanent School Bus Parking on Campus

In cases where the elementary school conducts off campus outings and the children do not walk to the outing, it will rent a small bus to accommodate approximately 10 students, with an appropriate number of chaperones/teachers. The semi-circular STATEMENT OF JUSTIFICATION SPECIAL USE PERMIT APPLICATION FOR CHILD CARE CENTER AND ELEMENTARY SCHOOL Applicant: Edugenius US Real Estate Group, LLC Tillet Hall, 9522 Fairview Avenue, City of Manassas, Virginia 20110 June 29, 2022 Page 30 of 32

driveway leading up to Tillet Hall is wide enough to accommodate a school bus pick up and drop off and is wide enough to allow for adequate space for other vehicles to pass by safely. Using the driveway avoids the rental school bus interfering with the parking spaces to the north and south. Renting a school bus as needed avoids the need for a permanent parking space.

The child care center does not conduct school bus outings for the small children in child care.

6. <u>Proposed Conditions to Minimize the Impact of the Use on</u> <u>Surrounding Properties</u>

The Applicant anticipates developing the conditions through conferences with the Planning and Development Staff and other sources. The conditions may include, without limitation, the following topics:

Compliance with the Comprehensive Plan and the Zoning Ordinance

Traffic and circulation.

Numbers of students, faculty and administrative staff.

Hours and days of operations.

Parking

Dumpster operations

Massing and materials and site features to comply with the Architectural Review Board's Certificates of Appropriateness

Compliance with the approved Special Use Permit plans.

Establish a Community Liaison contact.

Construction activity.

Compliance with applicable licensing and permits.

7. Fiscal Impact

The new child care and elementary school will be an economic benefit for the City and its residents for the reasons described. STATEMENT OF JUSTIFICATION SPECIAL USE PERMIT APPLICATION FOR CHILD CARE CENTER AND ELEMENTARY SCHOOL Applicant: Edugenius US Real Estate Group, LLC Tillet Hall, 9522 Fairview Avenue, City of Manassas, Virginia 20110 June 29, 2022 Page 31 of 32

8. **Public Utilities**

The Applicant's plans show that the Edugenius Project will be served by available, existing public utilities.

9. Environmental

The Applicant will comply with applicable environmental regulations. More particularly, please see the plans included among the Special Use Permit Application materials that address trees and stormwater management.

10. Traffic Impact Analysis

As noted earlier, the traffic consulting firm of Gorove Slade already provided traffic counts to Transportation. Gorove Slade is scheduled to provide their Traffic Impact Study in July 2022. It is incorporated by reference into this Statement of Justification.

11. Conclusion

For all of the above reasons, the Applicant respectfully requests approval of its Special Use Permit Application and looks forward to working with the City.

We look forward to working with you. Thank you for your consideration.

Timothy Dugan

Timothy Dugan Bean Kinney & Korman, P.C. 2311 Wilson Blvd. 5th Floor Arlington, VA 22201 <u>tdugan@beankinney.com</u> 703-526-5583

STATEMENT OF JUSTIFICATION SPECIAL USE PERMIT APPLICATION FOR CHILD CARE CENTER AND ELEMENTARY SCHOOL Applicant: Edugenius US Real Estate Group, LLC Tillet Hall, 9522 Fairview Avenue, City of Manassas, Virginia 20110 June 29, 2022 Page 32 of 32

Attachments List	
No.	Description
1)	February 28, 2022 Manassas City Planning Department
	determination letter about parking spaces
2)	Excerpt from empirical data collected nationwide by the
	Institute of Transportation Engineers ("ITE") ITE Manual
	Excerpt About Parking Rate for Child Care Center Based on
	Gross Floor Area
3)	ITE Manual Excerpt About Parking Rate for Child Care Center
	Based on the Number of Students



Department of Community Development Planning & Development

February 28, 2022

Bean, Kinney & Korman, PC Attn: Timothy Dugan 2311 Wilson Blvd., Suite 500 Arlington, VA 22201

Parsons Kenneth F and Phillips Karen E Trustees 14319 Dumfries Road Manassas, VA 20112

Re: 9522 Fairview Avenue, Manassas, VA 20110 Tax Map #: 100/01 00/ 202/ / ZAOI #2022-0031

Per our conversation on February 14, 2022 you have requested written determination to a question on the property listed above (hereinafter the "Property"). The following determination was based on the review of the information in your correspondence dated, February 9, 2022. In response, I provide the following information:

- 1. The parking requirement for an Educational Facility, Primary or Secondary, pursuant to section 130-204 of the City of Manassas Zoning Ordinance is: 1 space for each non-instructional staff and 1 space for every 20 students.
- 2. Based on the proposal for an Elementary School on the property with 2 non-instructional staff and 120 students a total 8 parking spaces would be required.
- 3. The parking requirement for a Day Care, pursuant to section 130-204 of the City of Manassas Zoning Ordinance is: 1 space per 250 Square Feet of Gross Building area.
- 4. Based on the proposal for a 17,898 square foot Day Care on the property a total of 72 parking spaces would be required.
- 5. However, the proposed Day Care includes several non-traditional spaces (larger corridors, movement rooms, and a "Kinderkitchen") which overlap with instructional space typically found in a conventional educational facility classroom.
- 6. Considering the parking standard applied to a Day Care use is typically applied to a traditional Day Care which includes predominantly conventional classroom space, it would be reasonable to evaluate the parking for the proposed Day Care based on the amount of conventional classroom space, excluding common areas such as movement rooms and "Kinderkitchen."
- 7. Based on the information submitted in your February 9, 2022 correspondence a Day Care with 9,500 square feet of conventional classroom space would require a total of 38 parking spaces. This calculation should be reflected as part of the SUP plan required for approval by City Council of a Day Care and Educational Facility in this zoning district.

I trust that this information, which reflects the status of the parcel on this date, addresses your inquiry. Unless otherwise stated, this verification shall supersede any prior zoning interpretation or verification for this property. If you have any questions relating to this information, please contact me at (703) 257-8223.

You have a right to appeal this decision of the Zoning Administrator within thirty days in accordance with Virginia Code §15.2-2311. This decision shall be final and unappealable if not appealed within thirty days. You may appeal by filing a notice of appeal, specifying the grounds thereof, with the Zoning Administrator at the address given in this letter. You can also obtain additional information regarding the filing of an appeal from the office of the Zoning Administrator. The fee for an appeal is \$500, plus the cost of advertising.

Sincerely,

Gregory J. Bokan, AICP Land Development Manager Zoning Administrator



CITY OF MANASSAS ARCHITECTURAL REVIEW BOARD CERTIFICATE OF APPROPRIATENESS Issued Date: April 19, 2022 Expires: April 19, 2024

ARB CASE #2022-0029

Owner(s):Kenneth and Karen ParsonsApplicant(s):Timothy DuganSite Address:9522 Fairview Ave.Tax Map #:101/01 00/202//

Motion: Mr. Hersch, Vice-Chair Carter Ayes: Mr. Hersch, Vice-Chair Carter, Chairperson Alten, Ms. Settle Nays: Ms. Brent, Mr. Pires, Mr. Porter

Approved Activity:

New Construction (Concept): The schematic design including the height, mass, scale, form, and placement of two new construction buildings. The elevation facing 9518 Fairview Avenue will have a gable roof as depicted in the attachments.

Demolition of addition to a Landmark Structure: A one-story wood-frame addition on the Carriage House will be demolished.

Jan Alten, Chairperson

MMlh

Christen Miller, Clerk

Attachments Enclosed: Site Layout and Elevations













0 50.



CITY OF MANASSAS ARCHITECTURAL REVIEW BOARD CERTIFICATE OF APPROPRIATENESS Issued Date: May 19, 2022 Expires: May 19, 2024

ARB CASE #2022-0029

Owner(s): Kenneth and Karen Parsons Applicant(s): **Timothy Dugan** Site Address: 9522 Fairview Ave. Tax Map #: 101/01 00/202//

Motion: First: Ms. Brent; Second: Vice-Chair Carter Ayes: Ms. Brent, Vice-Chair Carter, Chairperson Alten, Mr. Hersch, Mr. Pires, Mr. Porter, Ms. Settle

Approved Activity: Materials for New Construction Buildings

- 1. Brick: Glen Gery Georgian, and Belden Winter Mist.
- 2a. Simulated Wood Panel: Trespa Meteon Decorative High-Pressure in the color Elegant Oak with a matt finish.
- 2b. Trim: Trespa Meteon Decorative High-Pressure Laminate in the color Pure White with a matt finish.
- 2c. Colored Accent Panels: Trespa Meteon Decorative High-Pressure Laminate in the colors Royal Blue, Red Orange, and Lime Green with a matt finish.
- 3a. Double-hung Window: Pella 2/2, double-hung aluminum clad wood window in the color Iron Ore with a simulated divided light with spacer.
- 3b. Store-front windows: Aluminum-frame windows with the Trifab VersaGlaze 451T in a color to match Iron Ore.
- 3c. Doors: 500 Wide Stile Entrance aluminum-frame in a color to match Iron Ore.
- 3d. Curtainwall: Kaawneer 1600, 2 ½ aluminum-frame in a color to match Iron Ore.
- 4a. Glazing for windows and doors: Guardian Glass SN62 clear, insulated, low-e glass.
- 4b. Glazing for select windows: Guardian SunGuard Spandrel HT glazing in an opaque black color.
- 5. Roof: Snap-lock metal seam roof with a 1 ¾ inch seam in the color graphite.
- 6. Brick Mortar: Holcim Satin color

Jan-Alten, Chairperson

Christen Miller, Clerk

Attachments Enclosed: Material Details



9522 FAIRVIEW AVE

ARCHITECTURAL REVIEW BOARD MATERIALS

New Construction Only MANASSAS, VA

May 17, 2022







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APPROVED DESIGN ARB CASE #: 2022~0029 SIGNED: ______ DATE: ______ OFFICE OF COMMUNITY DEVELOPMENT

1. BRICK

- 2. SIMULATED WOOD PANELS + TRIM
- 3. WINDOWS
- 4. GLAZING
- 5. ROOFING



SAMPLES IN SUN



SAMPLES IN SHADE









M

1. BRICK

- Material: Modular Face Brick
- Basis-of-Design Product: Belden White Mist
- The brick proposed on the new construction was selected to compliment the existing red brick and avoid completing with the historic structures.






2. SINULATED WOOD PANELS & TRIN

2.A - SIMULATED WOOD PANELS

- Material: Decorative High-Pressure Laminate (DHPL)
- Basis-of-Design Product: Trespa Meteon; Elegant Oak
- The panels have a wood appearance that are also sustainable, durable, and weather resistant.
- The material will be cut in long, linear boards and installed a vertical pattern.





2.B - TRIM

- Material: Decorative High-Pressure Laminate (DHPL)
- Basis-of-Design Product: Trespa Meteon; Pure White
- The panels have a solid color finish that are also sustainable, durable, and weather resistant.

2.C - COLORED PANELS

- Material: Decorative High-Pressure Laminate (DHPL)
- Basis-of-Design Product: Trespa Meteon; Blue, Orange, Green
- The panels have a solid color finish that are also sustainable, durable, and weather resistant.









lumen

	C 1	Diffuse			Oblique			Specular		
Colour code	Colour name		*			*			*	
10500	Athens White	-	-	-	-	-	-	-	•	•
L0651	Italian Greige	-	-	•						
L1971	Iceland Grey	-	-	•						
L2151	London Grey	-	-	•	-	-	-	-	•	-
L2581	New York Grey	-	-	•	-	-	-	-	•	•
L3673	Olive Green	-	-	•						
L9000	Metropolis Black	-	-	•	-	-	-	-	-	-
With metallic e	effect									
LM0561	Roman Bronze	-	-	•	-	-	-			
LM0641	China Gold	-	-	•	-	-	-			
LM1055	Persian Copper	-	-	•	-	-	-			
LM2181	Siberian Platinum	-	-	•	-	-	-			
LM5101	Paris Silver	-			-	-	-			

TRESPA® METEON® LUMEN DIFFUSE, LUMEN OBLIQUE AND ALL LUMEN WITH METALLIC EFFECT (PRODUCTS WITH LM CODE) FEATURE A DIRECTIONAL COLOURED SURFACE. * WITH SATIN REVERSE.

METALLICS

			Satin			Rock*			
Colour code	Colour name								
M04.4.1	Titanium Silver	-	-	-	-	-	-		
M05.5.1	Titanium Bronze	-	-	-	-	-	-		
M05.6.1	Urban Brown	-	-	-	-	-	-		
M06.4.1	Amber	-	-	-	-	-	-		
M21.3.4	Azurite Blue	-	-	-	-	-	-		
M21.8.1	Graphite Grey	-	-	-	-	•	-		
M51.0.1	Aluminium Grey	•	-	-	-	-	-		
M51.0.2	Urban Grey	•	-	-	-	•	-		
M53.0.1	Copper Red	•	-	-	-		-		
M53.0.2	Copper Yellow			-	-				

TRESPA® METEON® METALLICS PANELS FEATURE A DIRECTIONAL COLOURED SURFACE.

* WITH SATIN REVERSE.

FOCUS

Cala	Diffuse		
de Colour name			
Brooklyn Classic	•	•	
Brooklyn Luna	•	•	
Brooklyn Antracite		•	
Chester Grey		•	
	Colour name Brooklyn Classic Brooklyn Luna Brooklyn Antracite Chester Grey	Colour name Dif Brooklyn Classic • Brooklyn Luna • Brooklyn Antracite • Chester Grey •	

	Cili	Diffuse		
Colour code	Colour name			
With metallic e	effect			
CM03.06	Tribeca Gold	•	•	
CM03.16	Tribeca Iron	•	•	
CM03.24	Tribeca Zinc	•	•	
CM09.03	Brooklyn Steel	•	•	
CM09.06	Brooklyn Bronze	•	•	
CM09.51	Brooklyn Aluminium	•	•	

TRESPA® METEON® FOCUS PANELS FEATURE A DIRECTIONAL COLOURED SURFACE.

APPROVED DESIGN ARB CASE #: __ SIGNED: 0/19/2022 OFFICE OF COMMUNITY DEVELOPMENT TYPES SINGLE SIDED DECORATIVE DECORATIVE SIDE WITH NON DECORATIVE BLACK REVERSE DOUBLE SIDED DECORATIVE SAME COLOUR FOR FRONT AND REVERSE SIDE OF THE PANEL VARITOP DECORATIVE SIDE WITH STANDARD WHITE DECORATIVE SATIN REVERSE (A03.0.0) DUOCOLOUR DIFFERENT COLOUR FOR FRONT AND REVERSE SIDE OF THE PANEL SIZES 2 3 1 3050 x 1530 mm (≈ 120 x 60 inch) 2 3650 x 1860 mm (≈ 143 x 73 inch) 3 4270 x 2130 mm (≈ 168 x 83 inch)* * THE LARGE SIZE OF THIS PANEL ALLOWS AN EFFICIENT MACHINING OF THE PRODUCT. THICKNESSES 6 mm (≈ 1/4 inch) 8 mm (≈ 5/16 inch) 10 mm (≈ 3/8 inch) 13 mm (≈ 1/2 inch)

NOTE

PLEASE BE AWARE THAT NOT ALL PRODUCT-FINISH COMBINATIONS ARE POSSIBLE. FOR AVAILABLE SHEET SIZES, THICKNESSES AND FINISHES IN YOUR COUNTRY, PLEASE CHECK **TRESPA.INFO** FOR THE DETAILED AND UP TO DATE DELIVERY PROGRAMME AND MATERIAL PROPERTY DATASHEET. ALTERNATIVELY YOU CAN USE THE PRODUCT SELECTOR ON **TRESPA.COM** (AFTER CHOOSING THE COUNTRY WHERE THE PROJECT IS LOCATED).

ORDER SAMPLES AT TRESPA.COM

FINISHES



TRESPA® METEON® WOOD AND NATURALS DECORS

APPROVED DESIGN ARB CASE #: 2022-0029 SIGNED: 0000000 DATE: 5/19/2022 OFFICE OF COMMUNITY DEVELOPMENT



METEON® WOOD

NATURE AT ITS BEST

Wood structures that tell a story. Tones that either breathe more warmth or convey coolness. And endless possibilities that bring natural textures (or finishes) to life. The Trespa^{*} Meteon^{*} Wood collection is nature at its best. Providing you with wood looks that are indistinguishable from the real thing, this collection brings your wood ideas perfectly to life. From fresh contemporary designs to classic weathered looks – the distinctive designs make it possible to make details more eye-catching and façades even more standout.

With the Trespa[®] Meteon[®] Wood collection, you can unleash your ideas in new ways, while creating tomorrow's wood-inspired designs. For a timeless appearance.



DISCOVER THE TRESPA® METEON® WOOD COLLECTION AND REIMAGINE YOUR NEXT PROJECT





31 WINDOWS + DOORS

3.A - DOUBLE-HUNG WINDOWS

APPROVED DESIGN ARB CASE #: 2022~0029 SIGNED: 00000000 DATE: 5/19/2022 OFFICE OF COMMUNITY DEVELOPMENT

- Material: Aluminum-Clad Wood Windows
- Basis-of-Design Product: Pella, Lifestyle Series
- Finish: Iron Ore
- 2/2 Double-hung windows are proposed at all punched openings.





Pella[®] Lifestyle Series

Clad/Wood

#1 performing wood window and patio door for the combination of energy, sound and value.¹

Triple-pane casement



Dual-pane casement



Available in these window and patio door styles:5



Performance redefined

You don't have to compromise on any aspect of performance. Available performance solutions offer an unbeatable combination of energy efficiency, sound control and value.¹

• ENERGY STAR® certified²

Pella products offer energy-efficient options that will meet or exceed ENERGY STAR guidelines in all 50 states. Pella Lifestyle Series products with triple-pane glass have been awarded ENERGY STAR Most Efficient Mark in 2022.

• Enhanced sound control

Our patented, triple-pane design with Advanced Low-E glass allows for mixed glass thickness for enhanced sound dampening resulting in an average 52% noise reduction versus single-pane windows.³

• Popular features and options

Low-maintenance aluminum-clad exteriors. Factory prefinish with a choice of several paints and stains, or choose primed or unfinished. Several grille types and patterns and high-transparency screens are also available.

• Intentional design for improved durability

Intentional jamb/sill design helps seal the end grain of the wood and elevates it off the rough opening, reducing the potential for moisture.

• Durable 3-way corner joint

Three-way corner joints are made up of mortise-and-tenon, metal fasteners and commercial adhesive for added strength and durability.

• Low-maintenance exteriors

Aluminum-clad exteriors with EnduraClad® finish resists fading and chalking. It is applied in an overlapping fashion for exceptional protection.

• Exclusive wood protection

Pella's exclusive EnduraGuard® wood protection is applied after the pieces have been cut and milled, but prior to final assembly. It provides advanced protection against the effects of moisture, decay, stains from mold and mildew – as well as termite damage.

• Best limited lifetime warranty⁴

Pella Lifestyle Series products are covered by the best limited lifetime warranty in the industry for wood windows and patio doors.⁴

• Testing beyond requirements

At Pella, our products are tested beyond requirements to help ensure they have long-lasting performance and reduce call-backs for you.

Product Specifications

	nation nation nation nation					Performance Values			
Window & Patio Door Styles	Win. Width	Min. Height	Max. Width	Max. Height	Performance Class & Grade	U-Factor	SHGC	STC	
Awning Dual-pane vent	21"	17"	59"	59"	LC30	0.25-0.35	0.19-0.51	25-28	
Awning Triple-pane vent	21"	17"	59"	59"	LC25-CW50	0.12-0.19	0.24-0.56	31-37	
Casement Dual-pane vent	17"	17"	35"	73"	LC30-LC50	0.25-0.35	0.19-0.51	25-31	
Casement Triple-pane vent	17"	17"	35"	73"	R20-CW50	0.20-0.25	0.17-0.46	31-37	
Fixed Casement Dual-pane	17"	17"	73"	73"	LC30-LC50	0.23-0.50	0.19-0.66	29-32	
Fixed Casement Triple-pane	17"	17"	73"	73"	R20-CW50	0.19-0.24	0.17-0.46	33-37	
Double-Hung Dual-pane vent	21"	35"	41.5"	77"	LC30-LC50	0.19-0.66	0.20-0.56	27-31	
Hinged Patio Door Dual-pane single door	30"	80"	38"	96"	LC50	0.25-0.29	0.18-0.48	31	
Hinged Patio Door Triple-pane single door	30"	80"	38"	96"	LC55	0.22-0.26	0.14-0.38	34-36	
Hinged Patio Door Dual-pane double door	60"	80"	75"	96"	LC50	0.25-0.29	0.18-0.48	30-32	
Hinged Patio Door Triple-pane double door	50"	80"	75"	96"	LC55	0.22-0.26	0.14-0.38	34-36	
Sliding Patio Door Dual-pane single-door fixed (O)	31"	80"	49"	96"	CW50	0.24-0.33	0.18-0.51	27	
Sliding Patio Door Triple-pane single-door fixed (O)	31"	80"	49"	96"	CW60	0.21-0.27	0.17-0.45	33-36	
Sliding Patio Door Dual-pane double-door vent (OX or XO)	60"	80"	96"	96"	R20-LC50	0.24-0.33	0.18-0.51	29-32	
Sliding Patio Door Triple-pane double-door vent (OX or XO)	60"	80"	96"	96"	R25-LC60	0.21-0.27	0.17-0.45	33-36	

Colors

Prefinished Pine Interior Colors	We can pre also availab	We can prefinish pine in your choice of se also available.				
	White	Bright White	Linen White			
Aluminum-Clad Exterior Colors	Our low-maintenance EnduraClad® ext Seacoast EnduraClad protective finish					
	Black	White	Brown			
	Iron Ore	Portobello	Putty			
Integrated Blinds ⁶	Raise blinds up for an unobstructed view oblinds are available manual or motorized					
		-	-			

White

Window sizes available in 1/4" increments

rs. For more information regarding performance, visit pella.com/performance. For more information regarding frame and installation types, visit installpella.com.

Window Hardware



Satin Nickel Satin Brass

Patio Door Hardware

Essential Collection Elevate your style and transform a home with elegant selections.

Sliding Patio

Door Handle



Hinged Patio Door Handle



Added Peace of Mind

Integrated Security
Sensors

Integrated

Shades⁶

Integrated wireless security sensors maintain aesthetics, streamline security installation and ensure no warranty loss is caused by post-installation drilling. Sensors can be monitored via the free Pella® Insynctive® App and are compatible with major security panel systems.⁷ For more information, go to connectpella.com.

Our best integrated fabric shades feature a white exterior fabric for a uniform look from the street. Integrated and accessible shades are available manual or motorized with Pella Insynctive technology.

Light-Filtering: White Silver Maize

Poplar

White

Bisaue

Haven't landed on the final blind or shade color selection? No problem. With our patented triple-pane design, you and your customer can make those decisions later in the schedule. Our triple-pane products come with all of the hardware to add a blind or shade straight from the factory or at a later time in the building or remodeling process.

several paint and stain colors. Unfinished or primed and ready-to-paint are





Stain





erior finish resists fading and helps protect windows and patio doors for years. for coastal projects with high salt exposure is also available.



w or tilt to let in just the right amount of light. Our best integrated and accessible ed with Pella® Insynctive® technology.











Performance Packages

To make things easier, we've created performance packages.

Performance solutions offer an unbeatable combination of energy efficiency, sound control and value.¹ Create room-by-room solutions with the upgraded triple-pane glass design.

All values below are averages compared with single-pane windows.



Pella® Lifestyle Series offers products awarded ENERGY STAR® Most Efficient for 2022.²

Base	Performance 71% More Energy Efficient [®] + 34% Noise Reduction ³	Sound Control 52% Noise Reduction ³	Energy Efficiency 83% More Energy Efficient ⁸	Ultimate Performance 79% More Energy Efficient ⁸ + 52% Noise Reduction ³
Low-E Clear	Low-E Clear Clear	Low-E Clear Clear	Low-E Hard Coat Clear	Low-E Hard Coat Clear
Advanced Low-E	Advanced Low-E SunDefense Low-E or NaturalSun Low-E	Advanced Low-E Sound-reduction glazing	AdvancedComfort	AdvancedComfort Sound-reduction glazing
Two panes of insulating, energy-efficient glass and our most popular features and options.	A triple-pane glass design for a combination of both improved energy efficiency and sound performance.	Triple-pane glass design featuring mixed glass thicknesses for enhanced sound dampering.	A triple-pane glass design with upgraded AdvancedComfort Low-E glass for enhanced energy efficiency.	A triple-pane glass design featuring mixed glass thicknesses with upgraded AdvancedComfort Low-E glass for enhanced energy efficiency.

Patented triple-pane glass design gives flexibility to add integrated blinds or shades without impacting performance.

Grilles

Choose the look of true divided light, removable roomside grilles or make cleaning easier by selecting grilles-between-the-glass.





The Best Limited Lifetime Warranty in the Industry

We know your reputation matters and you stake your reputation on quality, dependable products. That's why we have the best limited lifetime warranty in the industry for wood windows and patio doors.⁴

- ¹ Performance solutions require upgrades to triple-pane, AdvancedComfort Low-E and mixed glass thickness. Based on comparing product quotes and published STC/OITC and U-Factor ratings of leading national wood window and patio door brands.
- ² Some Pella products may not meet ENERGY STAR certification in Canada. For more information, contact your local Pella sales representative or go to nrcan.gc.ca/energy/products/categories/fenestration/13739.
- ³ Reduction in sound based on OITC ratings of Pella Lifestyle Series windows with respective performance package compared to a single-pane wood or vinyl window with an OITC of 19. Calculated by using the sound transmission loss values in the 80 to 4000 Hz range as measured in accordance with ASTM E-90(09). Actual results may vary
- ⁴ Based on comparing written limited warranties of leading national wood window and wood patio door brands. See written limited warranty for details, including exceptions and limitations, at pella.com/warranty.
- ⁵ Double-hung windows available in dual-pane only.

- Available with triple-pane products only.
- $^7\,\,$ Requires the Insynctive App on a smart device, an Insynctive Bridge and a wireless home router with internet connection.
- ⁸ Window energy efficiency calculated in a computer simulation using RESFEN 6.0 default parameters for a 2000 sq. foot new construction single-story home when Pella Lifestyle Series windows with the respective performance package are compared to a single-pane wood or vinyl window. The energy efficiency and actual savings will vary by location. The average window energy efficiency is based on a national average of 94 modeled cities across the country and weighting based on population. For more details see pella.com/methodology.
- $^{\circ}\,$ Appearance of exterior grille color may vary depending on the Low-E insulating glass selection.



3.B - STOREFRONT WINDOWS

- Material: Aluminum-Framed Storefront
- Basis-of-Design Product: Kawneer Trifab 451T, 2" sight-line
- Storefront window system is proposed at the large window openings.
- The aluminum will be painted with the manufacturer's fluoropolymer finish in a custom gray color to match the double-hung windows.



APPROVED DESIGN ARB CASE #: _______ SIGNED: ______ DATE: ______ OFFICE OF COMMUNITY DEVELOPMENT

City of Manassas ARB Materials Submission 9522 Fairview Ave

TRIFAB[®] VG (VERSAGLAZE[®]) TRIFAB[®] VG 450, 451 & 451T (THERMAL) FRAMING SYSTEMS & TRIFAB[®] 451UT (ULTRA THERMAL) FRAMING SYSTEM

Design + Performance Versatility with Unmatched Fabrication Flexibility





Trifab[®] VersaGlaze[®] is built on the proven and successful Trifab[®] platform – with all the versatility its name implies. There are enough framing system choices, fabrication methods, design options and performance levels to please the most discerning building owner, architect and installer. The 4.5" depth Trifab[®] VersaGlaze[®] Framing System family is available with non-thermal, thermal and ultra-thermal performance levels. The ultra-thermal Trifab[®] 451UT Framing System, is designed for the most demanding thermal performance and employs a dual Isolock[®] thermal break.

AESTHETICS

Trifab[®] VersaGlaze[®] Framing Systems offer designers a choice of front-, center-, back- or multi-plane glass applications. Structural silicone

glazing (SSG) and weatherseal glazing options further expand designers' choices, allowing for a greater range of possibilities for specific project requirements and architectural styles. All systems have a 4-1/2" frame depth; Trifab[®] VersaGlaze[®] 450 has 1-3/4" sightlines, while Trifab[®] VersaGlaze[®] 451/451T and Trifab[®] 451UT have 2" sightlines.

With seamless incorporation of Kawneer entrances or windows, including GLASSvent[®] visually frameless ventilators, Trifab[®] framing can be used on almost any project. These framing systems can also be packaged with Kawneer curtain walls and overhead glazing, thereby providing a full range of proven, and tested, quality products for the owner, architect and installer from a single-source supplier.

ECONOMY

Trifab® VersaGlaze® 450/451/451T/451UT Framing Systems offer a variety of fabrication choices to suit your project:

- Screw Spline for economical continuous runs utilizing two-piece vertical members that provide the option to pre-assemble units with controlled shop labor costs and smaller field crews for handling and installation. (available for all systems)
- Shear Block for punched openings or continuous runs using tubular moldings with shear block clips that provide tight joints for transporting large pre-assembled multi-lite units. (available for 450/451/451T systems)
- Stick for fast, easy field fabrication. Field measurements and material cuts can be done when metal is on the jobsite. (available for 450/451/451T systems)
- Pre-glazed The combination of screw spline construction with pre-glazing in the shop accelerates installation and reduces field labor time while minimizing disruption to the surrounding area or existing tenants. Making it an exceptional choice for new or retrofit applications, particularly in urban areas or where space is limited. (available for 451/451T/451UT framing)



Brighton Landing Cambridge, Massachusetts ARCHITECT ADD Inc., Cambridge, Massachusetts GLAZING CONTRACTOR Ipswich Bay Glass Company, Inc., Rowley, Massachusetts PHOTOGRAPHER © Gordon Schenck, Jr.

All systems can be flush glazed from either the inside or outside. The weatherseal option provides an alternative to SSG vertical mullions for Trifab® VersaGlaze® 450/451/451T. This ABS/ASA rigid polymer extrusion allows complete inside glazing and creates a flush glass appearance on the building exterior without the added labor of scaffolding or swing stages. Additionally, high-performance flashing options are engineered to eliminate perimeter sill fasteners and associated blind seals.

FOR THE FINISHING TOUCH

Architectural Class I anodized aluminum and painted finishes in fluoropolymer (AAMA 2605) and solvent-free powder coatings (AAMA 2604) offer a variety of color choices.

PERFORMANCE

Kawneer's Isolock® thermal break technology creates a composite section, prevents dry shrinkage and is available on Trifab® VersaGlaze® 451T. For even greater thermal performance, a dual Isolock® thermal break is used on Trifab® 451UT.



Trifab® 451UT uses a dual Isolock® thermal break (right) and features a new highperformance sill design, which incorporates a screw-applied end dam (left), ensuring positive engagement and tight joints between the sill flashing and end dam.

U-factor, CRF values and STC ratings for Trifab® framing systems vary depending upon the glass plane application. Project-specific U-factors can be determined for each individual project. (See the Kawneer Architectural Manual or Kawneer.com for additional information.)

Thermal simulations showing temperature variations from exterior/cold side to interior/warm side.







Trifab[®] VersaGlaze[®] 451

11.4" 19.75 281° 364° 448° 531° 615° 698° F

WARM

PERFORMANCE TEST STANDARDS

COLD

Air Infiltration	ASTM E283
Water	AAMA 501, ASTM E331
Structural	ASTM E330
Thermal	AAMA 1503
Thermal Break	AAMA 505, AAMA TIR-A8
Acoustical	AAMA 1801, ASTM E1425





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Kawneer Company, Inc. Technology Park / Atlanta 555 Guthridge Court Norcross, GA 30092

770 449 5555 kawneer.com



ARCHITECTURAL SYSTEMS | ENTRANCES + FRAMING | CURTAIN WALLS | WINDOWS

3.C - STOREFRONT ENTRANCE DOORS



- Material: Aluminum-Framed Storefront Doors
- Basis-of-Design Product: 500 Wide Stile Entrance
- Storefront doors are proposed at all ground level entrances, except where shown as solid hollow metal doors on the west elevation.
- The aluminum will be painted with the manufacturer's fluoropolymer finish in a custom gray color to match the double-hung windows.





190, 350 AND 500 STANDARD ENTRANCES

Single-Source Packages Generate Versatile First Impressions





Tough yet attractive, Kawneer's Standard Entrances are designed as a single-source package of door, door frame and hardware that is easily adaptable to custom requirements. Designed to complement new or remodel construction as well as modern or traditional architecture, they are engineered, constructed and tested to make a good first impression while withstanding the rigors of constant use by occupants and visitors.

PERFORMANCE

To resist both lever arm and torsion forces that constantly act on any door, all three entrances feature welded corner construction with Sigma deep penetration and fillet welds plus mechanical fastenings at each corner – a total of 16 welds per door. Each door corner comes with a limited lifetime warranty, good for the life of the door under normal use. It is transferable from building owner to owner and is in addition to the standard two-year warranty covering material and workmanship of each Kawneer door.





- 1. Thermoplastic elastomer weatherstrip in blade stop of frame jambs, header or
- transom bar. 2. Integral polymeric fin attached to adjustable astragal, creating an air barrier between pairs
- of doors. 3. Optional surfaceapplied bottom weatherstrip with flexible blade gasket. Extruded raised lip on threshold to provide continuous contact for
- 4. Standard 1/4" beveled glass stops to sheet water and dirt off without leaving residue

bottom weatherstrip.

5. Available in all finishes offered by Kawneer.

GENERAL

- Heights vary up to 10'; widths range from approximately 3' to 4'
- Door frame face widths range to a maximum of 4", while depths range to 6"
- Door operation is single- or double-acting with maximum security locks or touch bar panics standard
- Architect's classic 1" round, bent bar push/pull hardware is available in various finishes and sizes
- Infills range from 1/4" to 1"

FOR THE FINISHING TOUCH

Architectural Class I anodized aluminum finishes are available in clear and Permanodic® color choices.

Painted finishes, including fluoropolymer, that meet AAMA 2605 are offered in many standard choices and an unlimited number of specially designed colors.

Solvent-free powder coatings add the "green" element with high performance, durability and scratch resistance that meet the standards of AAMA 2604.



Kawneer's bulb neoprene weatherstripping forms a positive seal around the door frame and provides a substantial reduction in air infiltration, resulting in improved comfort and economies in heating and cooling costs. The system is wear- and temperature-resistant and replaces conventional weatherproofing. The bottom weatherstrip at the interior contains a flexible blade gasket to meet and contact the threshold, enhancing the air and water infiltration performance characteristics.

190 NARROW STILE ENTRANCE

- Is engineered for moderate traffic in applications such as stores, offices and apartment buildings
- Vertical stile measures 2-1/8", top rail 2-1/4" and bottom rail 3-7/8"
- Results in a slim look that meets virtually all construction requirements

350 MEDIUM STILE ENTRANCE

- Provides extra strength for applications such as schools, institutions and other high-traffic applications
- Vertical stiles and top rails measure 3-1/2"
- Bottom rail measures 6-1/2" for extra durability

500 WIDE STILE ENTRANCE

- Creates a monumental visual statement for applications such as banks, libraries and public buildings
- Vertical stiles and top rail measures 5"; bottom rail measures 6-1/2"
- Results in superior strength for buildings experiencing heavy traffic conditions



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Kawneer Company, Inc. Technology Park / Atlanta 555 Guthridge Court Norcross, GA 30092

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ARCHITECTURAL SYSTEMS | ENTRANCES + FRAMING | CURTAIN WALLS | WINDOWS

3.D - CURTAINWALL

APPROVED DESIGN ARB CASE #: _______ SIGNED:_______ DATE: _______ OFFICE OF COMMUNITY DEVELOPMENT

- Material: Aluminum-Framed Curtainwall
- Basis-of-Design Product: Kawneer 1600, 2 1/2" sightline
- The aluminum will be painted with the manufacturer's fluoropolymer finish in a custom gray color to match the double-hung windows.





IMPOSING STATEMENTS USED TOGETHER OR INDEPENDENTLY

1600 Wall System®1 / System®2 Curtain Wall

Building on the proven success of Kawneer's 1600 Wall System[®] that set the standard for curtain wall engineering, 1600 Wall System[®]1 Curtain Wall and 1600 Wall System[®]2 Curtain Wall provide reliability with versatile features.

Both are stick-fabricated, pressure-glazed curtain walls for low- to mid-rise applications and are designed to be used independently or as an integrated system to provide visual impact for almost any type of building.

- 1600 Wall System[®]1 is an outside-glazed, captured curtain wall.
- 1600 Wall System[®]2 is a structural silicone glazed (SSG) curtain wall.





APPROVED DESIGN
ARB CASE #: 2022-0029
SIGNED: Conden
DATE: 5/19/2022
OFFICE OF COMMUNITY DEVELOPMENT

4. GLAZING

4.A - CLEAR VISION GLAZING

APPROVED DESIGN ARB CASE #: 2022-0029 SIGNED: CMULL-DATE: 5/19/2022 OFFICE OF COMMUNITY DEVELOPMENT

- Material: Clear, insulated, low-e glass
- Basis-of-Design Product: Guardian Glass SN62
- Clear vision glazing will be used at all double hung-windows and at storefront system locations adjacent to interior occupied spaces as noted on the elevations.





SunGuard® eXtraSelective

SNX 62/27

More natural light, less heat gain, greater energy savings

Let natural light do what it does best with SunGuard SNX 62/27 coated glass. SNX 62/27 coated glass product offers the highest light-tosolar-gain ratio of all SunGuard coated glass. This triple silver coating is an excellent option for daylighting—making it easy to create more inviting spaces, increase occupant comfort and help achieve LEED® credits. Its neutral/green appearance stays true to your design vision while helping to gain the energy-saving performance you desire.

Product Information

Substrate Base Glass/Thickness	Clear Float/5-12 mm Guardian UltraClear/6-10 mm	Guardian CrystalBlue/6 mm Guardian CrystalGray/6-10 mm	Gray/6 mm Green/6 mm			
Applications	Facades / Windows / Doors / Curtain Walls / Roof	s / Skylights				
Manufacturing options	Tempered / Laminated (not embedded) / Heat Soaked / Bent / Heat Strengthened / Annealed					
Recommended Coating Positions	Double glazed: #2 or #3 surface (restrictions apply) Triple Glazed: #2 surface					
Maximum Size	130"x240"					
Edge Deletion	Yes					
Glass type	Low E					
Glass functions	Solar Control U-Value Benefit					
Fabrication options	Must be used in Insulating Glass Units					
Appearance	Clear					



APPROVED DESIGN ARB CASE #: 2022-0029 SIGNED: CMCLu-DATE: 5/19/2022 OFFICE OF COMMUNITY DEVELOPMENT

4.B - SPANDREL GLAZING

- Material: Opaque, insulated, low-e glass with ceramic frit coating
- Basis-of-Design Product: Guardian SunGuard Spandrel HT
- Color: Black
- Spandrel glass will be used in limited areas to block plenum spaces or structure from view when the storefront system extends above or below occupied spaces.
- The ceramic frit will be a black color applied to the #2 surface of the insulated glazing unit.



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GLAZ-TECH INDUSTRIES INTRODUCES SUNGUARD® SPANDREL HT

COATED SPANDREL STOCK SHEETS THAT ARE EASILY OUT TO SIZE & HEAT TREATED NOW AVAILABLE IN WARM GRAY, BLACK AND WHITE



Heat-Treatable Back-Painted Glass from Guardian

Glaz-Tech Industries offers a diverse and alternate solution for exterior applications with Guardian's **SunGuard® Spandrel HT**. Guardian's proprietary coatings allow for a more durable, uniform and ideal application where structural concealment is required. Available in three commonly specified colors, SunGuard Spandrel HT is perfect for harmonizing or giving your project a desired contrast against the vision glass area.

> GLAZ-TECH INDUSTRIES DESIGN THROUGH GLASS

C

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COMPLEMENTING THE VISION GLASS

For best results complementing or harmonizing appearance (color & reflection), the insulating glazing make-up should remain consistent (the same exterior glass color, SunGuard coating, etc.) and substitute the inboard lite with warm gray SunGuard Spandrel HT. - Guardian Industries

Characteristics of SunGuard® Spandrel HT

- · Proprietary coated stock sheets are easy to cut into final sizes.
- Available standard stock sheet size, 96" x 130" for ease of fabrication.
- · Colors available: Warm Gray, Black and White.
- · Superior durability to conventional frits prior to heat treating.
- · Coating permanently bonds to glass and has the characteristics of ceramic frit after post heat treatment.
- · Conceals structural building components such as columns, floors, HVAC systems, electrical wiring and plumbing.
- . Can be utilized for monolithic applications (#2 surface) or for insulating glazing units.
- · Heat-treatment fuses coating to the glass surface while reducing the risk of thermal stress breakage.
- Spandrel HT is not intended for interior applications.

Recommended Applications

- Storefronts
- Wall Cladding
- Custom Color Applications
- Hide Structural Utilities
- Curtain Walls
- Nonvision Areas
- Commercial Fixed Windows
- Security for Fallout Protection

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5.A - STANDING SEAM METAL ROOF

- Material: Pre-finished Aluminum Standing Seam Metal Roofing
- Basis-of-Design Product: Pac-Clad Snap-Loc
- Color: Graphite
- Standing seam metal roof will be used on all steep slope gable roofs (greater than 2"/Ft)



Standard Colors

	PATINA GREEN	▲	TEAL	HEMLOCK GREEN		FOREST GREEN		HARTFORD GREEN
\mathcal{A}	Y Y EVERGREEN	_^	HUNTER GREEN	ARCADIA GREEN		MILITARY BLUE		BERKSHIRE BLUE
۲	GRAPHITE	4	CHARCOAL	INTERSTATE BLUE		SLATE BLUE		AWARD BLUE
BL	MATTE BLACK ACK STEEL ALUMINUM		DARK BRONZE	BURNISHED SLATE		AGED BRONZE		MEDIUM BRONZE
	MANSARD BROWN		BURGUNDY	TERRA COTTA		CARDINAL RED		COLONIAL RED
	MIDNIGHT BRONZE		MUSKET GRAY	SIERRA TAN	A	SANDSTONE	•	ALMOND
	SLATE GRAY		CITYSCAPE	GRANITE		STONE WHITE		BONE WHITE



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SNAP-CLAD PANEL

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PRODUCT FEATURES

- Architectural/structural panel
- Factory-applied sealant available
- Continuous interlock
- Labor-saving one-piece design
- Pencil ribs upon request
- Striations upon request
- Factory eave notching available
- ▶ 30-year-non-prorated finish warranty
- Maximum factory-produced panel length is 64' (check w/factory for longer lengths)
- Weathertightness warranty available

- ▶ 43 stocked colors (24 gauge steel)
- 16 Stocked colors (22 gauge steel)
- ▶ 36 stocked colors (.032 aluminum)
- 22 stocked colors (.040 aluminum)
- Panels available in Galvalume Plus

UL CLASSIFICATION

- ▶ UL-580 Class 90 wind uplift
- UL-1897 wind uplift
- UL-790 Class A fire rated
- ▶ UL-263 fire resistance rated
- UL-2218 impact resistance rated

- UL-90 rated aluminum panel up to 16" O.C.
- UL-90 rated steel panel up to 18" O.C.

ASTM TESTS

- ASTM E1592 tested
- ASTM E283/1680 tested
- ASTM E331/1646 tested

FLORIDA BUILDING & MIAMI-DADE PRODUCT APPROVALS

Please refer to pac-clad.com or your local factory for specific product approval numbers for Snap-Clad.

Note: UL 90 is available on steel panels up to 18" on center, and on aluminum panels up to 16" on center.



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Brick Mortar:







ARCHITECTURAL REVIEW BOARD SUBMISSION

APPROVED DESIGN	
ARB CASE #: 2022-0029	
SIGNED:	
DAIE: 5/19/2022	
OFFICE OF COMMUNITY DEVELOPMENT	

ARCHITECTURAL SITE PLAN ENGEL EDUCATION GROUP FEASIBILITY STUDY 9522 FAIRVIEW AVE, MANASSAS, VA 20110





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ARCHITECTURAL REVIEW BOARD SUBMISSION

ELEVATION LEGEND




Traffic Impact Study

Edugenius Development

City of Manassas, Virginia



July 20, 2022

Prepared for:

Edugenius US Real Estate Group LLC 2319 S. Joyce Street Arlington, Virginia 22202



Prepared by:



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- Appendix B Raw Traffic Data and City-Provided Timing and Layout Sheets
- Appendix C VDOT Crash Data
- Appendix D HCM Level of Service Definitions
- Appendix E Intersection Capacity Analysis Worksheets 2022 Existing Conditions
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- Appendix H Peer Child Care Center Site Data

Executive Summary

Purpose of Report and Study Objectives

This report presents the findings of a Traffic Impact Study (TIS) conducted for the proposed Edugenius Development (the Site / the Development / the Property), situated in the City of Manassas (the City), Virginia.

This TIS was conducted in conjunction with the guidelines presented in the City of Manassas <u>Design and Construction Standards</u> <u>Manual</u> (DCSM). The document was prepared in accordance with best professional practices and standards that assess the impact of a proposed development on the transportation system. Traffic operational analyses, as presented in this TIS, involves the evaluation of anticipated roadway conditions with and without the proposed development. The analysis assists public officials and developers to balance the interrelations between efficient traffic movements with necessary lane access.

Based on the analysis, the proposed Edugenius Development is anticipated to have minimal impact on delays and queues at the existing study intersections, and no mitigations would be required per the analysis results. The following major conclusions are discussed below:

Site Location and Study Area

The Edugenius Development is proposed to be located along the western edge of Fairview Avenue (Route 4353) and northern edge of Wesley Avenue, south of Signal Hill Road (City Route 8) within the City of Manassas, Virginia. The vehicular study area includes five (5) existing intersections and two (2) future intersections.

Description of Proposed Development

The Edugenius Development is proposed to be situated on an approximately 3.3-acre single parcel of land, which can be identified on Manassas City GIS with the following Tax Map ID #: 100-01-00-202. The site is currently zoned as R-1 (Low Density, Single-Family Residential), with a Comprehensive Plan Land Use of Downtown.

The Applicant is proposing to redevelop the parcel with an approximately 120-student capacity elementary school and 125student capacity day care / childcare facility. The Edugenius Development in full is anticipated to be constructed and in operation by 2027.

Principal Findings, Conclusions, and Proposed Mitigations

Discussions regarding the study assumptions and relevant background information occurred with the City of Manassas staff during May, June and July, 2022, and a scoping document was agreed to. A copy of the agreed-upon scoping document is included in Appendix A.

The analysis presented in this report supports the following assumptions and findings:

Analysis Components

- As determined based on discussions with City staff, a growth rate of one-percent (1.0%) per year was applied to all
 movements at the intersection of Fairview Avenue (Route 4353) at Wellington Road (Route 4352), and propagated as
 mainline through movements at subsequent study intersections along the road network where applicable, to account for a
 regional increase in background traffic.
- The total Site is anticipated to generate approximately 152 new trips during the AM peak hour, 80 new trips during the PM peak hour, and 818 new daily trips on a typical weekday, after reductions.

Non-SOV Elements

• Various biking infrastructure exists along Tudor Lane, Signal Hill Road, and Richmond Avenue within the vicinity of the site; a shared use path exists along Wellington Road.

- No bus routes currently provide service within the vicinity of the site.
- The site is located within a 0.55-mile radius of the Manassas Train Station, which serves the Virginia Railway Express (VRE) and Amtrak, and within a 3.0-mile radius of the Manassas Regional Airport (HEF).

Infrastructure

• Currently, it is anticipated that the Edugenius Development will be accessed via one full-movement entrance along Wesley Avenue and one full-movement entrance along with one-partial movement entrance along Fairview Avenue.

Analysis Results

- The proposed Edugenius Development has minimal impact on delays and queues at the existing intersections.
- No mitigations are required as per the analysis results.
- One (1) existing study intersection (Fairview Avenue at Richmond Avenue) has one approach that operates at unacceptable levels of service (LOS) during one of the existing peak hours (PM peak hour only). The same approach is anticipated to continue to operate similarly at unacceptable LOS under all scenarios during the same peak hour.
- With the Edugenius Development in place (built and in operation), the LOS and delays are anticipated to remain similar
 as compared to future conditions without development; furthermore, with the addition of the Edugenius Development
 traffic, the increase in 95th percentile queue lengths is anticipated to be two standard car lengths (25 feet each) or less
 during either peak hour at any of the applicable lane groups of the study intersections, as compared to future conditions
 without development.
- The northbound through/right lane group of the signalized study intersection of Fairview Avenue at Manassas Police Department Entrance / Signal Hill Road has existing 95th percentile queue lengths that would spill past the proposed site entrances on Fairview Avenue; this is an existing condition and as noted above, the Edugenius Development traffic is anticipated to add only two standard car lengths or less during either peak hour as compared to future conditions without development. In addition, users of the proposed Development would have an alternative access point along Wesley Avenue with the ability to accommodate additional volumes.
- The remaining 95th percentile queue lengths are anticipated to be accommodated within the various storage bays under all scenarios.
- Based on a comparison with the number of parking spaces for peer child care center sites in the City of Manassas, the
 parking rate for the child care center portion of the proposed Edugenius Development, whether from a square footage (for
 conventional classroom space) or capacity perspective, is consistent with the parking rates for other peer child care center
 sites throughout the City, and would provide a practical and adequate amount of parking spaces.

Overall Conclusion

Based on the capacity and queueing analysis results, the proposed Edugenius Development will not have a substantial impact to the surrounding transportation and roadway network.

Introduction

This report presents the findings of a Traffic Impact Study (TIS) conducted for the proposed Edugenius Development (the Site / the Development / the Property), situated in the City of Manassas (the City), Virginia.

The Applicant is proposing to redevelop the site with an approximately 120-student capacity elementary school and 125-student capacity day care / childcare facility. The Edugenius Development in full is anticipated to be constructed and in operation by 2027.

The following tasks were completed as part of this study effort:

- Discussions regarding the study assumptions and relevant background information occurred with the City of Manassas staff during May, June and July, 2022, and scoping document was agreed to. A copy of the agreed-upon scoping document is included in Appendix A.
- Existing conditions were observed in the field to verify roadway geometry, pedestrian and bicycle infrastructure, and traffic flow characteristics.
- Traffic counts were collected at the existing study intersections on Tuesday, May 24, 2022, during the weekday morning and afternoon peak periods, from the hours of 6:00 AM to 9:00 AM and 4:00 PM to 7:00 PM. These counts were cross verified with 2016 counts at the same intersections and were determined to be consistent with pre-Covid conditions.
- The anticipated build-out year of the Edugenius Development was projected to be 2027 (as ultimately determined following distribution of the scoping document), as such 2027 future conditions were analyzed.
- The Future Conditions without Development scenario was projected based on the existing traffic volumes and an inherent growth to account for regional growth on the roadway network, as determined based on discussions with City staff.
 Following distribution of the scoping document, one (1) background development – arising from the conversion of the existing City of Manassas Police Department building into City government offices – was identified for inclusion in the study per City staff.
- Proposed site traffic volumes were derived based on the methodology outlined in the Institute of Transportation Engineers' (ITE) <u>Trip Generation Manual</u>, 11th Edition, publication, and were assigned to the road network based on the agreed upon direction of approach discussed during the aforementioned scoping discussions.
- The Future Conditions with Development scenario was projected based on the existing traffic volumes, regional growth, one (1) background development arising from the conversion of the existing City of Manassas Police Department building into City government offices (as identified for inclusion following distribution of the scoping document), and plans for the proposed Edugenius Development and surrounding vicinity.
- Intersection capacity and queueing analyses were performed for the identified study intersections for the 2022 Existing Conditions, 2027 Future Conditions without Development, and 2027 Future Conditions with Development scenarios during the weekday morning (AM), and weekday afternoon (PM) peak hours.
- Intersection capacity and queuing analyses were performed using Synchro, version 10, with results based on the Transportation Research Board's (TRB) <u>Highway Capacity Manual</u> (HCM) methodology and in following the City's <u>Design</u> and Construction Standards Manual (DCSM).

Sources of data for this study include information provided by the Institute of Transportation Engineers (ITE), the Virginia Department of Transportation (VDOT), the City of Manassas, and the office files and field reconnaissance efforts of Gorove Slade.

Background Information: Proposed Edugenius Development (Site & Nearby)

Description of the Existing Site

Site Location

The site is located in the City of Manassas, Virginia. It is situated along the western edge of Fairview Avenue (Route 4353) and northern edge of Wesley Avenue, south of Signal Hill Road (City Route 8). An aerial of the study area is provided in Figure 1.

A description of the proposed Edugenius Development is provided in the Introduction section of this report.



Figure 1: Site Location

Location within Jurisdiction and Region

The site is located at the southeastern edge of Old Town (Downtown) Manassas.

A regional aerial of the site is provided in Figure 2.



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Figure 2: Regional Location

Description of the Parcel(s)

The site will be situated on an approximately 3.3-acre single parcel of land, that can be identified with the following Tax Map ID #: 100-01-00-202, as illustrated in Figure 3.



Figure 3: City of Manassas Parcel Map (Parcels source: Manassas Zoning and Parcel Mapping App, https://cityofmanassas.maps.arcgis.com/apps/webappviewer/index.html?id=5435b6acfd274042a95914ad9ae97660)

Existing Zoning and Long-Range Land-Use

The parcel is currently zoned as R-1 (Low Density, Single-Family Residential), with a Comprehensive Plan Land Use of Downtown. Of note, the parcel is a part of the Historic Overlay District. The existing zoning is presented in Figure 4; the Comprehensive Plan Land Use is presented in Figure 5.

The Character Areas graphic from the City of Manassas Comprehensive Plan, which includes the Land Use, Gateways and Corridors is illustrated in Figure 6.



Figure 4: City of Manassas Zoning Map (Base zoning source: Manassas Zoning and Parcel Map App, https://cityofmanassas.maps.arcgis.com/apps/webappviewer/index.html?id=5435b6acfd274042a95914ad9ae97660)



Figure 5: City of Manassas Comprehensive Plan Land Use Map (Base land use source: Manassas Zoning and Parcel Map App, https://cityofmanassas.maps.arcgis.com/apps/webappviewer/index.html?id=5435b6acfd274042a95914ad9ae97660)



Figure 6: City of Manassas Character Areas Map

(Base area source: City of Manassas 2040 Comprehensive Plan,

https://www.manassasva.gov/Community%20Development/Comp%20Plan/2020%20Comprehensive%20Plan%20Character% 20Areas%20Map.pdf)

The proposed Edugenius Development is located on the western edge of Fairview Avenue and northern edge of Wesley Avenue. According to the <u>City of Manassas 2040 Comprehensive Plan</u> (The Comprehensive Plan), as adopted February 24, 2020, in particular the Mobility Chapter 6, there are no current recommendations to widen Fairview Avenue (nor any street) within the vicinity of the site.

Per the 2022 Five-Year Capital Improvement Program, Fairview Avenue is proposed to be improved by the City (Project T-075: Fairview Avenue / Tudor lane Signal Project) with a signal at Tudor Lane once a signal is warranted. Of note, it is anticipated that a signal would not be warranted in the current time frame of this Edugenius Development. The undergrounding of utilities and historic streetlight installation portion of the City project was completed in 2016.

A map of the Comprehensive Plan City-wide Transportation Recommendations is depicted in Figure 7.

Additionally, per Chapter 6 of the Plan, the following bicycle facility recommendations are planned or funded:

- Bike Lanes are planned along Signal Hill Road / Windsor Avenue / Tudor Lane / Prince William Street between the
 eastern City corporate limits and Grant Avenue. As will be depicted in the *Existing Bicycle and Pedestrian Facilities*section of the report, the roadway within the vicinity of the site currently includes a signed-on street bicycle route along
 one portion and shared lane markings (sharrows) along other portions.
- Shared lane markings are planned along the entirety of Fairview Avenue from the road terminus of Quarry Road to Wellington Road.
- The existing shared use path along Wellington Road is planned to gain a shared use path connection to the existing bike lanes along Richmond Avenue, and connecting to the ultimate shared lane markings along Fairview Avenue (the Richmond Avenue bike lanes are referenced in the *Existing Bicycle and Pedestrian Facilities* section of the report as well).

The bicycle facility recommendations City-wide are depicted in Figure 8.

Chapter 6 of the Plan also provides the following pedestrian facility recommendations:

- Connect the existing sidewalks and provide an off-street path connection between Fairview Avenue and Osbourn High School / Baldwin Elementary School. Note, a portion of this project was recently completed.
- Similarly, connect the existing sidewalks along Fairview Avenue and along Signal Hill Road with sidewalk gap connections along Tudor Lane and Windsor Avenue
- Intersection Safety Improvements are recommended at the intersection of Fairview Avenue and Wellington Road (northeast of Metz Middle School).
- In addition, it is recommended to improve pedestrian safety and access along the entirety of the Fairview Avenue corridor (as part of the overall Transportation Recommendations)

The pedestrian facility recommendations City-wide are depicted in Figure 9.



Figure 7: Comprehensive Plan – Transportation Recommendations (Source: <u>City of Manassas 2040 Comprehensive Plan</u>, Chapter 6: Mobility, https://www.manassasva.gov/community_development/planning_and_zoning/comprehensive_plan_update.php)

Parks, Recreation, and Cultural Sit

Roadw



Figure 8: Comprehensive Plan – Bicycle Network and Facility Recommendations (Source: <u>City of Manassas 2040 Comprehensive Plan</u>, Chapter 6: Mobility, https://www.manassasva.gov/community_development/planning_and_zoning/comprehensive_plan_update.php)



Figure 9: Comprehensive Plan – Pedestrian Network and Facility Recommendations (Source: <u>City of Manassas 2040 Comprehensive Plan</u>, Chapter 6: Mobility, https://www.manassasva.gov/community_development/planning_and_zoning/comprehensive_plan_update.php)

Descriptions of Geographic Scope of Study and Limits of the Study Area

The site is located in the City of Manassas, Virginia. The Edugenius Development is proposed to be situated along the western edge of Fairview Avenue (Route 4353) and northern edge of Wesley Avenue, south of Signal Hill Road (City Route 8).

The vehicular study area includes five (5) existing intersections as well as two (2) future intersections. The existing study intersections are as follows:

Intersection 1:	Fairview Avenue (Route 4353) at Tudor Lane (Route 9528) [four-legged, unsignalized],
Intersection 2:	Fairview Avenue (Route 4353) at Manassas Police Department Entrance/Signal Hill Road (City Route 8) [four-legged, signalized],
Intersection 3:	Fairview Avenue (Route 4353) at Wesley Avenue [four-legged, unsignalized],
Intersection 4:	Fairview Avenue (Route 4353) at Richmond Avenue (City Route 9) [four-legged, unsignalized], and
Intersection 5:	Fairview Avenue (Route 4353) at Wellington Road (Route 4352) [three-legged, signalized].
The future study inters	section is as follows:
Intersection 6:	Fairview Avenue (Route 4353) at Site Entrance 1 [three-legged, anticipated to be unsignalized],
Intersection 7:	Fairview Avenue (Route 4353) at Site Entrance 2 [three-legged, left-ins restricted, anticipated to be unsignalized], and

Intersection 8: Wesley Avenue at Site Entrance 3 [three-legged, anticipated to be unsignalized].

The future study intersections will provide access to the Site. An aerial of the study intersections is provided in Figure 10.



Figure 10: Study Intersections Map

Existing Roadway Network

A description of the major roadways within the immediate vicinity of the site is summarized in Table 1. Of note, for the purposes of this analysis, Fairview Avenue is assumed to run north to south, and the other study roadways, east to west.

		ting Road R							
Roadway	RTE #	Manassas City Classification	Lanes	Posted/Legal Speed Limit (mph)	On-Street Parking	AADT* (vpd)	Road Segment I	Between:	k-facto
Fairview Avenue	4353	Minor Arterial	2-Lane Undivided	25	No	13,000	Wellington Ave	SR 28 Center St	8.4%
Wellington Road	4352	Minor Arterial	4-Lane Divided	25	No	18,000	BUS SR 234 Dumfries Rd	Fairview Ave	9.5%
Signal Hill Road	8	Major Collector	2-Lane Undivided	25	Yes	6,100	Richmond Ave	Liberia Ave	8.7%
Tudor Lane	9528	-	2-Lane Undivided	25	(E of Fairview	2,500	Osbourn High School	Cul de sac	10.0%
Wesley Avenue	-	-	2-Lane Undivided	25	Yes	-	-	-	-
Richmond Avenue	9	Minor Collector	2-Lane Undivided	25	Yes	3,500	Fairview Ave	Liberia Ave	10.3%

Table 1: Summary of Existing Road Network

Planned Future Transportation Improvements

Roadway Improvements

There are currently no planned nor funded road improvements adjacent to nor within the vicinity of the site. A City-wide signal optimization was recently completed.

Bicycle and Pedestrian Improvements

There are currently no funded bicycle and pedestrian improvements adjacent to nor within the vicinity of the site.

Analysis of 2022 Existing Conditions

In order to project future traffic conditions, it was necessary to create an existing scenario.

Existing Transit Services

No bus routes currently provide service within the vicinity of the site, though bus routes service the Manassas Train Station.

The site is located within a 0.55-mile radius of the Manassas Train Station, which serves the VRE Manassas Line, which primarily provides commuter service to and from Broad Run Station to the west, and to and from Union Station in Washington, D.C. to the east, with a connection to the VRE Fredericksburg Line at the Alexandria Station. The station is served by Amtrak as well.

The site is also located within a 3.0-mile radius of the Manassas Regional Airport (HEF).

Existing Bicycle and Pedestrian Facilities

Dedicated bicycle infrastructure exists along roadways in the site, as follows:

- The northside of Wellington Road contains an off-street shared use path.
- On-Street bike lanes are striped in each direction along Richmond Avenue, generally east of Fairview Avenue.
- Shared lane markings (sharrows) exist along Tudor Lane and Signal Hill Road in the vicinity of the site; the two sections are connected via a signed shared roadway along a portion of Tudor Lane and Windsor Avenue.

The existing bicycle facilities are depicted in Figure 11.

Sidewalks and curb ramps generally exist along the corridors adjacent to and within the vicinity of the site. Adequate sidewalks and curb ramps exist along Fairview Avenue, Tudor Lane west of Fairview Avenue, Signal Hill Road, Wesley Avenue west of Fairview Avenue (one side), Richmond Avenue (one side west of Fairview Avenue), and Wellington Road (southside; the north side containing the aforementioned off-street shared use path). Marked striped crosswalks currently exist on at least one leg at all signalized intersections, while some unsignalized intersections include a marked striped crosswalk on at least one leg. Given the residential nature of the area, some of the lower speed roadways do not contain sidewalks through the neighborhoods, although, as mentioned previously, the Comprehensive Plan calls for some sidewalk gap connections through the neighborhoods.

A raised curb bump out and raised median exist on the east leg of Richmond Avenue at its intersection with Fairview Avenue as a traffic calming measure.

The existing pedestrian facilities are depicted in Figure 12, while Figure 14 depicts the existing pedestrian peak hour volumes, and Figure 13 the existing bicyclist peak hour volumes, based on the existing counts collected at the study intersections (as further described in the *Existing Traffic Volumes* subsection). For the purposes of this analysis, no inherent growth is assumed for pedestrian and cycling trips, and the pedestrians and cyclists are assumed to carry through to the study intersections to the south and north, as applicable.



Figure 11: Existing Bicycle Facilities



Figure 12: Existing Pedestrian Facilities



Figure 13: 2022 Existing Conditions – Peak Hour Bicyclist Volumes



Figure 14: 2022 Existing Conditions – Peak Hour Pedestrian Volumes

Existing Roadway Safety Assessment

Historical crash data was obtained from the VDOT's Crash Analysis Tool for the existing study intersections for a six-year period between January 2014 through December 2019, as more recent data may be influenced by the COVID-19 pandemic. A summary of the reported crashes for the study area intersections by severity is provided in Table 2. As based on the summary, a total of 25 crashes were reported for the study area intersections during the six-year period, with a majority being classified with a severity of Injury Collision (IC).

Inte	rsection	PDO	IC	Fatality	Total	Crash Rate (Per MEV)
1	Fairview Avenue at Tudor Lane	3	1	0	4	0.12
2	Fairview Avenue at Signal Hill Road	2	1	0	3	0.09
3	Fairview Avenue at Wesley Street	0	1	0	1	0.05
4	Fairview Avenue at Richmond Avenue	0	2	0	2	0.07
5	Fairview Avenue at Wellington Road	4	11	0	15	0.31
Tota	I Reported Crashes Analyzed	9	16	0	25	-
Per	centages	36.0%	64.0%	0.0%	100.0%	-

 Table 2: Study Area Historical Crash Summary (January 2014 – December 2019)

The intersection crash rate was computed for the existing study intersections using the following formula and was calculated as crashes per one million entering vehicles (MEV). The approach average daily traffic volumes (AADT_{approach}) were derived from calculations based on the existing link ADTs.

$$Rate_{intersection} = \frac{1,000,000 * \# of Crashes}{\# of Years * 365 \left(\frac{days}{year}\right) * ADT_{approach}}$$

It should be noted that according to the Institute of Transportation Engineers' (ITE) <u>Transportation Impact Analysis for Site</u> <u>Development</u>, a crash rate of 1.0 MEV or higher is an indication that further study is required. A rate over 1.0 MEV does not necessarily mean there is a significant problem at an intersection, but rather it is a threshold used to identify which intersections may have an elevated crash rate due to operational, geometric, or other deficiencies. Based on the crash rates, none of the existing study intersections were high crash locations.

A map of the study area showing the recorded and related intersection crashes that occurred during the study period and their recorded severity is presented in Figure 15. Detailed reported crash data is presented in the following tables by study intersection. The raw crash data by intersections is provided in Appendix C.



Figure 15: Reported Collision Map for Existing Study Area Intersections (January 2014 – December 2019)

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Table 3: VDOT Crash Data at Fairview Avenue and Tudor Lane (Study Intersection 1)

Intersection Crash Analysis	Crash Data for the Intersection of Fairview Avenue and Tudor Lane (2014 - 2019)							
Intersection of dat Analysis	2014	2015	2016	2017	2018	2019	Total	Relative Frequency
Crash Severity								
Fatal Collision (Type K)								0.00%
Injury Collision (Type A, B, and C)					1		1	25.00%
Туре А								
Type B								
Туре С					1		1	
Property Damage Only (Type PDO)			1		2		3	75.00%
TOTAL*			1		3		4	100.00%
Crash Type								
Fixed Object/ Single-Vehicle Crash					1		1	25.00%
Head-On								0.00%
Sideswipe / Same Direction								0.00%
Sideswipe / Opposite Direction								0.00%
Rear-End Collision					1		1	25.00%
Angle Collision			1				1	25.00%
Backed Into								0.00%
Pedestrian Collision					1		1	25.00%
Deer/Animal								0.00%
Other								0.00%
TOTAL*			1		3		4	100.00%
Other Factors								
Distracted Driver								0.00%
Alcohol**					1		1	25.00%
Work-Zone					1		1	25.00%
Inclement Weather (Non-Dry)					1		1	25.00%
Speeding								0.00%
Disregard of Traffic Control Device								0.00%
Pedestrian Injury***								N/A
Time of Day								
AMPeak Period (6 - 10 AM)					1		1	25.00%
Off Peak - Daytime (10 AM - 3 PM)					1		1	25.00%
PM Peak Period (3 - 7 PM)			1				1	25.00%
					1		1	25.00%

* It should be noted that an intersection radius of 200 feet was used in this analysis. Crashes also thought to be caused by the intersection may have been added based on the description of the crash and engineering judgement.

** Instances where the event was classified as "Unknown", "Not Known Whether Impaired", "Ability Not Impaired" were classified as alcohol related to provide a more conservative analysis.

Table 4: VDOT Crash Data at Fairview Avenue and Signal Hill Road (Study Intersection 2)

2014 2015 2016 2017 2018 2019 Total Relative Frequency Crash Severity	Intersection Crash Analysis	Crash Data for the Intersection of Fairview Avenue and Signal Hill Road (2014 - 2019)								
Crash Severity 0.00% Fatal Collision (Type A B, and C) 1 33.33% Type A 1 33.33% Type B 1 33.33% Type C 1 3 Property Damage Only (Type PDO) 1 1 3 Tope C 2 1 3 100.00% Crash Yoe 1 3 100.00% 2 66.67% TOTAL* 2 1 3 100.00% 3 100.00% Crash Yoe 0.00% 3 100.00% 3 3 00.00% 3 3 00.00% 3 3 0.00% 3 0.00% 3 3 0.00% 3 3 0.00% 3		2014	2015	2016	2017	2018	2019	Total	Relative Frequency	
Fatal Collision (Type K) 0.00% Injury Collision (Type A, B, and C) 1 3.3.33% Type A 1 3.3.33% Type B 7 7 8 Type C 3 1000% 8 Property Damage Only (Type PDO) 1 1 2 66.67% TorAL* 2 1 3.3.33% 1 3.3.33% Crash Type 7 9 1 1 3.3.33% Crash Type 1 1 3.3.33% 1 3.3.33% Gead-On 1 1 3.3.33% 1 3.3.33% Gleswije / Same Direction 1 3.3.33% 1 3.3.33% Sideswije / Opposite Direction 1 3.3.33% 1 3.3.33% Angle Collision 1 3.3.33% 1 3.3.33% 3.3.33% Backed Into 0.00% 1 3.3.33% 3.3.33% 3.3.3.3% 3.3.3.3% 3.3.3.3% 3.3.3.3% 3.3.3.3% 3.3.3.3% 3.3.3.3% 3.3.3.3	Crash Severity									
Injury Collision (Type A B, and C) 1 33.33% Type A 1 1 Type B 1 1 Type C 2 66.67% TOTAL 2 7 Total Same Construction 3 100.00% Crash Type 3 100.00% Total Same Construction 1 33.33% Head-On 1 1 33.33% Head-On 1 1 33.33% Sideswipe / Same Direction 1 0.00% Sideswipe / Opposite Direction 1 33.33% Argle Collision 1 33.33% Argle Collision 1 33.33% Backed Into 1 33.33% Deet/Animal 1 33.33% Other 0.00% 0.00% Total Control Device 1 33.33% Acohot* 0.00% 0.00% Other 0.00% 0.00% Other 1 33.33% Distacted Driver <t< td=""><td>Fatal Collision (Type K)</td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td>0.00%</td></t<>	Fatal Collision (Type K)								0.00%	
Type A 1 1 Type B Type C 66.67% Properly Damage Only (Type PDO) 1 1 2 66.67% TOTAL* 2 1 3 100.00% Crash Type 3 100.00% 33.33% Head-On 1 33.33% 1 Sideswipe / Same Direction 1 0.00% 33.33% Rear-End Collision 1 33.33% 33.33% Angle Collision 1 33.33% 33.33% Backed Into 1 33.33% 30.00% Other 0.00% 0.00% 0.00% Other Factors 0.00% 0.00% Distracted Driver 1 33.33% 33.33% Acohot** 0.00% 0.00% 0.00% Distracted Driver 1 0.00% 0.00% </td <td>Injury Collision (Type A, B, and C)</td> <td>1</td> <td></td> <td></td> <td></td> <td></td> <td></td> <td>1</td> <td>33.33%</td>	Injury Collision (Type A, B, and C)	1						1	33.33%	
Type B Type C Property Damage Only (Type PDO) 1 1 2 66.67% OTAL* 2 1 3 100.00% Crash Type 1 33.33% Fixed Object/ Single-Vehicle Crash 1 1 33.33% Head-On 1 0.00% 0.00% Sideswipe / Same Direction 1 0.00% 0.00% Sideswipe / Opposite Direction 1 33.33% 0.00% Rear-End Collision 1 33.33% 0.00% Rear-End Collision 1 33.33% 0.00% Pedestrian Collision 1 33.33% 0.00% Detr/Animal 0 0.00% 0.00% Other 0 0.00% 0.00% ToTAL* 2 1 33.33% Alcohot* 0 0.00% 0.00% Other Factors 0.00% 0.00% Inclement Weather (Non-Dry) 0.00% 0.00% Speeding <td>Туре А</td> <td>1</td> <td></td> <td></td> <td></td> <td></td> <td></td> <td>1</td> <td></td>	Туре А	1						1		
Type C Image Only (Type PDO) 1 1 2 66.67% TOTAL 2 1 3 100.00% Crash Type 3 100.00% Crash Type 1 33.33% Head-On 1 33.33% Head-On 0.00% 0.00% Sideswipe / Same Direction 1 0.00% Sideswipe / Opposite Direction 1 0.00% Rear-End Collision 1 33.33% Angle Collision 1 33.33% Angle Collision 1 33.33% Backed Into 1 0.00% Pedestrian Collision 1 0.00% Other 0.00% 0.00% TOTAL 2 1 3 Other 0.00% 0.00% Other 0.00% 0.00% Other 1 33.33% Distracted Driver 1 33.33% Nacholit* 0.00% 0.00% Distracted Driver 1	Туре В									
Property Damage Only (Type PDO) 1 1 2 66.67% TOTAL* 2 1 3 100.00% Crash Type	Туре С									
TOTAL* 2 1 3 100.00% Crash Type	Property Damage Only (Type PDO)	1		1				2	66.67%	
Crash Type Fixed Object/ Single-Vehicle Crash 1 33.33% Head-On 0.00% 0.00% Sideswipe / Same Direction 0.00% 0.00% Sideswipe / Same Direction 1 0.00% Sideswipe / Opposite Direction 1 33.33% Argle Collision 1 33.33% Angle Collision 1 33.33% Backed Into 1 33.33% Backed Into 0.00% 0.00% Deer/Animal 0.00% 0.00% Other 0.00% 0.00% TOTAL* 2 1 33.33% Distracted Driver 1 33.33% Acohol** 0.00% 0.00% Work-Zone 0.00% 0.00% Inclement Weather (Non-Dry) 0.00% 0.00% Speeding 0.00% 0.00% Distragard of Traffic Control Device 0.00% 0.00% Pedestrian Injury*** NIA NIA Time of Day 2 66.67% 0.00%	TOTAL*	2		1				3	100.00%	
Fixed Object/ Single-Vehicle Crash 1 33.33% Head-On 0.00% Sideswipe / Same Direction 0.00% Sideswipe / Opposite Direction 0.00% Rear-End Collision 1 33.33% Angle Collision 1 33.33% Angle Collision 1 33.33% Angle Collision 1 33.33% Pedestrian Collision 1 33.33% Deer/Animal 0.00% 0.00% Other 0.00% 0.00% TOTAL* 2 1 0.00% Other 0.00% 0.00% 0.00% Other Factors	Crash Type									
Head-On 0.00% Sideswipe / Same Direction 0.00% Sideswipe / Opposite Direction 0.00% Rear-End Collision 1 33.33% Angle Collision 1 33.33% Backed Into 1 33.33% Pedestrian Collision 1 33.33% Deer/Animal 0.00% 0.00% Other Factors 0.00% 0.00% Other Factors 0.00% 0.00% Other Factors 0.00% 0.00% Work-Zone 1 33.33% Inclement Weather (Non-Dry) 0.00% 0.00% Speeding 0.00% 0.00% Disregard of Traffic Control Device 0.00% 0.00% Pedestrian Injury*** N/A N/A Time of Day 2 66.67% 0.00%	Fixed Object/ Single-Vehicle Crash			1				1	33.33%	
Sideswipe / Same Direction 0.00% Sideswipe / Opposite Direction 0.00% Rear-End Collision 1 33.33% Angle Collision 1 33.33% Angle Collision 1 33.33% Backed Into 1 33.33% Pedestrian Collision 0.00% 0.00% Pedestrian Collision 0 0.00% Deer/Animal 0 0.00% Other 2 1 3 0.00% Other Factors 0.00% 0.00% 0.00% Other 1 33.33% 0.00% Vork-Zone 1 33.33% 0.00% Inclement Weather (Non-Dry) 0.00% 0.00% 0.00% Disregard of Traffic Control Device 0.00% 0.00% 0.00% Pedestrian Injury*** N/A N/A Time of Day 2 66.67% 0.00%	Head-On								0.00%	
Sideswipe / Opposite Direction 0.00% Rear-End Collision 1 33.33% Angle Collision 1 33.33% Backed Into 1 33.33% Backed Into 0.00% 0.00% Pedestrian Collision 0.00% 0.00% Deer/Animal 0.00% 0.00% Other 0.00% 0.00% TOTAL* 2 1 3 Distracted Driver 1 33.33% 0.00% Other 0.00% 0.00% 0.00% Distracted Driver 1 33.33% 0.00% Vork-Zone 1 33.33% 0.00% Inclement Weather (Non-Dry) 0.00% 0.00% 0.00% Speeding 0.00% 0.00% 0.00% Disregard of Traffic Control Device 0.00% 0.00% Pedestrian Injury*** N/A N/A Time of Day 2 66.67% 0.00%	Sideswipe / Same Direction								0.00%	
Rear-End Collision 1 33.33% Angle Collision 1 33.33% Backed Into 1 33.33% Backed Into 1 33.33% Pedestrian Collision 0.00% 0.00% Deer/Animal 0.00% 0.00% Other 0.00% 0.00% TOTAL* 2 1 3 Other Factors 0.00% 0.00% Distracted Driver 1 33.33% Alcohol** 0 0.00% Vork-Zone 0.00% 0.00% Inclement Weather (Non-Dry) 0.00% 0.00% Speeding 0.00% 0.00% Disregard of Traffic Control Device 0.00% 0.00% Pedestrian Injury*** N/A N/A Time of Day 2 66.67% 0.00%	Sideswipe / Opposite Direction								0.00%	
Angle Collision 1 33.33% Backed Into 0.00% Pedestrian Collision 0.00% Deer/Animal 0.00% Other 0.00% TOTAL* 2 1 0.00% Other Factors 0.00% 0.00% Distracted Driver 1 33.33% Alcohol** 0 0.00% Work-Zone 1 33.33% Inclement Weather (Non-Dry) 0.00% 0.00% Speeding 0.00% 0.00% Disregard of Traffic Control Device 0.00% 0.00% Pedestrian Injury*** N/A N/A Time of Day 2 66.67% 0.00%	Rear-End Collision	1						1	33.33%	
Backed Into 0.00% Pedestrian Collision 0.00% Deer/Animal 0.00% Other 0.00% TOTAL* 2 1 0.00% Other Factors 0.00% 0.00% Distracted Driver 1 33.33% Alcohol** 1 33.33% Nork-Zone 0.00% 0.00% Inclement Weather (Non-Dry) 0.00% 0.00% Speeding 0.00% 0.00% Disregard of Traffic Control Device 0.00% 0.00% Pedestrian Injury*** C N/A Time of Day 2 66.67% 0.00%	Angle Collision	1						1	33.33%	
Pedestrian Collision 0.00% Deer/Animal 0.00% Other 0.00% TOTAL* 2 1 3 0.00% Other Factors 0.00% 0.00% 0.00% Other Sectors 0.00% 0.00% 0.00% Other Sectors 0.00% 0.00% 0.00% Vork-Zone 1 33.33% 0.00% Inclement Weather (Non-Dry) 0.00% 0.00% 0.00% Speeding 0.00% 0.00% 0.00% Distracted Driver 2 66.67% 0.00% MPeak Period (6 - 10 AM) 2 66.67% 0.00%	Backed Into								0.00%	
Deer/Animal 0.00% Other 0.00% TOTAL* 2 1 3 100.00% Other Factors 0.00% 0.00% 0.00% 0.00% Other Factors 1 33.33% 0.00% 0.00% 0.00% Alcohol** 1 33.33% 0.00% <td>Pedestrian Collision</td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td>0.00%</td>	Pedestrian Collision								0.00%	
Other 0.00% TOTAL* 2 1 3 100.00% Other Factors 0.00% 0.00% 0.00% Distracted Driver 1 33.33% 0.00% Acohol** 1 33.33% 0.00% Work-Zone 0.00% 0.00% 0.00% Inclement Weather (Non-Dry) Speeding 0.00% 0.00% Speeding 0.00% 0.00% 0.00% Disregard of Traffic Control Device 0.00% 0.00% Pedestrian Injury*** 1 2 66.67% Off Beak - Davinge (10 AM, 3 PM) 2 66.67% 0.00%	Deer/Animal								0.00%	
TOTAL* 2 1 3 100.00% Other Factors Distracted Driver 1 33.33% Acohol** 1 33.33% Acohol** 1 33.33% 0.00% <td< td=""><td>Other</td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td>0.00%</td></td<>	Other								0.00%	
Other Factors Distracted Driver 1 33.33% Alcohol** 1 33.33% Alcohol** 0.00% 0.00% Work-Zone 0.00% 0.00% Inclement Weather (Non-Dry) 0.00% 0.00% Speeding 0.00% 0.00% Disregard of Traffic Control Device 0.00% 0.00% Pedestrian Injury*** 0 N/A Time of Day 2 66.67% Off Beak - Davime (10 AM, 3 PM) 0.00% 0.00%	TOTAL*	2		1				3	100.00%	
Distracted Driver 1 33.33% Alcohol** 0.00% Work-Zone 0.00% Inclement Weather (Non-Dry) 0.00% Speeding 0.00% Disregard of Traffic Control Device 0.00% Pedestrian Injury*** 0.00% Time of Day 2 AM Peak Period (6 - 10 AM) 2 Off Beak - Daving (10 AM - 3 PM) 0.00%	Other Factors									
Alcohol** 0.00% Work-Zone 0.00% Inclement Weather (Non-Dry) 0.00% Speeding 0.00% Disregard of Traffic Control Device 0.00% Pedestrian Injury*** 0.00% Time of Day 2 AM Peak Period (6 - 10 AM) 2 Off Peak - Daving (10 AM, 3 PM) 0.00%	Distracted Driver	1						1	33.33%	
Work-Zone 0.00% Inclement Weather (Non-Dry) 0.00% Speeding 0.00% Disregard of Traffic Control Device 0.00% Pedestrian Injury*** 0.00% Time of Day 2 AM Peak Period (6 - 10 AM) 2 Off Peak - Daytime (10 AM - 3 PM) 0.00%	Alcohol**								0.00%	
Inclement Weather (Non-Dry) 0.00% Speeding 0.00% Disregard of Traffic Control Device 0.00% Pedestrian Injury*** N/A Time of Day 2 AM Peak Period (6 - 10 AM) 2 Off Peak - Daytime (10 AM - 3 PM) 0.00%	Work-Zone								0.00%	
Speeding 0.00% Disregard of Traffic Control Device 0.00% Pedestrian Injury*** N/A Time of Day AM Peak Period (6 - 10 AM) 2 66.67% Off Peak - Daytime (10 AM, 3 PM) 00% 00%	Inclement Weather (Non-Dry)								0.00%	
Disregard of Traffic Control Device 0.00% Pedestrian Injury*** N/A Time of Day AM Peak Period (6 - 10 AM) 2 66.67% Off Peak - Daytime (10 AM, 3 PM) 0.00% 0.00%	Speeding								0.00%	
Pedestrian Injury*** N/A Time of Day	Disregard of Traffic Control Device								0.00%	
Time of Day 2 66.67% AM Peak Period (6 - 10 AM) 2 66.67% Off Peak - Daytime (10 AM, 3 PM) 000%	Pedestrian Injury***								N/A	
AM Peak Period (6 - 10 AM) 2 66.67% Off Peak - Davtime (10 AM, 3 PM) 0.00% 0.00%	Time of Day									
0tt Peak - Daytime (10 AM - 3 PM)	AM Peak Period (6 - 10 AM)	2						2	66.67%	
	Off Peak - Daytime (10 AM - 3 PM)								0.00%	
PM Peak Period (3 - 7 PM) 0.00%	PMPeak Period (3 - 7 PM)								0.00%	
Off Peak - Nightume (7 PM - 6 AM) 1 33.33%	Off Peak - Nighttime (7 PM - 6 AM)			1				1	33.33%	

* It should be noted that an intersection radius of 200 feet was used in this analysis. Crashes also thought to be caused by the intersection may have been added based on the description of the crash and engineering judgement.

** Instances where the event was classified as "Unknow n", "Not Know n Whether Impaired", "Ability Not Impaired" were classified as alcohol related to provide a more conservative analysis.

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Table 5: VDOT Crash Data at Fairview Avenue and Wesley Avenue (Study Intersection 3)

Intersection Crash Analysis	Crash Data for the Intersection of Fairview Avenue and Wesley Avenue (2014 - 2019)								
intersection ordan Anarya's	2014	2015	2016	2017	2018	2019	Total	Relative Frequency	
Crash Severity									
Fatal Collision (Type K)								0.00%	
Injury Collision (Type A, B, and C)	1						1	100.00%	
Туре А									
Туре В	1						1		
Туре С									
Property Damage Only (Type PDO)								0.00%	
TOTAL*	1						1	100.00%	
Crash Type									
Fixed Object/ Single-Vehicle Crash								0.00%	
Head-On								0.00%	
Sideswipe / Same Direction								0.00%	
Sideswipe / Opposite Direction								0.00%	
Rear-End Collision								0.00%	
Angle Collision								0.00%	
Backed Into								0.00%	
Pedestrian Collision								0.00%	
Deer/Animal								0.00%	
Other	1						1	100.00%	
TOTAL*	1						1	100.00%	
Other Factors									
Distracted Driver	1						1	100.00%	
Alcohol**								0.00%	
Work-Zone								0.00%	
Inclement Weather (Non-Dry)	1						1	100.00%	
Speeding								0.00%	
Disregard of Traffic Control Device								0.00%	
Pedestrian Injury***								N/A	
Time of Day								0.000/	
AM Peak Period (6 - 10 AM)								0.00%	
On Peak - Daytime (10 AM - 3 PM)	1						1	0.00%	
Off Peak - Nighttime (7 PM - 6 AM	1						1	0.00%	
CALCULATED CRASH RATE							0.05	Crashes per MEV	

* It should be noted that an intersection radius of 200 feet was used in this analysis. Crashes also thought to be caused by the intersection may have been added based on the description of the crash and engineering judgement.

** Instances where the event was classified as "Unknown", "Not Known Whether Impaired", "Ability Not Impaired" were classified as alcohol related to provide a more conservative analysis.

Table 6: VDOT Crash Data at Fairview Avenue and Richmond Avenue (Study Intersection 4)

Intersection Crash Analysis		Crash Data for the Intersection of Fairview Avenue and Richmond Avenue (2014 - 2019)								
	2014	2015	2016	2017	2018	2019	Total	Relative Frequency		
Crash Severity										
Fatal Collision (Type K)								0.00%		
Injury Collision (Type A, B, and C)				1	1		2	100.00%		
Туре А										
Туре В										
Туре С				1	1		2			
Property Damage Only (Type PDO)								0.00%		
TOTAL*				1	1		2	100.00%		
Crash Type										
Fixed Object/ Single-Vehicle Crash								0.00%		
Head-On								0.00%		
Sideswipe / Same Direction								0.00%		
Sideswipe / Opposite Direction								0.00%		
Rear-End Collision								0.00%		
Angle Collision				1	1		2	100.00%		
Backed Into				-				0.00%		
Pedestrian Collision								0.00%		
Deer/Animal								0.00%		
Other								0.00%		
TOTAL*				1	1		2	100.00%		
Other Factors										
Distracted Driver								0.00%		
Alcohol**								0.00%		
Work-Zone								0.00%		
Inclement Weather (Non-Dry)					1		1	50.00%		
Speeding								0.00%		
Disregard of Traffic Control Device								0.00%		
Pedestrian Injury***								N/A		
Time of Day										
AMPeak Period (6 - 10 AM)				1			1	50.00%		
Off Peak - Daytime (10 AM - 3 PM)								0.00%		
PM Peak Period (3 - 7 PM)								0.00%		
Off Peak - Nighttime (7 PM - 6 AM)					1		1	50.00%		
CALCULATED CRASH RATE							0.07	Crashes per MEV		

* It should be noted that an intersection radius of 200 feet was used in this analysis. Crashes also thought to be caused by the intersection may have been added based on the description of the crash and engineering judgement.

** Instances where the event was classified as "Unknow n", "Not Know n Whether Impaired", "Ability Not Impaired" were classified as alcohol related to provide a more conservative analysis.

Table 7: VDOT Crash Data at Fairview Avenue and Wellington Road (Study Intersection 5)

Intersection Crash Analysis	Crash Data for the Intersection of Fairview Avenue and Wellington Road (2014 - 2019)								
	2014	2015	2016	2017	2018	2019	Total	Relative Frequency	
Crash Severity									
Fatal Collision (Type K)								0.00%	
Injury Collision (Type A, B, and C)	2	4	2	1	1	1	11	73.33%	
Туре А		1					1		
Туре В					1	1	2		
Туре С	2	3	2	1			8		
Property Damage Only (Type PDO)		2	1	1			4	26.67%	
TOTAL*	2	6	3	2	1	1	15	100.00%	
Crash Type									
Fixed Object/ Single-Vehicle Crash	1						1	6.67%	
Head-On				1			1	6.67%	
Sideswipe / Same Direction								0.00%	
Sideswipe / Opposite Direction								0.00%	
Rear-End Collision		4	1		1	1	7	46.67%	
Angle Collision	1	2	1	1			5	33.33%	
Backed Into							-	0.00%	
Pedestrian Collision								0.00%	
Deer/Animal								0.00%	
Other			1				1	6.67%	
TOTAL*	2	6	3	2	1	1	15	100.00%	
Other Factors									
Distracted Driver			1				1	6.67%	
Alcohol**				1	1		2	13.33%	
Work-Zone								0.00%	
Inclement Weather (Non-Dry)	2	2	1				5	33.33%	
Speeding								0.00%	
Disregard of Traffic Control Device	1	1					2	13.33%	
Pedestrian Injury***								N/A	
Time of Day									
AMPeak Period (6 - 10 AM)								0.00%	
Off Peak - Daytime (10 AM - 3 PM)		1	1		1		3	20.00%	
PMPeak Period (3 - 7 PM)	2	4	1	1		1	9	60.00%	
Off Peak - Nighttime (7 PM - 6 AM)		1	1	1			3	20.00%	

 CALCULATED CRASH RATE
 0.31
 Crashes per MEV

 * It should be noted that an intersection radius of 200 feet was used in this analysis. Crashes also thought to be caused by the intersection may have been added based on the description of the crash and engineering judgement.

** Instances where the event was classified as "Unknown", "Not Known Whether Impaired", "Ability Not Impaired" were classified as alcohol related to provide a more conservative analysis.

Existing Traffic Volumes

In order to determine the weekday morning (AM) and weekday afternoon (PM) peak hour turning movement traffic volumes, turning movement counts (TMC) were collected at the existing study intersections on Tuesday, May 24, 2022, during AM and PM peak periods (a total of six (6) hours), or from the hours of 6:00 AM to 9:00 AM and 4:00 PM to 7:00 PM. These counts were appropriate balanced and cross verified with 2016 counts at the same intersections and were determined to be consistent with pre-Covid conditions, as per the scoping discussions.

From the turning movement counts, the following system peak hours were determined.

- AM Peak Hour: 7:30 AM to 8:30 AM
- PM Peak Hour: 4:15 PM to 5:15 PM

The 2022 existing road network configuration is presented in Figure 16. The existing peak hour traffic volumes for the intersections within the study area are illustrated in Figure 17. The average daily traffic (ADT) volumes, depicted in this figure and in subsequent volume graphics, were calculated based on the PM peak hour turning movement volumes and multiplied by the historic VDOT k-factors from 2019. If the historic data was not available for a given roadway or roadway segment, then a k-factor of 0.10 was assumed. The raw traffic counts are provided in Appendix B.



Figure 16: 2022 Existing Conditions – Roadway Network Geometric Configuration and Traffic Control Devices


Figure 17: 2022 Existing Conditions – Vehicular Traffic Volumes

Intersection capacity and queuing analyses were performed for the 2022 Existing Conditions scenario at the study area intersections during AM and PM peak hours. *Synchro*, version 10, was used to analyze the study intersections with results based on the 2010 methodology¹ presented in the Transportation Research Board's (TRB) <u>Highway Capacity Manual</u> (HCM) and analysis guidelines provided in the City of Manassas' DCSM. The analysis herein includes level of service (LOS), delay, and queue length comparisons for the turning movements analyzed.

Signal timings for Study Intersection 2: Fairview Avenue at Manassas Police Department Entrance/Signal Hill Road and Study Intersection 5: Fairview Avenue at Wellington Road were obtained from the City and were utilized in the analysis models based in *Synchro*. Traffic operation conditions as well as lane configurations were field verified. The existing traffic volumes discussed in the aforementioned section as well as other relevant data were entered into the analysis models. For the purposes of this analysis, the existing peak hour factors (PHF) were based on the traffic count data and utilized on a by-intersection basis; PHF in the range of 0.85 to 1.00 were used for the existing scenario, consistent with the scoping discussions. As discussed in the scope, the heavy vehicle percentages (HV%) are based on the existing counts per movement.

Per the scoping discussions with City staff, it would be considered acceptable and/or desirable to achieve an approach LOS D or better for traffic operations using HCM methodology. The results of the intersection capacity analyses from *Synchro* are presented in Table 8 and graphically in Figure 18. The results are expressed in LOS and delay (seconds per vehicle) for overall signalized intersections and per approach and lane group for all study intersections. The overall signalized intersections, and any approaches, that operate at LOS E, or F are displayed in red.

The 95th percentile queue lengths were also determined from *Synchro* and are expressed in feet. The lane groups where the queue lengths exceeded the available effective storage capacity of existing turn lanes are displayed in red.

The description of different LOS and delay are included in Appendix D. The detailed analysis worksheets of 2022 Existing Conditions are contained in Appendix E.

Based on the capacity analysis of existing conditions, the signalized study intersections operate at an acceptable overall level of service during both peak hours. Similarly, the remaining existing study intersections operate at acceptable approach levels of service during both peak hours, except for the westbound approach of Study Intersection 4: Fairview Avenue at Richmond Avenue, which operates at an unacceptable level of service during the PM peak hour only.

Based on the queuing analysis performed for existing conditions, all turning movements at the study intersections had 95th percentile queue lengths that were accommodated within the available storage lengths of the turn bays.

¹ It should be noted that HCM 2000 methodology was used in lieu of HCM 2010 if the HCM 2010 methodology was not applicable. HCM 2010 could not be applicable in such cases as nonstandard National Electrical Manufacturers Association (NEMA) configurations, shared lane configurations, placement of loop detectors, U-turns, etc.

Table 8: 2022 Existing Co	onditions – Intersection (Capacity and Q	ueuing Analys	is Results
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No. Intersection (Movement) Efficiency bit room (in the control of th					AM Peak Ho	ur	PM Peak Hour			
Index Lane (EW) & Faiview Avenue (NS) Synchro Synchro 1 Tudor Lane (EW) & Faiview Avenue (NS)	No.	Intersection (Movement)	Effective Storage Length (ft.)	LOS	Delay (sec/veh)	95th % Queue (ft.) [7]	LOS	Delay (sec/veh)	95th % Queue (ft.)	
1 Tutor Lare (EM) Fairview Avenue (WS) Overall Interaction (WS) Eastbourd Approach C 200 C 20.1 Eastbourd Approach C 20.0 C 20.1 50.0 Eastbourd Approach C 16.2 B 10.7 0. Westbourd Approach C 16.2 B 10.7 0. Nonthbourd Approach C 16.2 B 10.7 0. Southbourd Approach 20.0 A 8.7 2.3 A 9.3 15. Southbourd Approach A 8.4 0 A 8.0 0 2 Massess PD Entry Statistical C 20.3 B 17.7 13. Coverall Intersection (Spinalized) C 20.4 4.4 15 D 37.7 13. Westbourd Approach C 20.4 4.4 5 D 37.7 13. Westbourd Approach C 20.4 C 21.8 6.3 16.5 15.5 15.5		the second s			Synchro			Synchro		
Eastbourd Approach C 200 C 20.1 Eastbourd Approach C 162 B 167 20.1 50. Westbourd Approach C 162 3 B 107 0. Northbourd Approach C 162 3 B 107 0. Northbourd Approach C 162 3 B 107 0. Southbourd Approach A 8.7 23 A 9.3 15. Southbourd Approach D 414 D A 8.0 0 Parview Arenne (NS) D 414 D 37.7 13. 0. Eastbourd LeftThru C 30.4 4.3 C 28.6 53. Vestbourd Approach C 20.4 4.4 D 3.7 13. Vestbourd LeftThru C 30.4 4.3 C 28.6 53. Vestbourd Approach C 26.0 0 21.8 29	1	Tudor Lane (E/W) & Fairview Avenue (N/S) Overall Intersection (TWSC)								
Eastbound Left/ThursRight C 200 50 C 201 50 Westbound Left/ThursRight C 16.2 3 B 10.7 0 Northbound Left 280 A 8.7 23 A 9.3 15 Southbound Approach A 8.4 0 A 8.0 0 2 Manassa DE Dtr. / Signal Hill Rood (EW) & A 8.4 0 A 8.0 0 2 Manassa DE Dtr. / Signal Hill Rood (EW) & C 20.3 B 17.9 C 26.6 17.9 C 26.6 17.7 13 Westbound Left/Thur Kight D 41.4 10 27.7 13 Westbound Left/Thur Kight 110 C 30.9 C 26.6 14.6 63 14.6 63 14.6 63 14.6 63 15.7 13 Westbound Approach C 20.4 4.6 16.7 16.5 12.8 27.1 13 16.6 16.7 16.6 <		Eastbound Approach		С	20.0		С	20.1		
Westbound Approach C 162 B 10.7 Westbound Approach C 162 3 B 10.7 0. Northbound Approach C 162 3 B 10.7 0. Southbound Approach C 162 A 8.7 23 A 9.3 15. Southbound Approach A 8.4 0 A 8.0 0 Coverail Intersection (Signalized) C 20.3 B 17.9 12. Eastbound Approach D 41.4 D 37.7 13. 13. Westbound Approach C 20.9 C 22.6 5.3 Westbound Approach C 20.4 4.3 C 22.6 5.3 Northbound Approach C 22.4 22.4 C 21.8 20. Vestbound Left Thu C 26.0 A 9.7 9.4 9.5 12.3 Southbound Indicigit 26.0 A		Eastbound Left/Thru/Right		Ç	20.0		¢			
Westbound Left Thur Right C 16.2 3 B 10.7 0 Northbound Approach 280 A 8.7 23 A 9.3 15 Southbound Left Thur Right A 8.4 0 A 8.0 0 2 Manass PD Entr / Signal Hall Road (E/W & A 8.4 0 A 8.0 0 Correal Intersection (Signalized) C 20.3 B 17.9 D 57.7 13 Westbound Left/Thur Right D 41.4 15 D 37.7 13 Westbound Right 110 C 30.4 4.3 C 28.6 53 Westbound Left/Thur C 20.9 7.5 B 14.6 63 Northbound Left 260 0 0.0 0 B 11.6 23 22.1 3 Southbound ThurRight 260 A 5.7 89 A 9.5 12.3 22.1 3 So		Westbound Approach		С	16.2		B	10.7		
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Northbound Left Thur Right 280 A 8.7 23 A 9.3 15 Southbound Left Thur Right A 8.4 0 A 8.0 0 2 Manassa PD Entr / Signal Neil Rood (EM) & Events Avenue (N)S C 20.3 B 17.9 Coverall Intersection (Signalized) C 20.3 B 17.9 Eastound Approach D 41.4 D 37.7 Westbound Heft Thur Westbound Heft Thur Westbound Heft Thur Westbound Heft Thur Westbound Heft Thur Southbound Left Thur Westbound Hught C 30.4 4.3 C 28.6 5.3 Northbound Left Thur Westbound Thur Southbound Approach C 20.4 4.8 1.6 0 Northbound Approach C 20.4 8 11.5 20 0 0 0 12.1 28.2 27.1 28.2 27.1 28.2 27.1 28.2 27.1 28.2 27.1 28.2 27.1 28.1 28.1 28.1 28.2 27.1 28.2 27.1 28.2 27		Northbound Approach								
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Orthom Explosion C AD D H14 D D17.7 Eastbound Laf, ThruRight D 414 D 37.7 13 Westbound Laf, ThruRight D 414 D 37.7 13 Westbound Left Thru C 30.9 C 28.6 53 Northbound Right 110 C 30.9 75 B 14.6 53 Northbound Right 110 C 30.9 75 B 14.6 53 Northbound Right 260 0 0 B 14.6 0 21.8 232 Southbound Left 260 A 8.1 8 11.5 52.123 Southbound ThruRight 260 A 8.1 8 13.7 23 Southbound Left 260 A 8.1 13.7 23 213 Westownul Left ThruRight C 15.2 3 C 17.8 0 Southbound Left ThruRight C </td <td>2</td> <td>Manassas PD Entr / Signal Hill Road (E/W) & Fairview Avenue (N/S) Overall Interaction (Sanalized)</td> <td></td> <td>~</td> <td>20.2</td> <td>_</td> <td></td> <td>170</td> <td></td>	2	Manassas PD Entr / Signal Hill Road (E/W) & Fairview Avenue (N/S) Overall Interaction (Sanalized)		~	20.2	_		170		
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Eastbound Approach C 16.0 C 16.0 3 C 16.0 16.0 3 16.0 3 16.0 16.0 16.0 16.0 16.0 16.0 16.0 16.0 16.0 16.0 16.0 16.0 16.0 16.0 16.0 16.0 16.0 16.0	4	Richmond Avenue (E/W) & Fairview Avenue (N/S) Overall Intersection (TWSC)								
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Westbound Approach C 22.4 E 48.3 115 Northbound Approach C 22.4 38 E 48.3 115 Northbound Approach A 8.0 0 A 8.2 0 Southbound Approach A 9.0 0 A 8.2 0 Southbound Left/Thru/Right A 9.0 0 A 8.5 3 5 Wellington Road (E/W) & Fairview Avenue (N/S) B 11.7 B 12.5 Eastbound Left B 11.7 B 12.5 5 Eastbound Approach A 6.0 A 5.6 Eastbound Left 225 A 8.1 73 A 7.7 93 Eastbound Left 225 A 8.1 73 B 13.1 124 Westbound Approach B 14.5 97 B 13.1 124 Westbound Approach C 24.8 C 33.6 21 <td></td> <td>Eastbound Left/Thru/Right</td> <td></td> <td>Q</td> <td></td> <td></td> <td>Ç</td> <td>16.0</td> <td></td>		Eastbound Left/Thru/Right		Q			Ç	16.0		
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Northbound Approach A 8.0 A 8.2 0 Southbound Approach A 9.0 A 8.2 0 Southbound Approach A 9.0 0 A 8.5 3 5 Wellington Road (E/W) & Fairview Avenue (N/S) B 11.7 B 12.5 Overall Intersection (Signalized) B 11.7 B 12.5 Eastbound Approach A 6.0 A 5.6 Eastbound Left 225 A 8.1 7.3 A 7.7 93 Eastbound Approach B 13.5 B 11.4 Westbound Approach B 13.5 B 11.4 Westbound Approach B 14.5 97 B 13.1 124 Westbound Right 235 B 12.4 27 A 8.6 21 Southbound Approach C 24.8 C 33.6 21 Southbound Right 0.0 0 0 0 0 0		Westbound Left/Thru/Right		Ç			Ę			
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Southbound Approach Southbound Left/Thru/Right A 9.0 A 8.5 3 5 Wellington Road (E/W) & Fairview Avenue (N/S) Overall Intersection (Signalized) B 11.7 B 12.5 Eastbound Approach A 6.0 A 56 Eastbound Approach B 11.7 B 12.5 Eastbound Approach A 6.0 A 56 Eastbound Approach B 13.5 B 11.4 Westbound Approach B 13.5 B 11.4 Westbound Approach B 14.5 97 B 13.1 124 Westbound Right 235 B 12.4 27 A 8.6 21 Southbound Approach C 24.8 C 33.6 21 Southbound Right 0.0 0 A 0.0 0 0		Northbound Left/Thru/Right		A	<u>8.0</u>	9	A		Q	
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5 Weilington Road (20%) & Fairliew Avenue (r05) Overall Intersection (Signalized) B 11.7 B 12.5 Eastbound Approach A 6.0 A 5.6 Eastbound Left 225 A 8.1 73 A 7.7 93 Eastbound Thru A 4.4 42 A 4.5 84 Westbound Approach B 13.5 B 11.4 Westbound Right 235 B 12.4 27 A 8.6 21 Southbound Approach C 24.8 C 33.6 21 33.6 33.6 34		Southbound Left I hru/Kight		<u>A</u>	9.0	0	A	8.5	3	
B 11.7 B 12.5 Eastbound Approach A 6.0 A 5.6 Eastbound Left 225 A 8.1 73 A 7.7 93 Eastbound Left 225 A 8.1 73 A 7.7 93 Westbound Approach B 13.5 B 11.4 Westbound Approach B 14.5 97 B 13.1 124 Westbound Right 235 B 12.4 27 A 8.6 21 Southbound Approach C 24.8 C 33.6 94 Southbound Right A 0.0 0 A 0.0 0	5	Wellington Road (E/W) & Fairview Avenue (N/S)		P				125		
A 0.0 A 5.0 Eastbound Left 225 A 8.1 73 A 7.7 93 Eastbound Thru A 4.4 42 A 4.5 84 Westbound Approach B 13.5 B 11.4 Westbound Right 235 B 124 27 A 8.6 21 Southbound Approach C 24.8 C 33.6 94 Southbound Right A 0.0 0 0 0 0 0		Eastbound Anornach		0	60			56		
Lastbound Lin 225 A 6.1 7.3 A 7.1 93 Eastbound Thru A 4.4 42 A 4.5 84 Westbound Approach B 13.5 B 11.4 93 Westbound Thru B 14.5 97 B 13.1 124 Westbound Right 235 B 12.4 27 A 8.6 21 Southbound Lift C 24.8 C 33.6 94 Southbound Right A 0.0 0 A 0.0 0		Easthound Lat	225	A	0.0	72	4	7.7	0.2	
Westbound Approach B 13.5 B 11.4 Westbound Thru B 14.5 97 B 13.1 124 Westbound Right 235 B 12.4 27 A 8.6 21 Southbound Left C 24.8 C 33.6 5 <		Easthound Thru	225	~	0.1	13	~	1.0	93	
Westbound Thru B 14.5 97 B 13.1 124 Westbound Right 235 B 12.4 27 A 8.6 21 Southbound Left C 24.8 C 33.6 54 Southbound Right A 0.0 0 A 0.0 0		Westhound Anomach			125					
Westbound Right 235 B 12.4 27 A 8.6 21 Southbound Approach C 24.8 C 33.6 C 33.6 94 Southbound Right A 0.0 0 A 0.0 0		Westbound Thru		8	14.6	97	B	13.4	124	
Southbound Approach C 24.8 C 33.6 Southbound Left C 24.8 46 C 33.6 94 Southbound Right A 0.0 0 A 0.0 0		Westbound Right	235	R	12.4	27	4	8.6	21	
Southbound Left C 24.8 46 C 33.6 94 Southbound Right A 0.0 0 A 0.0 0		Southbound Approach	235	C	24.8		ĉ	33.6	21	
Southbound Right A 0.0 0 A 0.0 0		Southbound Left		C	24.8	46	č	33.6	9.4	
		Southbound Right		A	0.0	0	A	0.0	0	

NOTES:

[1] Effective storage length is based on the storage length plus one-half of the taper length per TOSAM guidelines.
 [2] #: 95th percentile queues (reported from Synchro) exceed capacity; actual queues may be longer. Queues shown are based on the maximum after two cycles.



Figure 18: 2022 Existing Conditions – Level of Service Results

Analysis of 2027 Future Conditions without Development

For the purposes of this study, the Edugenius Development in full is anticipated to be constructed by 2027, as ultimately determined following distribution of the scoping document; this scenario analyzes the future without development conditions for the year 2027.

Future without Development Traffic Volumes

The derivation of future without development traffic volumes was based on assumptions and parameters discussed with the City during the scoping process for this report. The future conditions include anticipated inherent regional growth, the inclusion of any potential background developments around the vicinity of the site, and any anticipated roadway improvements.

Inherent Regional Growth

The Edugenius Development in full is anticipated to be complete in 2027, as ultimately determined following distribution of the scoping document.

In order to account for increased demand on the traffic network between 2022 and 2027, an inherent growth rate was applied to the future scenarios. This "inherent" growth was anticipated to account for regional development within the at-large area, which would ultimately result in increased roadway demand. Furthermore, the inherent growth was anticipated to account for any potential background developments unaccounted for within the vicinity of the study area. According to historical VDOT AADT data (as shown in Table 9), there has been slight to moderate growth along the major road network within the study area in recent years.

Table 9: Historical Regional Growth within Vicinity of the Road Network

				<u>Publis</u>	hed VDOT A	ADT			Growth R	late / Year	
Road Segment	From	То	2015	2016	2017	2018	2019	2015- 2019	2016- 2019	2017- 2019	2018- 2019
Fairview Avenue	Wellington Ave	SR 28 Center St	13,000	13,000	13,000	13,000	13,000	0%	0%	0%	0%
Wellington Rd	BUS SR 234 Dumfries Rd	Fairview Ave	14,000	16,000	17,000	16,000	18,000	6%	4%	3%	13%
Signal Hill Rd	Richmond Ave	Liberia Ave	6,200	6,100	6,200	6,000	6,100	0%	0%	-1%	2%
Tudor Ln	Osbourn High School	Cul de sac	2,500	2,500	2,500	2,500	2,500	0%	0%	0%	0%
Richmond Avenue	Fairview Ave	Liberia Ave	2,700	3,300	3,400	3,300	3,500	7%	2%	1%	6%

As agreed upon in the scope for this study, to account for 2027 future conditions (as ultimately determined following distribution of the scoping document), an inherent growth rate of one-percent (1.0%), compounded annually over a three-year period, between 2022 to 2027 (and totaling 5.10% growth of the existing volumes) was applied to all turning movements at Study Intersection 5: Fairview Avenue at Wellington Road. The growth volumes were propagated as mainline through movements at subsequent study intersections along the road network where applicable.

The inherent regional growth volumes (for the period between 2022 and 2027) are illustrated in Figure 19.

Potential Background Development(s)

In addition to the inherent regional growth reflecting increased traffic demand, one (1) background development in the vicinity of the site was identified for inclusion in this study to be conservative under 2027 future conditions as based on discussions during the scoping process with the City (and identified for inclusion in the study following distribution of the scoping document).

The background development includes the potential conversion of the existing City of Manassas Police Department building, located adjacent to Study Intersection 2: Fairview Avenue at Manassas Police Department/Signal Hill Road, into City government offices. This conversion is referenced and recommended per Chapter 7 of the City's Comprehensive Plan. Of note, per the City's 2023 Five-Year Capital Improvement Program, the Police Department functions will be relocated to a planned public safety facility along Grant Avenue, west of the site.

Given that the police department building is currently operating under existing conditions, trips associated with the use are included in the existing counts. The police station trips are found to be similar and consistent with trips for a similar use under

ITE's <u>Trip Generation Manual</u>, 11th edition, publication. Therefore, with the proposed higher density under the conversion to City government offices, the delta, or difference, in trips between the existing use and proposed use was added to the road network, under a routing consistent with the direction of approach for the proposed Edugenius Development, and as based on existing traffic flows and engineering judgment. The delta in trips for the background development is depicted in Table 10.

Table 10: Police Station to City Government Offices Conversion – Site Trip Generation (Difference in Trips from Existing Conditions)

					W	e e k d	ay		
Land Use	ITE Code	Size		AM Peak	Hour	Р	M Peak I	Hour	Daily
			In	Out	Total	In	Out	Total	Total
Proposed Use									
Government Office Building	730	36.376 kSF of GFA	91	30	121	15	47	62	822
Existing Use									
Police Station ^{[1][2]}	575	36.376 kSF of GFA	12	5	17	5	12	17	170
Notes									

[1]: As no LUC exists for Police Station in ITE 11, the similar ITE LUC 575 (Fire and Rescue Station) was used. The associated trips are consistent with existing conditions in the field. [2]: ITE LUC 575 (Fire and Rescue Station) does not have data for the AM peak hour nor weekday daily period in ITE 11. The AM peak hour was assumed to be similar to the PM peak hour while the weekday daily trips were assumed to be 10 times the PM peak hour. The associated trips are consistent with existing conditions in the field.

Difference (Proposed Minus Existing)	79	25	104	10	35	45	652

The assignment of the delta in trips for the background development is illustrated in Figure 20.

Potential Roadway Improvement(s)

No major roadway improvements, near or within the vicinity of the site, were anticipated to significantly affect future roadway conditions on the road network by 2027, nor were any identified in the scoping discussions for inclusion in this study under the 2027 Future Conditions without Development scenario. As such, no roadway improvements were included in the 2027 future scenarios. The 2027 Future Conditions without Development road network configuration will remail the same as the existing network configuration (Figure 16). Of note, a City-wide signal optimization was recently completed.

Future without Development Traffic Volumes

In order to forecast future roadway traffic volumes for the year 2027, the 2022 existing traffic volumes were combined with the inherent growth traffic volumes and background site traffic (difference from existing conditions). The 2027 Future Conditions without Development traffic volumes are illustrated in Figure 21.



Figure 19: Projected Inherent Regional Growth Traffic Volumes (2022 to 2027)



Figure 20: Background Development Traffic Assignment – Police Station to City Government Offices Conversion (Difference in Trips from Existing Conditions)



Figure 21: 2027 Future Conditions without Development – Vehicular Traffic Volumes

Future without Development Intersection Capacity and Queueing Analysis

Intersection capacity analyses were performed for the 2027 Future Conditions without Development scenario at the study area intersections during the AM and PM peak hours. *Synchro*, version 10, was used to analyze the study intersections with results based on the methodology presented in TRB's HCM and analysis guidelines provided in the City of Manassas' DCSM. The analysis herein includes LOS, delay, and queue length comparisons for the turning movements analyzed.

For the purposes of this analysis, the future PHF were based on the traffic counts and utilized on a by-intersection basis; PHF in the range of 0.92 to 1.00 were used for the future scenario, consistent with the agreed-upon scope. The heavy vehicle percentages (HV%) were the same as those used in the 2022 Existing Conditions scenario analysis.

Per the scoping discussions with City staff, it would be considered acceptable and/or desirable to achieve an approach and a lane group LOS D or better for traffic operations using HCM methodology. The results of the intersection capacity analyses from *Synchro* are presented in Table 11 and graphically in Figure 22. The results are expressed in LOS and delay (seconds per vehicle) for overall signalized intersections and per approach and lane group for all study intersections. The overall signalized intersections, and any approaches, that operate at LOS E, or F are displayed in red.

The 95th percentile queue lengths were also determined from *Synchro* and are expressed in feet. The lane groups where the queue lengths exceeded the available storage lengths of turn lanes are displayed in red.

The detailed analysis worksheets of the 2027 Future Conditions without Development are contained in Appendix F.

Based on the capacity analysis of future without development conditions, the signalized study intersections are anticipated to continue to operate at an acceptable overall level of service during both peak hours. Similarly, the remaining study intersections are anticipated to continue to operate at acceptable approach levels of service during both peak hours, except for the westbound approach of Study Intersection 4: Fairview Avenue at Richmond Avenue, which is anticipated to continue to operate at an unacceptable level of service (dropping from LOS E to LOS F under this scenario) during the PM peak hour only.

Based on the queuing analysis performed for this scenario, all turning movements at the study intersections are anticipated to continue to have 95th percentile queue lengths that would continue to be accommodated within the available storage lengths of the turn bays.

				AM Peak Ho	ur		PM Peak Ho	Jr
		Effective Storage	LOS	Delay	95th %	LOS	Delay	95th %
No.	Intersection (Movement)	Length (ft.)		(sec/veh)	Queue (ft.)		(sec/veh)	Queue (ft.)
		[1]			[2]			[2]
				Synchro			Synchro	
1	Tudor Lane (E/W) & Fairview Avenue (N/S)							
	Overall Intersection (TWSC)							
	Eastbound Approach		С	23.3		С	22.0	
	Eastbound Left/Thru/Right		С	23.3	70	С	22.0	58
	Westbound Approach		С	17.3		В	11.0	
	Westbound Left/Thru/Right		С	17.3	3	В	11.0	0
	Northbound Approach	000						10
		280	A	8.9	23	A	9.4	18
	Southbound Approach		^	0 5	0	^	0.1	0
2	Manassas PD Entr / Signal Hill Road (E/W) &		A	0.5	0	A	0.1	0
-	Fairview Avenue (N/S)							
	Overall Intersection (Signalized)		с	21.9		в	19.4	
	Eastbound Approach		D	45.3		D	41.4	
	Eastbound Left/Thru/Right		D	45.3	34	D	41.4	36
	Westbound Approach		D	36.0		С	31.3	
	Westbound Left/Thru		D	36.1	55	С	31.2	56
	Westbound Right	110	D	36.0	75	В	15.3	63
	Northbound Approach		С	21.2		С	23.4	
	Northbound Left	260	В	12.5	6	В	15.3	3
	Northbound Thru/Right		С	21.2	#494	С	23.4	309
	Southbound Approach		В	10.4		В	12.2	
	Southbound Left	260	Α	10.0	90	В	10.3	123
	Southbound Thru/Right		В	10.6	176	В	13.4	300
3	Wesley Avenue (E/W) & Fairview Avenue (N/S)							
	Overall Intersection (TWSC)							
	Eastbound Approach		С	17.2		В	14.7	
	Eastbound Left/Thru/Right		С	17.2	3	В	14.7	0
	Westbound Approach		C	15.8		C	19.6	
	Westbound Left/Inru/Right		C	15.8	0	C	19.6	0
	Northbound Approach			7.0	0			0
	Northbound Lett/Inru/Right		A	7.8	0	A	8.4	0
	Southbound Approach		^	0.6	0	^	0.1	0
-	Bichmond Avenue (FM) & Fairview Avenue (N/S)		A	0.0	0	A	0.1	0
-	Overall Intersection (TWSC)							
	Fastbound Approach		c	17 9		C	173	
	Eastbound Left/Thru/Right		C C	17.0	3	C C	17.3	3
	Westbound Approach		D	27.0	0	F	67.5	
	Westbound Left/Thru/Right		D	27.0	50	F	67.5	143
	Northbound Approach		_			-		
	Northbound Left/Thru/Right		А	8.1	0	А	8.3	0
	Southbound Approach				-			
	Southbound Left/Thru/Right		А	9.3	3	А	8.7	3
5	Wellington Road (E/W) & Fairview Avenue (N/S)							
	Overall Intersection (Signalized)		в	12.2		в	12.9	
	Eastbound Approach		Α	6.3		Α	6.0	
	Eastbound Left	225	А	8.6	85	А	8.4	101
	Eastbound Thru		А	4.5	46	А	4.8	91
	Westbound Approach		В	14.1		В	11.8	
	Westbound Thru		В	14.8	105	В	13.6	132
	Westbound Right	235	В	13.3	40	Α	8.8	26
	Southbound Approach		С	24.8		С	33.2	
	Southbound Left		С	24.8	48	С	33.2	102
1	Southbound Right		А	0.0	0	А	0.0	0

NOTES:

[1] Effective storage length is based on the storage length plus one-half of the taper length per TOSAM guidelines.

[2] #: 95th percentile queues (reported from Synchro) exceed capacity; actual queues may be longer. Queues shown are based on the maximum after two cycles.



Figure 22: 2027 Future Conditions without Development – Level of Service Results

Analysis of 2027 Future Conditions with Development

For the purposes of this study, the Edugenius Development in full is anticipated to be constructed and in operation by 2027, as ultimately determined following distribution of the scoping document; this scenario analyzes the future with development conditions for the year 2027.

Site Description

The Applicant is proposing to redevelop the site with an approximately 120-student capacity elementary school and 125-student capacity day care / childcare facility. A conceptual plan for the Site is illustrated in Figure 23.



Figure 23: Conceptual Development Plan For conceptual purposes only.

Proposed Site Access

Currently, it is anticipated that the Edugenius Development will be accessed via one full-movement entrance along Wesley Avenue and one full-movement entrance along with one-partial movement entrance along Fairview Avenue.

For Study Intersection 7: Fairview Avenue at Site Entrance 2, it is anticipated that, due to its proximity to the existing Study Intersection 3: Fairview Avenue at Wesley Avenue, it will be preferred by City staff to not allow for left-in movements at this location; Therefore, it is anticipated that users of the site (employees and individuals dropping off/picking up students and/or children) will be informed not to turn in from the left at this site entrance.

Proposed Site Circulation

Currently, the Site is planned to consist of both an elementary school building and a separate child care center building on the same parcel. In order to consider the anticipated vehicular circulation for the Site, and identify any potential conflicts, this subsection discusses the conceptual layout of the Site in more detail, drop off and pick up times (and implied hours of operation) for each facility and different anticipated user groups for each, and their respective arrival and departure times. Of note, all of these items are subject to change during the Application process.

As per Figure 23, a parking lot and cul-de-sac in the northern section of the Site will serve the elementary school faculty and staff, and pick up and drop off operations for the students, while a parking lot in the southern section of the Site will serve the child care facility faculty and staff, and pick up and drop off operations for the children. An on-site driveway will connect these two parking lot areas (and Study Intersection 6 and Study Intersection 7), while the child care center parking lot will also be served by Study Intersection 8 along Wesley Avenue; the multiple entrances to the site will be interconnected.

The elementary school portion of the Site is anticipated to have drop off times of 7:00 AM to 8:00 AM in the morning during the weekdays (non-holiday), and pick up beginning at 3:00 PM in the afternoon during the weekdays (non-holiday), running up until 6:00 PM, including after-school care and planned activities. Of note, these anticipated drop off and pick up times are earlier than the respective hours for the surrounding elementary schools in the vicinity of the Site, including the nearby Baldwin Elementary School along Prince William Street / Tudor Lane.

The child care center portion of the Site is anticipated to have drop off times of 6:30 AM to 9:00 AM in the morning during the weekdays, and the latest pick up time of 6:00 PM in the afternoon during the weekdays. These anticipated drop off and pick up times are consistent with surrounding child care centers in the vicinity of the Site.

The user groups include faculty and staff for both the elementary school and child care center, respectively, who would arrive prior to students and children of the respective buildings. For the elementary school, following the arrival of faculty and staff, those students being dropped off would be anticipated to arrive next. Similarly, the child care center would have a drop off period for the children (over a longer period) following arrival of staff.

For the afternoon, it is anticipated that elementary school students would be released first, utilizing pick up lines to depart. Pick up times for the child care center users would occur over a longer period (up until 6:00 PM) and generally apart from the anticipated general release of elementary school students. It should be noted that after school pick up hours for the school would continue up until 6:00 PM. Remaining faculty and staff would then be anticipated to be the last ones to leave each weekday.

It should be noted that there would be split vehicular traffic flows generally serving the elementary school and child care facility separately, and that the projected site trip generation, discussed in the next subsection, considers the maximum number of trips of each use, spread across a peak hour. During the AM peak hour, both the elementary school and child care center, on average, each would be anticipated to have a car enter or exit once every 39 seconds between each car over the hour. During the PM peak hour, while the child care center, on average, would be anticipated to have a car enter or exit once every 39 seconds between each car over the hour. During the PM peak hour, while the child care center, on average, would be anticipated to have a car enter or exit once every 40 seconds between each car over the hour (similar to AM peak hour), the elementary school, on average would be anticipated to have a car enter or exit once every two minutes (116 seconds) between each car over the hour. All other hours of the day would be expected to have even longer time spans between each vehicle.

Thus, while some overlap may occur between the uses of the Site in terms of vehicular site circulation, the user groups will be spread out over the morning and afternoon periods, and the AM and PM peak hours will not be anticipated to experience a high number of vehicles arriving and departing during the respective hour. Therefore, that these user groups would be spread across the multiple access points, no significant issues are expected with the proposed Site circulation.

A graphic of the site circulation is included in Figure 24.



Figure 24: Proposed Site Circulation For conceptual purposes only.

Projected Site Trip Generation

In order to calculate the total trips generated by the proposed Edugenius Development, the Institute of Transportation Engineers' (ITE) <u>Trip Generation Manual</u>, 11th Edition, publication was used to determine the total trips going into and out of the subject study site during the AM and PM peak hours as well as the typical number of weekday daily trips. The projected trip generation for the proposed Edugenius Development is depicted in Table 12.

As per the scoping discussions, a pass-by trip reduction of 44% during the PM peak hour and of an assumed 34% during the AM peak hour and weekday daily period were included in the trip generation for the trips generated by the Day Care Center (ITE Land Use Code 565); these pass-by trip reduction rates are based on the ITE <u>Trip Generation Manual</u>, 11th Edition, publication. Of note, this publication does not include pass-by trip reduction rates for elementary school/private school uses; therefore, and as agreed to in the scoping discussions, no pass-by trip reductions were included for the elementary/private school use. No other trip reductions were included in the trip generation.

Table 12: Projected Site Trip Generation (Peak Hour of the Adjacent Street; ITE 11th Ed.)

Contraction of the second s					-	— W.	eekd	ay -	-	
Land Use	ITE Code Size		AM Peak Hour			PM Peak Hour			Daily	
and the second se			1.000	In	Out	Total	In	Out	Total	Total
Proposed Development			and the second second				-			
Private School (K-8)		530	120 Students	52	40	92	14	17	31	493
Day Care Center		565	125 Students	48	43	91	42	47	89	492
	Pass-By Reduction*		34% / 44%	-16	-15	-31	-19	-21	-40	-167
	Subtotal	without Re	ductions / Driveway Trips	100	83	183	56	64	120	985
and the second second second		Net	new site trips (Proposed)	84	68	152	37	43	80	818

*Pass-By reduction for ITE LUC 565 for the PM peak hour was based on ITE's TGM 11th ED (44%). No data was available for the AM and Daily periods, as such 34% was assumed (PM minus 10%).

As illustrated in the table above, the Site is expected to generate approximately 152 new total trips during the AM peak hour, 80 new total trips during the PM peak hour, and 818 new total daily trips on a typical weekday, after reductions.

Distribution and Assignment of Site Traffic

The distribution and assignment of the site generated trips were based on the existing traffic patterns, engineering judgement, the nature of the proposed Edugenius Development, and with the guidance and input from City staff. The site direction of approach for the peak hour trips is presented in Figure 25.

Using the direction of approach, the nature of the proposed Edugenius Development, and the location of the proposed site entrance per current plans for the Edugenius Development, the site generated trips were assigned to the road network, as illustrated in Figure 26. The associated pass-by trip assignment is illustrated in Figure 27.



Figure 25: Global Vehicular Direction of Approach (Site Trip Distribution)



Figure 26: Site Generated Trip Assignment



Figure 27: Associated Pass-by Trip Assignment

Future with Development Traffic Volumes

The future with development traffic volumes were determined by adding the assigned site generated traffic volumes to the 2027 Future without Development traffic volumes. The proposed future lane configuration is illustrated in Figure 28. The 2027 Future Conditions with Development vehicular traffic volumes are depicted in Figure 29.



Figure 28: 2027 Future Conditions with Development – Proposed Roadway Network Geometric Configuration and Traffic Control Devices



Figure 29: 2027 Future Conditions with Development – Vehicular Traffic Volumes

Future with Development Intersection Capacity and Queueing Analysis

Intersection capacity analyses were performed for the 2027 Future Conditions with Development scenario at the study area intersections during the AM and PM peak hours. *Synchro*, version 10, was used to analyze the study intersections with results based on the methodology presented in the TRB's HCM and analysis guidelines provided in City of Manassas' DCSM. The analysis herein includes LOS, delay, and queue length comparisons for the turning movements analyzed.

For the purposes of this analysis, the future PHF were based on the traffic counts and utilized on a by-intersection basis; PHF in the range of 0.92 to 1.00 were used for the future scenario, consistent with the agreed-upon scope. The heavy vehicle percentages (HV%) were the same as those used in the 2022 Existing Conditions and 2027 Future Conditions without Development scenario analysis.

The results of the intersection capacity analyses from *Synchro* are presented in Table 13 and graphically in Figure 30. The results are expressed in LOS and delay (seconds per vehicle) for overall signalized intersections and per approach and lane group for all study intersections. The overall signalized intersections, and any approaches, that operate at LOS E, or F are displayed in red.

The 95th percentile queue lengths were also determined from *Synchro* and are expressed in feet. The lane groups where the queue lengths exceeded the available storage lengths of future turn lanes are displayed in red.

The detailed analysis worksheets of the 2027 Future Conditions with Development are contained in Appendix G.

As mentioned previously, it would be considered acceptable and/or desirable to achieve an approach and a lane group LOS D or better for traffic operations using HCM methodology. Furthermore, if an overall signalized, or any approach, was operating at an unacceptable LOS under future without development conditions, it was assumed acceptable to achieve non-degradation.

Based on the capacity analysis of future with development conditions, with the addition of Site traffic volumes and associated pass-by volumes, the signalized study intersections are anticipated to continue to operate at an acceptable overall level of service during both peak hours. Similarly, the remaining study intersections are anticipated to continue to operate at acceptable approach levels of service during both peak hours, except for the westbound approach of Study Intersection 4: Fairview Avenue at Richmond Avenue, which is anticipated to continue to operate at an unacceptable level of service during the PM peak hour only. It should be noted that this approach's result is an existing and future without development condition, with which the Edugenius Development adds less than one car length of queue added compared to future conditions without development; moreover, the Edugenius Development generally only adds mainline through traffic at the intersection. As previously mentioned, raised curb bump outs and a raised median are in place on the east leg of the intersection.

Based on the queuing analysis performed for this scenario, all turning movements at the study intersections are anticipated to continue to have 95th percentile queue lengths that would continue to be accommodated within the available storage lengths of the turn bays. While the northbound through/right lane group of Study Intersection 2: Fairview Avenue at Manassas Police Department Entrance / Signal Hill Road is anticipated to have 95th percentile queue lengths that would spill past the site entrances on Fairview Avenue during both peak hours, this result is an existing condition, and compared to future conditions without development, the Site adds approximately two standard car lengths or less during either peak hour. Of note, Site users have an alternative entrance/exit along Wesley Avenue which could accommodate additional volumes per the analysis results.

The site entrances are anticipated to operate at acceptable levels of service during both peak hours, with 95th percentile queue lengths anticipated to be less than one standard car length during either peak hour. There is no anticipated queue overlap between Study Intersection 6, Study Intersection 7, nor Study Intersection 3 (nor Study Intersection 8) under these conditions (the aforementioned overlap with Study Intersection 2 is described in the previous paragraph).

Based on the capacity and queueing analysis results, the proposed Edugenius Development will not have a substantial impact to the surrounding transportation and roadway network.

				AM Peak Ho	ur	Anaryon	PM Peak Ho	ır
		Effective Storage	1.05	Delay	95th %	1.05	Delay	95th %
No.	Intersection (Movement)	Length (ft.)	200	(sec/veh)	Queue (ft.)	200	(sec/veh)	Queue (ft.)
		[1]			[2]			[2]
				Synchro			Synchro	
1	Tudor Lane (E/W) & Fairview Avenue (N/S)							
	Overall Intersection (TWSC)							
	Eastbound Approach		D	25.9		С	23.0	
	Eastbound Left/Thru/Right		D	25.9	80	С	23.0	60
	Westbound Approach		С	18.0		В	11.2	
	Westbound Left/Thru/Right		С	18.0	3	В	11.2	0
	Northbound Approach							
	Northbound Left	280	A	9.0	23	A	9.5	18
	Southbound Approach							
	Southbound Len/Inru/Right		A	8.6	0	A	8.2	0
2	Fairview Avenue (N/S)							
	Overall Intersection (Signalized)		с	22.6		C	20.0	
	Eastbound Approach		D	45.4		D	42.6	
	Eastbound U-Turn/Left/Thru/Right		D	45.4	34	D	42.6	36
	Westbound Approach		D	36.0		C	32.6	
	Westbound Left/Thru		D	36.6	67	С	33.3	61
	Westbound Right	110	D	36.0	75	С	32.5	63
	Northbound Approach		С	22.7		С	24.4	
	Northbound Left		В	12.5	6	В	15.5	3
	Northbound Thru/Right		С	22.8	#544	С	24.5	330
	Southbound Approach		В	10.9		В	12.2	
	Southbound Left	260	В	10.8	90	В	10.5	123
	Southbound Thru/Right		В	11.0	198	В	13.1	312
3	Wesley Avenue (E/W) & Fairview Avenue (N/S)							
	Overall Intersection (TWSC)							
	Eastbound Approach		С	15.4		В	14.7	
	Eastbound Left/Thru/Right		С	15.4	5	В	14.7	5
	Westbound Approach		С	17.9		С	21.8	
	Westbound Left/Thru/Right		С	17.9	3	С	21.8	0
	Northbound Approach							
	Northbound Left/Inru/Right		A	7.9	3	A	8.5	3
	Southbound Approach			0.7	0		0.4	0
	Bichmond Avenue (EM) & Fairview Avenue (N/S)		A	8./	0	A	8.1	0
1 7	Overall Intersection (TWSC)							
	Easthound Approach		c	197		C	18.2	
	Eastbound Left/Thru/Right		C C	19.7	3	C C	18.2	3
	Westbound Approach		D	31.9		F	80.8	
	Westbound Left/Thru/Right		D	31.9	60	F	80.8	160
	Northbound Approach							
	Northbound Left/Thru/Right		А	8.2	0	А	8.4	0
	Southbound Approach							
	Southbound Left/Thru/Right		А	9.5	3	А	8.7	3
5	Wellington Road (E/W) & Fairview Avenue (N/S)							
	Overall Intersection (Signalized)		В	12.6		В	13.1	
	Eastbound Approach		Α	6.5		Α	6.2	
	Eastbound Left	225	А	8.9	92	A	8.7	105
	Eastbound Thru		A	4.5	47	A	5.0	93
	Westbound Approach		В	14.5		В	12.0	
	Westbound Thru		В	15.0	106	В	13.9	134
	Westbound Right	235	В	14.0	50	A	8.9	29
	Southbound Approach		С	25.0		С	33.0	
	Southbound Left		С	25.0	53	С	33.0	105
1	Southbound Right		A	0.0	0	A	0.0	0

Table 13: 2027 Future Conditions with Development – Intersection Capacity and Queuing Analysis Results

NOTES:

[1] Effective storage length is based on the storage length plus one-half of the taper length per TOSAM guidelines.

[2] #: 95th percentile queues (reported from Synchro) exceed capacity; actual queues may be longer. Queues shown are based on the maximum after two cycles.

Table 13: 2027 Future Conditions with Development – Intersection Capacity and Queuing Analysis Results (Continued)

Ì				AM Peak Ho	ur		PM Peak Ho	ur
No.	Intersection (Movement)	Effective Storage Length (ft.) ^[1]	LOS	Delay (sec/veh)	95th % Queue (ft.) ^[2]	LOS	Delay (sec/veh)	95th % Queue (ft.) ^[2]
				Synchro			Synchro	
6	Site Entrance 1 (E/W) & Fairview Avenue (N/S) Overall Intersection (TWSC)							
	Eastbound Approach		В	13.2		С	15.1	
	Eastbound Left/Right		В	13.2	5	С	15.1	3
	Northbound Approach							
	Northbound Left/Thru		А	8.0	0	А	8.6	0
7	Site Entrance 2 (E/W) & Fairview Avenue (N/S) Overall Intersection (TWSC)							
	Eastbound Approach		В	14.3		С	16.0	
	Eastbound Left/Right		В	14.3	8	С	16.0	8
8	Wesley Avenue (E/W) & Site Entrance 3 (N/S) Overall Intersection (TWSC)							
	Eastbound Approach							
	Eastbound Left/Thru		А	0.0	0	А	0.0	0
	Southbound Approach		Α	8.7		Α	8.7	
	Southbound Left/Right		А	8.7	3	А	8.7	3

NOTES:

[1] Effective storage length is based on the storage length plus one-half of the taper length per TOSAM guidelines.

[2] #: 95th percentile queues (reported from Synchro) exceed capacity; actual queues may be longer. Queues shown are based on the maximum after two cycles.



Figure 30: 2027 Future Conditions with Development - Level of Service Results

Overall Comparison of Analysis Scenarios

A level of service and delay comparison for all planning scenarios is presented in Table 14. The 95th percentile queue length comparison is presented in Table 15.

Table 14: Intersection Level of Service and Delay Comparison (2027)

			Le	vel of Service	(LOS) (Sec./Ve	h.)	
No.	Intersection (Movement)		AM Peak Hour			PM Peak Hour	
		0000 E	2027 Future	2027 Total		2027 Future	2027 Total
	Tudes Long (EAN) & Estation Anoma (N(C))	2022 Existing	Background	Future	2022 Existing	Background	Future
1	Tudor Lane (E/W) & Fairview Avenue (N/S)						
	Overall Intersection (TWSC)	C (20)	C (22.2)	D (25 0)	C (20 1)	C (22)	C (22)
	Easthound Approach	C(20)	C(23.3)	D (25.9)	C(20.1)	C(22)	C(23)
		C(16.2)	C(17.3)	C (18)	B (10 7)	B (11)	B (11 2)
	Westbound Left/Thru/Right	C (16.2)	C (17.3)	C (18)	B (10.7)	B (11)	B (11.2)
	Northbound Approach	0(10.2)	0 (11.0)	0 (10)	D(10.1)	B(11)	B(11.2)
	Northbound Left	A(8.7)	A(8.9)	A (9)	A(9.3)	A(9.4)	A (9.5)
	Southbound Approach	(- /	()	(-)	(/	(-)	(/
	Southbound Left/Thru/Right	A(8.4)	A (8.5)	A(8.6)	A(8)	A(8.1)	A (8.2)
2	Manassas PD Entr / Signal Hill Road (E/W) & Fairview Avenue (N/S)						
	Overall Intersection (Signalized)	C (20.3)	C (21.9)	C (22.6)	B (17.9)	B (19.4)	C (20)
	Eastbound Approach	D (41.4)	D (45.3)	D (45.4)	D (37.7)	D (41.4)	D (42.6)
	Eastbound U-Turn/Left/Thru/Right	D (41.4)	D (45.3)	D (45.4)	D (37.7)	D (41.4)	D (42.6)
	Westbound Approach	C (30.9)	D (36)	D (36)	C (28.6)	C (31.3)	C (32.6)
	Westbound Left/Thru	C (30.4)	D (36.1)	D (36.6)	C (28.6)	C (31.2)	C (33.3)
	Westbound Right	C (30.9)	D (36)	D (36)	B (14.6)	B (15.3)	C (32.5)
	Northbound Approach	C (22.4)	C (21.2)	C (22.7)	C (21.8)	C (23.4)	C (24.4)
	Northbound Left	A(U)	B (12.5)	B (12.5)	B (14.6)	B (15.3)	B (15.5)
	Northbound Infu/Right	C (22.4)	C (21.2)	C (22.8)	C (21.8)	C (23.4)	C (24.5)
	Southbound Approach	A (8.1)	Δ(10.4) Δ(10)	B (10.9)	D(11.5)	D(12.2) B(10.2)	D(12.2) B(10.5)
	Southbound Thru/Pight	A (9.7)	R(10)	B (10.0)	R(9.5)	B (10.3) B (12.4)	B (10.3) B (12.1)
3	Wesley Avenue (F/W) & Fairview Avenue (N/S)	A(0.7)	D(10.0)	B(11)	D(12.0)	D(13.4)	B(13.1)
l °	Overall Intersection (TWSC)						
	Easthound Approach	C (16.6)	C(172)	C (154)	B(137)	B (14 7)	B (14 7)
	Eastbound Left/Thru/Right	C (16.6)	C (17.2)	C (15.4)	B (13.7)	B (14.7)	B (14.7)
	Westbound Approach	C (15.2)	C (15.8)	C (17.9)	C (17.8)	C (19.6)	C (21.8)
	Westbound Left/Thru/Right	C (15.2)	C (15.8)	C (17.9)	C (17.8)	C (19.6)	C (21.8)
	Northbound Approach	()	. ,	. ,	. ,	. ,	()
	Northbound Left/Thru/Right	A(7.8)	A(7.8)	A(7.9)	A (8.2)	A(8.4)	A (8.5)
	Southbound Approach						
	Southbound Left/Thru/Right	A(8.5)	A (8.6)	A(8.7)	A (8)	A(8.1)	A(8.1)
4	Richmond Avenue (E/W) & Fairview Avenue (N/S)						
	Overall Intersection (TWSC)						
	Eastbound Approach	C (16)	C (17.9)	C (19.7)	C (16)	C (17.3)	C (18.2)
	Eastbound Left/Thru/Right	C (16)	C (17.9)	C (19.7)	C (16)	C (17.3)	C (18.2)
	Westbound Approach	C (22.4)	D (27)	D (31.9)	E (48.3)	F (67.5)	F (80.8)
	Westbound Left/Thru/Right	C (22.4)	D (27)	D (31.9)	E (48.3)	F (67.5)	F (80.8)
	Northbound Approach	A (9)	A (0 1)	A (9.2)	A (0.0)	A (0.2)	A (Q 4)
	Southbound Approach	A(6)	A(0.1)	A(0.2)	A(0.2)	A(0.3)	A (0.4)
	Southbound Left/Thru/Pight	A (Q)	A (0.3)	A (0.5)	A (8 5)	A (8 7)	A (8 7)
5	Wellington Boad (EMI) & Estruiew Avenue (N/S)	A(9)	A(9.3)	A (9.5)	A(0.5)	A(0.7)	A(0.7)
J	Overall Intersection (Signalized)	B (11 7)	B (12 2)	B (12.6)	B (12 5)	B (12 9)	B (13.1)
	Easthound Approach	A (6)	A (6.3)	A (6.5)	A (5.6)	A (6)	A (6.2)
	Eastbound Left	A(8.1)	A (8.6)	A (8.9)	A(7.7)	A(8.4)	A(8.7)
	Eastbound Thru	A(4.4)	A(4.5)	A(4.5)	A(4.5)	A(4.8)	A(5)
	Westbound Approach	B (13.5)	B (14.1)	B (14.5)	B (11.4)	B (11.8)	B (12)
	Westbound Thru	B (14.5)	B (14.8)	B (15)	B (13.1)	B (13.6)	B (13.9)
	Westbound Right	B (12.4)	B (13.3)	B (14)	A (8.6)	A (8.8)	A (8.9)
	Southbound Approach	C (24.8)	C (24.8)	C (25)	C (33.6)	C (33.2)	C (33)
	Southbound Left	C (24.8)	C (24.8)	C (25)	C (33.6)	C (33.2)	C (33)
	Southbound Right	A(0)	A (0)	A (0)	A (0)	A (0)	A (0)
6	Site Entrance 1 (E/W) & Fairview Avenue (N/S)						
	Overall Intersection (TWSC)						
	Eastbound Approach			B (13.2)			C (15.1)
	Eastbound Left/Right			B (13.2)			C (15.1)
	INORTHD OUND APPROACH			A (C)			A (0.0)
<u> </u>	Normbound Lett/Inru			A(8)			A (8.6)
'	Site Entrance 2 (E/W) & Fairview Avenue (NS)						
	Easthound Approach			B (14 2)			C (16)
	Easthound Left/Right			B (14.3) B (14.3)			C (16)
8	Wesley Avenue (FW) & Site Entrance 3 (N/S)			D (14.3)			0(10)
ľ	Overall Intersection (TWSC)						
	Eastbound Approach						
	Eastbound Left/Thru			A(0)			A(0)
	Southbound Approach			A (8.7)			A (8.7)
	Southbound Left/Right			A(8.7)			A (8.7)
Notes:	· · · ·	•		N= 1	•		N= 7

[1]: 2027 Future Background is 2027 Future Conditions without Development scenario, which includes inherent growth plus one background development, in addition to existing conditions.
 [2] 2027 Total Future is 2027 Future Conditions with Development scenario, which includes Future Background conditions plus Edugenius Development.

Table 15: Intersection 95th Percentile Queue Length Comparison (2027)

			· · ·	95th Pe	rcentile Queu	es (ft.)		
No.	Intersection (Movement)	Effective Storage Length (ft.)	2022 Existing	AM Peak Hour 2027 Future Background	2027 Total Future	2022 Existing	PM Peak Hour 2027 Future Background	2027 Total Future
1	Tudor Lane (E/W) & Fairview Avenue (N/S)							
	Eastbound Approach							
	Eastbound Left/Thru/Right		58	70	80	50	58	60
	Westbound Approach Westbound Left/Thru/Right		3	3	3	0	0	0
	Northbound Approach						•	
	Northbound Left	280	23	23	23	15	18	18
	Southbound Left/Thru/Right		0	0	0	0	0	0
2	Manassas PD Entr / Signal Hill Road (E/W) & Fairview Avenue (N/S)							
	Eastbound Approach							
	Eastbound U-Turn/Left/Thru/Right		16	34	34	13	36	36
	Westbound Approach Westbound Left/Thru		43	55	67	53	56	61
	Westbound Right	110	75	75	75	63	63	63
	Northbound Approach		0	0	6	0	2	2
	Northbound Thru/Right		#445	#494	#544	282	309	330
	Southbound Approach							
	Southbound Left Southbound Thru/Right	260	89 106	90 176	90 198	123 271	123 300	123 312
3	Wesley Avenue (E/W) & Fairview Avenue (N/S)		100	110	100	211	000	012
	Overall Intersection (TWSC)							
	Eastbound Left/Thru/Right		3	3	5	0	0	5
	Westbound Approach		_	<u>^</u>	<u>^</u>		•	<u>^</u>
	Westbound Left/Thru/Right Northbound Approach		3	0	3	0	0	0
	Northbound Left/Thru/Right		0	0	3	0	0	3
	Southbound Approach		0	0	0	0	0	0
4	Richmond Avenue (E/W) & Fairview Avenue (N/S)		0	0	0	0	0	0
	Overall Intersection (TWSC)							
	Eastbound Left/Thru/Right		3	3	3	3	3	3
	Westbound Approach							100
	Westbound Left/Thru/Right		38	50	60	115	143	160
	Northbound Left/Thru/Right		0	0	0	0	0	0
	Southbound Approach		0	3	3	3	3	3
5	Wellington Road (E/W) & Fairview Avenue (N/S)		0	5	5	5	5	5
	Overall Intersection (Signalized)							
	Eastbound Left	225	73	85	92	93	101	105
	Eastbound Thru		42	46	47	84	91	93
	Westbound Approach Westbound Thru		97	105	106	124	132	134
	Westbound Right	235	27	40	50	21	26	29
	Southbound Approach		46	48	53	04	102	105
	Southbound Len		0	40	0	0	0	0
6	Site Entrance 1 (E/W) & Fairview Avenue (N/S)							
	Eastbound Approach							
	Eastbound Left/Right				5			3
	Northbound Approach				0			0
7	Site Entrance 2 (E/W) & Fairview Avenue (N/S)	-			U			U
	Overall Intersection (TWSC)							
	Eastbound Left/Right				8			8
8	Wesley Avenue (E/W) & Site Entrance 3 (N/S)		1					
	Uverall Intersection (TWSC) Eastbound Approach							
	Eastbound Left/Thru				0			0
	Southbound Approach Southbound Left/Right				3			3
<u> </u>	oounbound coloringin	I	I		5			J

[1]: 2027 Future Background is 2027 Future Conditions without Development scenario, which includes inherent growth plus one background development, in addition to existing conditions. [2] 2027 Total Future is 2027 Future Conditions with Development scenario, which includes Future Background conditions plus Edugenius Development.

Based on the capacity and queueing analysis results, the LOS with development is anticipated to remain similar as compared to future conditions without development; furthermore, with the addition of the proposed Edugenius Development traffic, the increase in 95th percentile queue lengths is anticipated to be two standard car lengths (25 feet each) or less during either peak hours at any of the applicable lane groups of the study intersections as compared to future conditions without development

Therefore, the proposed Edugenius Development will not have a substantial impact to the surrounding transportation and roadway network.

Peer Child Care Center Site Parking Comparison

To consider the adequacy of the proposed number of parking spaces to be provided for the child care center portion of the proposed Edugenius Development, a comparison analysis of the number of parking spaces for peer child care center sites in the City of Manassas was conducted for this study.

Relevant data discussed in the subsections below is included in Appendix H. Appropriate waivers to the parking requirement, if required, will be submitted as necessary.

Square Footage Comparison

The 46 parking spaces² for the child care center of the proposed Edugenius Development are depicted on the conceptual plan illustrated in Figure 23 and in Figure 24. Per the City's Zoning Ordinance minimum off-street parking requirements (and per the Downtown Special District requirements as well) for child care center uses, a minimum of 1 off-street parking space is required per 250 square feet of a building's gross square feet (SF). As described below, utilizing such a rate would yield an impractical and excessive amount of parking spaces.

The total building square footage of the child care center portion of the proposed Edugenius Development is approximately 17,898 square feet. Of note, only approximately 9,500 square feet of the total is proposed to be utilized for conventional classroom space. As noted in the City's Planning Department Determination Letter, dated February 28, 2022, the remaining square footage would be dedicated to "non-traditional spaces such as larger corridors, movement rooms and a "Kinderkitchen" which overlap with instructional space typically found in a conventional educational facility."

The selected peer child care center sites analyzed are assumed to consist of predominantly conventional classroom space. Non-traditional spaces, if any, present in the selected peer child care center sites analyzed were not measured nor considered. It is assumed that the peer child care center sites consist of conventional classroom space in their entirety.

Sources of information for the peer locations include the City of Manassas Commissioner of the Revenue's Real Estate Assessment Information Site (for the year built, square footage, and number of parking spaces for each location) and the Virginia Department of Social Services (which lists capacity and hours of operation for each location). Table 16 presents the results of the peer site parking rate comparison, with the building square footage, the year built and location of each peer site. Of note as well, for this portion of the proposed Edugenius Development, the Applicant's proposed child care center operating hours and capacity are consistent with other child care centers in the City of Manassas.

More specifically, Table 16 includes a column that lists the total building square footage of the peer child care center sites. The table also includes columns that list the peer child care center sites' capacity (students, faculty, and staff) and the number of parking spaces provided.

² It should be noted that the 46 parking spaces described in this peer comparison analysis are preliminary in nature and subject to modifications based on final engineering associated with the Special Use Permit Application and other subsequent entitlements.

Table 16: Peer City of Manassas Child Care Center Site Parking Comparison

				Hours of (Operation						Parking Spaces	Parking Spaces
#	Year Built*	Name	Address / Location	Open**	Close**	Capacity***	Square Feet (SF)*	Parking Spaces*	Accessible Spaces*	Total Spaces	per SF (SF / Total Spaces)	per Person (Total Spaces / Capacity)
	Proposed	CHILD CARE CENTER	9522 Fairview Avenue	6:30 (Proposed)	18:00 (Proposed)	153 (Proposed)	9,500 (Proposed)	44	2	46	207	0.30
1	2019	The Goddard School	10405 Dumfries Road	7:00	18:00	158	10,371	47	3	50	207	0.32
2	1988	La Petite Academy of Manassas	10023 Dumfries Road	6:30	18:30	160	6,874	25	3	28	246	0.18
3	1988	Minnieland Academy	9511 Technology Drive	6:30	18:00	130	6,300	19	1	20	315	0.15
4	2005	The Compass School	9811 Godwin Drive	6:30	18:00	248	14,589	49	2	51	286	0.21
5	2003	Merit Wellington	10249 Hendley Road	6:00	18:00	300	9,866	53	4	57	173	0.19
6	1982	Merit Manassas	8485 Plantation Lane	6:30	18:00	341	14,304	47	0	47	304	0.14
7	1900/1950/1995	Merit Old Town	8757 Signal Hill Road	6:30	18:00	142	8,846	29	0	29	305	0.20
8	1994	Childtime of Manassas	8461 Signal Hill Road	6:00	18:00	214	8,880	44	2	46	193	0.21
9	1985	Learning Tree	9840 Grant Avenue	6:00	18:30	100	4,408	15	0	15	294	0.15
10	1955/1995	Robert Day Child Care Center	9208 Centreville Road	6:00	18:30	25	2,500	8	0	8	313	0.32
11	1999	Carousel Children's Academy	9151 Centreville Road	6:00	18:45	150	6,000	18	2	20	300	0.13

Notes.

*Source: City of Manassas Commissioner of the Revenue's Real Estate Assessment Information Site, https://gis.vgsi.com/manassasva/

**Per Virginia Department of Social Services (VDSS), Child Care Center website, or Google, as applicable.

***Assumed to include combined total number of children and faculty/staff. Source: Virginia Department of Social Services (VDSS), https://www.dss.virginia.gov/facility/search/cc2.cgi

As presented in the table above, the peer child care center sites have substantially less overall building square footage that the overall building square footage of the proposed Edugenius Development (which is approximately 17,898 square feet as mentioned above) and yet, they have closely comparable capacity numbers (i.e., children, faculty, and staff) with the proposed Edugenius Development. Further, their number of parking spaces are closely comparable with the proposed Edugenius Development.

Notwithstanding the substantially greater building square footage of the proposed Edugenius Development, it is reasonable for the proposed Edugenius Development to provide only the number of parking spaces based on the proposed approximately 9,500 square feet of conventional classroom space, rather than the total building square footage of approximately 17,898 square feet. As noted, the proposed Edugenius Development child care parking space yield (based on 9,500 square feet) is closely comparable to the other peer child care center sites. These sites generally are operating within smaller buildings but have similar capacity. Therefore, it is reasonable to conclude that, by using the square footage of conventional classroom space, the 46 parking spaces provided for the child care center portion of the proposed Edugenius Development would yield an acceptable number of parking spaces. In fact, providing additional parking spaces from what is currently proposed (based on total building square footage) would result in the provision of excessive parking spaces. These additional parking spaces could alter the effective layout of the Site and could cause impacts beyond those ones related strictly to transportation concerns.

Such negative consequences, with the provision of excess parking spaces (and asphalt), include an increase of the heat island effect (affecting quality of life for residents and visitors to the City), unnecessary destruction of trees and areas for new plantings (also affecting quality of life), excessive stormwater management to accommodate the increased impervious area (affecting costs and the environment), less green areas for playground spaces for children, a less aesthetically pleasing campus within the Manassas Historic Overlay District, and smaller building footprints for the elementary school and child care center, which would ultimately eliminate the ability to provide the extra space that is fundamental to this particular program of education.

Building Capacity Comparison

In addition, an alternative analysis approach also supports the practicability and adequacy of the proposed number of parking spaces provided for the child care center portion of the proposed Edugenius Development. Parking demand based on person capacity (i.e., children, faculty and staff) closely compares with peer child care center sites. Existing peer child care centers, primarily within the City of Manassas, also were assessed based on their total capacity (which is assumed to include the combined total number of enrolled children and faculty/staff, for consistency) and their parking provided (along with hours of operation) in order to provide a consistent comparison of parking rates (number of spaces per person) for the child care center portion of the proposed Edugenius Development (which, of note, would include the 125-student capacity plus additional faculty and staff for a total of approximately 153-person capacity for this alternative comparison).

Based on the results of this alternative comparison analysis, also depicted in Table 16, the parking rate for the child care center portion of the proposed Edugenius Development is approximately 0.30 of a space per person. The rate is nearly identical to the estimated approximate parking rate for The Goddard School on Dumfries Road (0.32 of a parking space per person), which was recently approved and constructed in 2019. The child care center portion of the proposed Edugenius Development and the Goddard School have similar capacity, number of parking spaces, and hours of operation. Of important note also, the estimated parking rates for the existing child care center facilities in the City vary but average below 0.20 spaces per person.

Based on the results of this alternative comparison analysis based on capacity, the parking rate (number of spaces per person) for the child care center of the proposed Edugenius Development is consistent with the parking rates for other peer child care center sites throughout the City.

Conclusion

This report presents findings of a Traffic Impact Study (TIS) conducted for the proposed Edugenius Development (the Site / the Development / the Property), situated in the City of Manassas (the City), Virginia.

The Edugenius Development is proposed to be situated on an approximately 3.3-acre single parcel of land, which can be identified on Manassas City GIS with the following Tax Map ID #: 100-01-00-202. The site is currently zoned as R-1 (Low Density, Single-Family Residential), with a Comprehensive Plan Land Use of Downtown.

The Applicant is proposing to redevelop the parcel with an approximately 120-student capacity elementary school and 125student capacity day care / childcare facility. The Edugenius Development in full is anticipated to be constructed and in operation by 2027.

The analysis presented in this report supports the following assumptions and findings:

Analysis Components

- As determined based on discussions with City staff, a growth rate of one-percent (1.0%) per year was applied to all
 movements at the intersection of Fairview Avenue (Route 4353) at Wellington Road (Route 4352), and propagated as
 mainline through movements at subsequent study intersections along the road network where applicable, to account for a
 regional increase in background traffic.
- The total Site is anticipated to generate approximately 152 new trips during the AM peak hour, 80 new trips during the PM peak hour, and 818 new daily trips on a typical weekday, after reductions.

Non-SOV Elements

- Various biking infrastructure exists along Tudor Lane, Signal Hill Road, and Richmond Avenue within the vicinity of the site; a shared use path exists along Wellington Road.
- No bus routes currently provide service within the vicinity of the site.
- The site is located within a 0.55-mile radius of the Manassas Train Station, which serves the Virginia Railway Express (VRE) and Amtrak, and within a 3.0-mile radius of the Manassas Regional Airport (HEF).

Infrastructure

• Currently, it is anticipated that the Edugenius Development will be accessed via one full-movement entrance along Wesley Avenue and one full-movement entrance along with one-partial movement entrance along Fairview Avenue.

Analysis Results

- One (1) existing study intersection (Fairview Avenue at Richmond Avenue) has one approach that operates at unacceptable levels of service (LOS) during one of the existing peak hours (PM peak hour only). The same approach is anticipated to continue to operate similarly at unacceptable LOS under all scenarios during the same peak hour.
- With the Edugenius Development in place (built and in operation), the LOS and delays are anticipated to remain similar
 as compared to future conditions without development; furthermore, with the addition of the Edugenius Development
 traffic, the increase in 95th percentile queue lengths is anticipated to be two standard car lengths (25 feet each) or less
 during either peak hour at any of the applicable lane groups of the study intersections, as compared to future conditions
 without development.
- The northbound through/right lane group of the signalized study intersection of Fairview Avenue at Manassas Police Department Entrance / Signal Hill Road has existing 95th percentile queue lengths that would spill past the proposed site entrances on Fairview Avenue; this is an existing condition and as noted above, the Edugenius Development traffic is

anticipated to add only two standard car lengths or less during either peak hour as compared to future conditions without development. In addition, users of the proposed Development would have an alternative access point along Wesley Avenue with the ability to accommodate additional volumes.

- The remaining 95th percentile queue lengths are anticipated to be accommodated within the various storage bays under all scenarios.
- The proposed Edugenius Development has minimal impact on delays and queues at the existing intersections.
- No mitigations are required as per the analysis results.
- Based on a comparison with the number of parking spaces for peer child care center sites in the City of Manassas, the
 parking rate for the child care center portion of the proposed Edugenius Development, whether from a square footage (for
 conventional classroom space) or capacity perspective, is consistent with the parking rates for other peer child care center
 sites throughout the City, and would provide a practical and adequate amount of parking spaces.

Overall Conclusion

Based on the capacity and queueing analysis results, the proposed Edugenius Development will not have a substantial impact to the surrounding transportation and roadway network.

Transportation Technical Appendix

Edugenius Development

City of Manassas, Virginia

July 20, 2022


CONTENTS

- A. Scoping Document
- B. Raw Traffic Data and City-Provided Timing and Layout Sheets
- C. VDOT Crash Data
- D. HCM Level of Service Definitions
- E. Intersection Capacity Analysis Worksheets 2022 Existing Conditions
- F. Intersection Capacity Analysis Worksheets 2027 Future without Development
- G. Intersection Capacity Analysis Worksheets 2027 Future with Development
- H. Peer Child Care Center Site Data

A. Scoping Document

PRE-SCOPE OF WORK MEETING FORM

Information on the Project Traffic Impact Analysis Base Assumptions

The applicant is responsible for entering the relevant information and submitting the form to VDOT and the locality no less than three (3) business days prior to the meeting. If a form is not received by this deadline, the scope of work meeting may be postponed.

Contact Information						
Consultant Name: Tele:	Kevin Sitzman, Gor 571-248-0992	ove Slade Associa	tes, Inc			
E-mail:	kds@goroveslade.	com				
	JUSIE Eligei					
E-mail:	josefinengel@gmai	il.com				
Project Information						
Project Name:	9522 Fairview Ave	nue	Locali	ity/County:	City of	f Manassas
Project Location: (Attach regional and site specific location map)	The proposed develo and south of Signal H	opment will be locat Iill Road in the City	ed west of Mana	of Fairview Avenu ssas, Virginia.	ue, north	n of Wesley Avenue,
Submission Type	Comp Plan 🗌	REZ/SUP 🖂		Site Plan 🗌		Subd Plat 🗌
Project Description: (Including details on the land use, acreage, phasing, access location, etc. Attach additional sheet if necessary)	The development wi Mapper with the follo is currently zoned as The site is anticipate capacity and a 125-s operation by 2025. Access to the site wil access entrances on	ll be situated on one owing Tax Map #'s: R-1 (Residential). d to consist of an ele tudent capacity day ll be provided by on Fairview Avenue.	e parcel 100/01 ementar care. Th e full acc	of land that can be 00/ 202. The app y school with an a e development is cess entrance on V	e identifi roximat approxin anticipa Vesley A	ied on City's Tax ely 3.3-acre property nately 120-student ated build out and in Avenue and two full
	Residential	Commercial		Mixed Use 🗌		Other 🖂
Proposed Use(s): (Check all that apply; attach additional pages as necessary)	Other Use(s): Inst ITE LU Code(s): 53 Square Ft or Other ITE LU Code(s): 56 Square Ft or Other	itutional Uses 0 – Private School Variable: 120 Stu 5 – Day Care Cent Variable: 125 Stu	(K-8) dents er dents	Commercial U ITE LU Code(s) Square Ft or Ot	Jse(s)): n/a ther Va	riable: n/a
Total Peak Hour Trip Projection:	Less than 100	100 - 499 🖂		500 - 999		1,000 or more 🗌

It is important for the applicant to provide sufficient information to county and VDOT staff so that questions regarding geographic scope, alternate methodology, or other issues can be answered at the scoping meeting.

Traffic Impact Analys	is A	ssumption	S				
Study Period	Exi	sting Year: 20	022	Build-out Year	: 2025	5	Design Year: N/A
Study Area Boundaries	No	rth: Tudor Laı	ne		Sout	th: Welling	ton Road
(Attach map)	We	st: Fairview A	lvenue		East	:: Ellicott La	ne
External Factors That Could Affect Project (Planned road improvements, other nearby developments)	No	ne.					
Consistency With Comprehensive Plan (Land use, transportation plan)	Yes						
Available Traffic Data (Historical, forecasts)	VD 202	OT Historical 22 Turning Mo	AADT Dat ovement C	a (2015-2019) Jounts (TMCs)			
	Roa (to	ad Name: Fair /from the Nor	rview Ave rth) – 30%	nue	Roa (to/	d Name: We from the So	ellington Road utheast) – 30%
Trip Distribution (Please refer to attached Figure 2 in Supplement)	Roa (to	ad Name: Rich /from the Eas	nmond Ave st) – 5%	enue	Roa (to/	d Name: W from the So	ellington Road uthwest) – 15%
rigure 2 in Supprement)	Roa (to	ad Name: Tud /from the Nor	or Lane rthwest) –	5%	Roa (to/	d Name: Sig from the Ea	nal Hill Road st) – 15%
Annual Vehicle Trip	1.0	%/year	Peak Pe (check all	riod for Study that apply)		AM 🛛 F	PM 🗌 SAT
(See Note 5.)	(20	22-2025)	Peak Ho (with redu	our of the Adj.	AM:	152 / PM: 8	30 / Daily: 818
	1.	Fairview Av	renue and	Tudor Lane	7.	Fairview A (Future Ir	Avenue and Site Entrance 2 atersection)
Study Intersections	2.	Fairview Av	renue and	Signal Hill Road	8.	Wesley Av (Future Ir	venue and Site Entrance 3 Itersection)
and/or Road Segments (Attach additional sheets as	3.	Fairview Av	enue and	Wesley Avenue	9.		
necessary) (Please refer to attached	4.	Fairview Av Avenue	renue and	Richmond	10.		
Figure 1.)	5.	Fairview Av Road	renue and	Wellington	11.		
	6.	Fairview Av (Future Inte	venue and ersection)	Site Entrance 1	12.		
Trin Adiustreant	Inte	ernal allowance Yes 🖾 No	Reduction	:	Pass	-by allowand es 🗌 No	e Reduction:
Factors					ITE L	UC 565: 44%	(PM), assumed 34% (AM/DAY)
			_		Base	d on Trip Gene	eration Manual 11 th Ed.
Software Methodology	\square	Synchro 🗌] HCS (v.20	000/+) 🗌 SID	RA		1 Other

It is important for the applicant to provide sufficient information to county and VDOT staff so that questions regarding geographic scope, alternate methodology, or other issues can be answered at the scoping meeting.

Traffic Signal Proposed or Affected (Analysis software to be used, progression speed, cycle length)	 Existing traffic signals that could be affected: 1. Fairview Avenue and Signal Hill Road 2. Fairview Avenue and Wellington Analysis Software: Synchro, version 10 Results: HCM 2010 Methodology (See Note 9) Queue Lengths to be Reported: 95th Percentile from Synchro
Improvement(s) Assumed or to be Considered	None.
Background Traffic Studies Considered	None.
Plan Submission	Master Development Plan (MDP)Generalized Development Plan (GDP)Preliminary/Sketch PlanOther Plan type (Final Site, Subd. Plan)
Additional Issues to be Addressed	Queuing analysis Actuation/Coordination Weaving analysis Merge analysis Bike/Ped Accommodations Intersection(s) TDM Measures Other ()

NOTES on ASSUMPTIONS:

- 1. Signal timings will be obtained from the City.
- 2. The scenarios to be included in the study are Existing Conditions (2022), Future without Development (2025) and Future with Development (2025). The study will analyze AM and PM peak hours.
- 3. Turning movement counts (TMCs) that were collected in 2022 will be used as the base for existing conditions.
 - a. Note, these turning movement counts were cross verified with 2016 counts at the same intersections and were determined to be consistent with pre-Covid conditions.
- 4. Existing peak hour factors will be based on the traffic counts and utilized on a by-intersection basis. Peak hours factors by intersection in the range of 0.85 to 1.00 will be used for existing scenario. Peak hour factors of 0.92 will be used for all future scenarios if the existing peak hour factor by intersection is less than 0.92.
- 5. Heavy vehicle percentages (HV%) will be based on count data. For any new intersection, the HV% will be based on a default *Synchro* value of 2.0% per movement.
- 6. To account for 2025 future conditions, an inherent growth of 1.0% per year (between 2022 and 2025) will be applied to all movement at the intersection of Fairview Avenue and Wellington Road. In order to balance the network, the growth volumes will be propagated to other study intersections as mainline through volumes, as applicable.
- 7. For any approach, a level of service (LOS) D or better, or non-degradation, will be considered as acceptable/desirable for traffic operations. For all approaches, the projected future conditions without development LOS and delay should be maintained or comparable to future with development conditions, if possible. Will show intersection, approach, and movement LOS.
- 8. Will provide 95th percentile queues from Synchro.
- 9. HCM 2010 methodology will be utilized where applicable; HCM 2000 methodology will be utilized if HCM 2010 methodology is not applicable.
- 10. Crash data obtained from VDOT's Crash Analysis Tool will be analyzed at existing study intersections (2014-2019, as more recent data may be influenced by the COVID-19 pandemic).

SIGNED: _

Applicant or Consultant

PRINT NAME: <u>Kevin Sitzman, PE</u> Applicant or Consultant

SIGNED: _

DATE: _____

DATE: 6/14/2022

City of Manassas Representative

PRINT NAME:

City of Manassas Representative





Figure 1: Site Location and Study Intersections

www.goroveslade.com



Figure 2: Direction of Approach



Figure 3: Concept Plan

Table 2: Trip Generation – Peak Hour of the Adjacent Street (ITE 11th Edition)

						We	ekda	ay ·		
Land Use	ITI	E Code	Size	A	VI Peak	Hour	PM	Peak	Hour	Daily
				In	Out	Total	In	Out	Total	Total
Proposed Development										
Private School (K-8)		530	120 Students	52	40	92	14	17	31	493
Day Care Center		565	125 Students	48	43	91	42	47	89	492
	Pass-By Reduction*		34% / 44%	-16	-15	-31	-19	-21	-40	-167
	Subtotal	without R	eductions / Driveway Trips	100	83	183	56	64	120	985
		Ne	et new site trips (Proposed)	84	68	152	37	43	80	818

*Pass-By reduction for ITE LUC 565 for the PM peak hour was based on ITE's TGM 11th ED (44%). No data was available for the AM and Daily periods, as such 34% was assumed (PM minus 10%).

Subject: FW: [EXTERNAL]RE: [EXTERNAL]RE: [EXTERNAL]Barracuda Networks' Email Gateway Defense Quarantine Notification

From: Nyika Roberson-Ramos <<u>nroberson-ramos@ci.manassas.va.us</u>> Sent: Thursday, July 7, 2022 4:03 PM To: Niraja Chandrapu <<u>niraja.chandrapu@goroveslade.com</u>> Subject: RE: [EXTERNAL]RE: [EXTERNAL]RE: [EXTERNAL]Barracuda Networks' Email Gateway Defense Quarantine Notification

Hello Niraja,

Yes, will be converted to city or school board offices by end of 2023.

Please let me know if I may be of further assistance.

Thank you, Nyika Roberson-Ramos, PE Senior Engineer (o) 703-257-8397 |(c) 571-208-4832 nroberson-ramos@ci.manassas.va.us



From: Niraja Chandrapu <<u>niraja.chandrapu@goroveslade.com</u>>
Sent: Thursday, July 7, 2022 2:21 PM
To: Nyika Roberson-Ramos <<u>nroberson-ramos@ci.manassas.va.us</u>>
Subject: [EXTERNAL]RE: [EXTERNAL]RE: [EXTERNAL]Barracuda Networks' Email Gateway Defense Quarantine Notification

[EXTERNAL]

Nyika,

If the conversion is not going to happen by 2025 then we will not include it as background development. For the current use, the counts include the trips associated with it.

Please let me know if you would be available for a quick call. I tried to reach you on your cell, but could not leave a message as your inbox is full.

Thanks, Niraja.

Niraja Chandrapu, PE, PTOE Director of Engineering Project Manager

GOROVE SLADE Transportation Planners and Engineers T 703.787.9595 / C 848.250.7327 *We Moved!* 4114 Legato Road / Suite 650 / Fairfax, VA 22033 nc@goroveslade.com / www.goroveslade.com

Please consider the environment before printing this email. Thank you.

From: Nyika Roberson-Ramos <<u>nroberson-ramos@ci.manassas.va.us</u>>
 Sent: Thursday, July 7, 2022 2:14 PM
 To: Niraja Chandrapu <<u>niraja.chandrapu@goroveslade.com</u>>
 Subject: RE: [EXTERNAL]RE: [EXTERNAL]Barracuda Networks' Email Gateway Defense Quarantine Notification

Good Afternoon Niraja,

This is a new building. Therefore, no conversion yet.

However, long-term/worst case, higher numbers are preferred.

Based on the number of offices, please use the higher numbers, but revise label to actual current use.

Please call me on my cell phone to discuss further if needed.

Thank you, Nyika Roberson-Ramos, PE Senior Engineer (o) 703-257-8397 |(c) 571-208-4832 nroberson-ramos@ci.manassas.va.us



From: Niraja Chandrapu <<u>niraja.chandrapu@goroveslade.com</u>>
Sent: Thursday, July 7, 2022 1:16 PM
To: Nyika Roberson-Ramos <<u>nroberson-ramos@ci.manassas.va.us</u>>
Subject: [EXTERNAL]RE: [EXTERNAL]Barracuda Networks' Email Gateway Defense Quarantine Notification

[EXTERNAL]

Hi Nyika,

Per the Comprehensive Plan/Capital Improvements Program (see below), the police station may be converted to City Office use. See also below the trip generation for converting the existing Police Station to a government office. Please let us know if you would like us to consider the conversion as a background development for the Fairview Avenue project (Tillet Hall Special Use Permit)?

Chapter 7 Comp. Plan excerpt:

City Facility

- 1 Marvin L. Gillum City Hall
- 2 Public Works / Utility Facility
- 3 City Offices (Planned)
- 4 Public Safety Facility (Planned)
- 5 Manassas Fire Department
- 6 Manassas Rescue Squad
- 7 Fire and Rescue Station 21
- 8 Fire and Rescue Offices
- 9 Social Services Offices
- 10 Manassas Museum
- 11 Airport Terminal

City Schools

- 1 Baldwin Elementary/Intermediate School
- 2 Dean Elementary School
- 3 Haydon Elementary School
- 4 Mayfield Intermediate School
- 5 Round Elementary School
- 6 Weems Elementary School
- 7 Metz Middle School

8 Osbourn High School

Utility

- 1 Substation
- 2 Generator
- 3 Water Tower/Tank
- 4 Telecommunication Facility
- 5 UOSA Pump Station

CIP Excerpt:

Public Safety Facility (P-015)

Continued construction of the Public Safety Facility at 9608 Grant Avenue is underway. The facility will include Police Headquarters, consolidated public safety logistics, 911 Center, Emergency Operations Center, Fire & Rescue Administration, and the IT Department.



Public Safety Facility

Comparison for Government Office:



				M	reekda
TE Code	Size	1	AM Peak	Hour	PM
		In	Out	Total	In
730	36.376 kSF of GFA	91	30	121	15
575	36.376 kSF of GFA	12	5	17	5
	TE Code 730 575	TE Code Size 730 36.376 kSF of GFA 575 36.376 kSF of GFA	TE Code Size 730 36.376 kSF of GFA 91 575 36.376 kSF of GFA 12	TE Code Size AM Peak 730 36.376 kSF of GFA 91 30 575 36.376 kSF of GFA 12 5	TE Code Size AM Peak Hour In Out Total 730 36.376 kSF of GFA 91 30 121 575 36.376 kSF of GFA 12 5 17

Notes

[1]: As no LUC exists for Police Station in ITE 11, the similar ITE LUC 575 was used.

[2]: ITE LUC 575 (Fire and Rescue Station) does not have data for the AM peak hour nor week day daily period in ITE 11. The AM peak hour was assumed to be sim i week day daily trips were assumed to be 10 times the PM peak hour.

79

25

104

10

Difference (Proposed Minus Existing)

Thanks, Niraja.

Niraja Chandrapu, PE, PTOE

Director of Engineering Project Manager

GOROVE SLADE

Transportation Planners and Engineers T 703.787.9595 / C 848.250.7327 We Moved! 4114 Legato Road / Suite 650 / Fairfax, VA 22033 nc@goroveslade.com / www.goroveslade.com

Please consider the environment before printing this email. Thank you.

From: Nyika Roberson-Ramos <<u>nroberson-ramos@ci.manassas.va.us</u>>
 Sent: Wednesday, June 15, 2022 11:08 AM
 To: Niraja Chandrapu <<u>niraja.chandrapu@goroveslade.com</u>>
 Subject: RE: [EXTERNAL]Barracuda Networks' Email Gateway Defense Quarantine Notification

Good Morning Niraja,

Great speaking with you yesterday.

I sent the signed document to Com. Dev. and they will follow-up with Developer. Please include pedestrian information and school signage and striping, ect.

Best Regards, Nyika Roberson-Ramos, PE Senior Engineer (o) 703-257-8397 |(c) 571-208-4832 nroberson-ramos@ci.manassas.va.us



B. Raw Traffic Data and City-Provided Timing and Layout Sheets

LOCATION: F CITY/STATE:	airviev Manas	w Ave - ssas, V	Tudor A	' Ln											QC DATE:	: JOB 	‡: 1583 ∕lay 24	37601 2022
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+ 9 N/A +	* N/2 • • •		∧/A ≁		-	-\$ @@				↑ [-		N/A			N/A	
15-Min Count Period Reginping At		Fairvie (North	w Ave bound)			Fairvie (South	w Ave bound)			Tude (Eastb	or Ln oound)			Tudo (Westi	or Ln bound)		Total	Hourly Totals
6:00 AM 6:15 AM 6:30 AM 6:45 AM 7:00 AM 7:15 AM 7:30 AM 7:30 AM 8:00 AM 8:15 AM 8:30 AM 8:30 AM 8:45 AM	Left 20 29 15 25 45 38 53 80 61 64 69 51	Inru 50 30 47 77 94 90 110 112 131 104 97 114	Kight 0	U 0 0 0 0 0 0 0 0 0 0 0	Left 0 0 0 1 0 0 0 0 0 0 0 0 0 1 1 0 0 0 1 1 0 1 1 0	Inru 22 18 20 45 51 55 54 56 66 46 57 South	Right 6 5 4 2 5 7 10 17 16 3 8 bound	U 0 0 0 0 0 0 0 0 0 0 0 0 0	Left 5 1 1 5 6 1 2 4 8 7 5 4	Inru 0	Right 2 11 11 38 24 21 31 29 79 44 21 onund	U 0 0 0 0 0 0 0 0 0 0 0 0 0 0	Left 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	Ihru 0 0 1 1 0 0 0 0 0 0 0 0 0 0 0 0 0 0	Kight 1 1 1 0 0 2 1 0 2 1 2 1 0 2 1 0 2 1 0 2 1 0 2 1 0 2 1 0 0 2 1 0 0 2 1 0 0 2 1 0 1 0 1 0 1 0 1 0 1 0 1 0	U 0 0 0 0 0 0 0 0 0 0 0	106 95 100 195 226 213 260 284 314 319 266 258	496 616 734 894 983 1071 1177 1183 1157
Flowrates	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Tot	tal
Heavy Trucks Buses Pedestrians Bicycles Scooters	236 20 0	10 12 16 0	0	0	4 0 0	0 0 0	8 0	0	0	0 0 0	8 0	0	0	0 0 0	8 4 0	0	12 5: 1(0	2 6

Comments: Report generated on 6/2/2022 9:32 AM

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15-Min Count Period		Fairvie (North	w Ave bound)			Fairvie (South	w Ave bound)			Tud (Eastk	or Ln bound)			Tude (Westl	or Ln bound)		Total	Hourly
Beginning At	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U		TOLAIS
4:00 PM 4:15 PM	38 40	81 110	0 1	0	1	112 118	13 9	0 0	4	0	38 36	0	1	0	0 2	0	288 322	
4:30 PM	49	93	3	0	0	117	20	0	6	0	34	0	0	0	2	0	324	1255
5:00 PM	43 45	85	2	0	1	136	9	0	2	0	40	0	0	0	1	0	321	1255
5:15 PM	33 38	83 78	2	0	1	124 116	8 15	0	4	0	52 42	0	0	1	1	0	309 295	1277 1248
5:45 PM	46	75	0	0	1	97	11	Ő	6	2	37	0	0	0	0	0	275	1202
6:00 PM 6:15 PM	32 37	84 82	0 0	0 0	2 2	105 92	14 8	0 0	2	0 1	22 39	0 0	0	1 0	2 1	0 0	264 265	1143 1099
6:30 PM	17	68	Ő	Ő	1	72	13	Õ	7	2	24	Ő	Ő	Ő	ō	Õ	204	1008
6:45 PM	39	105 North	1 hound	0	0	8/ South	8 bound	0	3	U Eacth	31	0	1	1 Worth	0 bound	0	276	1009
Flowrates	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	То	tal
All Vehicles	196	372	12	0	0	468	80	0	24	0	136	0	0	0	8	0	12	.96
Buses Buses Pedestrians Bicycles Scooters	0	8 4 0	0		0	8 0 0	0		0	0 0 0	4 0		0	0 0	4		2	1 1
Comments:																		

Location: F City/state:	airviev Mana	w Ave - ssas, V/	Signa A	l Hill R	d										QC DATE:	: JOB 	‡: 1583 ∕Iay 24	37603 2022
8 🔶 5 🚽 1 🖬 6 🔶 0 7	384 6 205 • • • 092 • • 0 436 • 234	741 5 173 • • • 5 43 479	01 🔶 331 1 29 🔶 217			Pe Pea	ak-Hou k 15-M Data TH	r: 7:30 in: 8:0		- 8:30 / 8:15 unts	AM AM			0 + 0 0 • 0	34 0 24 - + + - + 0 35 26	32 4.6 9.9 4.4	23 • 2 • 0 • 34 • 9	24
4]		→ [] → [0		-	*					*	-		0 0 0			0 0 7 0	
• N/A • P	+ N/4 + + - + - + N/4		► N/A ►		-	-÷	•			┺ ┺	<u>1</u>	-		N/A	→ → + → → + N/		■ N/A	
15-Min Count Period Beginning At		Fairvie (Northl	w Ave bound)			Fairvie (South	w Ave bound)			Signal (Eastb	Hill Rd ound)			Signal (West	Hill Rd bound)		Total	Hourly Totals
6:00 AM 6:15 AM 6:30 AM 6:45 AM 7:00 AM 7:15 AM 7:30 AM 7:45 AM 8:00 AM 8:15 AM 8:30 AM 8:30 AM	Left 0 1 0 0 1 0 0 1 0 0 0 0 0 0 0 0 0 0 0	Thru 39 32 46 63 70 104 100 137 95 116 117	Right 2 7 12 20 10 9 8 16 10 5 3	U 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	Left 6 11 15 58 42 30 42 40 38 53 42 44	Thru 17 17 18 22 32 47 43 36 59 67 43 36	Right 1 0 2 0 2 0 2 0 2 0 2 0 2 0 2 0 2 0 3	U 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	Left 2 1 0 0 0 1 1 2 0 0 0 0 0 0 0 0 0 0 0 0	Thru 0 1 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	Right 0	U 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	Left 2 4 4 10 25 15 8 8 5 8 10 8 10 8	Thru 0 1 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	Right 25 26 15 41 71 49 66 101 57 77 48	U 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	94 101 112 216 250 224 275 294 316 315 266 259	523 679 802 965 1043 1109 1200 1191 1156
Peak 15-Min		North	pound			South	bound			Eastb	ound			West	pound		Tot	al
All Vehicles	Left 0	Thru 548	Right 64	U 0	Left 152	Thru 236	Right 8	U 0	Left 8	Thru 0	Right 0	U 0	Left 20	Thru 0	Right 228	U 0	12	64
Heavy Trucks Buses Pedestrians Bicycles Scooters	0	12 0 0	0		20 0	8 0 0	0 0		0	0 0 0	0		0	0 0 0	4 0		44 0 0	4

Location: F City/state:	airviev Manas	w Ave - ssas, V	Signa A	l Hill R	d										QC DATE:	C JOB # Tue, N	#: 1583 May 24	37604 2022
4 + 2 = 4 $4 + 6 + 0 = 7$	683 3 406 4 406 4 406 1 317 447	519 • 274 • 274 • 2 • 2 • • • • • • • • • • • • • • • • • • •	00 ← 241 0 41 ← 319			Pe Pea	ak-Hou k 15-M	ir: 4:30 lin: 5:0		- 5:30 5:15	PM PM			0 + 0 0 0 + 0	13 0 11 	12 5 11 5 • • •	1 + 0 7 0 +	0.8
6		→ [→ 1 → [1		-	1	↓ ↓				₽	-		0 0 0			0 0 7 0	
+ 3 N/A +	* N// • • • •		∧/A ✦		-		•			↑ [•]	<u>*</u>	_		N/A			► N/A	
15-Min Count Period	-	Fairvie (North	w Ave bound)			Fairvie (South	ew Ave bound)			Signal (Eastb	Hill Rd bound)			Signal (West	Hill Rd bound)		Total	Hourly Totals
4:00 PM	Left 0	1 hru 78	Right 10	0	Left 58	Thru 89	Right 0	0	Left 0	0	Kight 0	0	Left 10	i hru 0	Kight 56	0	301	
4:15 PM	0	87	6	0	62	87	0 0	0	1	0	0 0	0	10	0	50	0	303	
4:45 PM	Ō	76	7	0	64	107	1	0	0	0	0	0	6	0	49	0	310	1233
5:15 PM	0	72	13	0	73	102	2	0	1	2	0	0	1/	0	49	0	334	1266
5:30 PM	2	76	9	0	81	76	3	0	0	0	0	0	9	0	40	0	296	1266
5:45 PM 6:00 PM	0 1	70 73	8	0	64 53	64 74	0	0	0	1 0	1 2	0	8 8	0 0	50 41	0	266 263	1222 1151
6:15 PM	Ō	69	10	0	65	64	Ő	0	0	0	ō	Ő	8	Ö	43	0	259	1084
6:30 PM 6:45 PM	0 1	57 95	13 10	0	41 42	54 77	0	0 0	0	0 0	0 1	0	3 8	1 0	30 54	0 0	199 288	987 1009
Peak 15-Min	-	North	bound		12	South	bound	5	Ŭ	Easth	ound	5	5	West	bound	5	200	
Flowrates	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	To	tal
All Vehicles Heavy Trucks	0 0	288 8	52 0	0	292 4	408 8	0	0	0 0	4 0	0	0	68 0	0 0	224 4	0	13 2	36 4
Pedestrians Bicycles Scooters	0	0 0	0		0	0 0	0		0	4 0	0		0	0 0	0		2)

LOCATION: F CITY/STATE:	airvie [.] Mana	w Ave - ssas, V	Wesle A	ey Ave	2										QC DATE:	: JOB # Tue, N	‡: 1583 ⁄Jay 24	37605 2022
5 ★ 5 . 0 • 6 ★ 1 ·	236 4 23 4 23 4 23 4 23 4 23 4 23 4 23 5 4 4 23 5 4 4 23 5 4 6 4 7 4 9 4 1 49 235	502 1 1 4 4 4 2 497	2 + 5 0 3 + 2			Pea Pea	ak-Hou k 15-M Qual	r: 8:00 in: 8:0		9:00 / 8:15	AM AM		1	0 ★ 0 0 6.7 ★ 100			0 • 0 33.3 •	20
o 1		→ [] → [0		-		_ ↓			Ę	∰ \$	-		0 1 0			0 0 7 0	
• N/A • r	• N/.		N/A ➡		-	. :				₽		-		N/A			► N/A	
15-Min Count Period		Fairvie (North	w Ave bound)			Fairvie (South	w Ave bound)			Wesle (Eastb	ey Ave ound)			Wesle (Westl	ey Ave bound)		Total	Hourly Totals
6:00 AM 6:15 AM 6:30 AM 6:45 AM 7:00 AM 7:15 AM 7:30 AM 7:45 AM 8:00 AM	Left 0 0 0 0 0 0 0 0 1	Thru 41 38 55 84 76 87 109 107 148	Right 0 0 0 1 0 0	U 0 0 0 0 0 0 0 0 0 0	Left 0 0 1 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	Thru 18 22 27 50 68 48 44 64	Right 0 0 1 2 0 1 2 0 1 0 1	U 0 0 0 0 0 0 0 0 0 0	Left 0 1 1 2 1 0 0 2 1 1 1 0 0 1 1 1 0 0 1 1 1 0 0 1 1 1 1	Thru 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	Right 0	U 0 0 0 0 0 0 0 0 0	Left 0 1 0 1 0 0 1 0 1 0 1 1 1 1 1 1 1 1 1	Thru 0 0 0 0 0 1 0 0	Right 0 0 0 1 0 1 0 1 0	U 0 0 0 0 0 0 0 0 0 0	59 62 80 117 131 160 160 155 216	318 390 488 568 606 691
8:15 AM 8:30 AM 8:45 AM	0 0 0	109 117 120	1 0 1	0 0 0	0 0 0	75 50 42	2 2 0	1 0 0	1 2 1	0 0 0	1 0 0	0 0 0	0 2 0	0 0 0	0 1 0	0 0 0	190 174 164	721 735 744
Peak 15-Min		North	bound			South	bound			Eastb	ound			West	bound		To	tal
All Vehicles	Left	Thru 592	Right	U	Left	Thru 256	Right	U	Left 4	Thru	Right	U	Left 4	Thru	Right 4	U	.0	54
Heavy Trucks	0	16	0	U	0	8	0	U	0	0	0	0	0	0	0	U	2	4
Pedestrians Bicycles Scooters	0	0 0	0		0	0 0	0		0	0 0	0		0	0 0	0		()
Comments:																		

Location: F City/state:	airvie Mana	w Ave - ssas, V	Wesle A	ey Ave	9										QC DATE:	C <mark>JOB</mark> # Tue, N	‡: 1583 Лау 24	37606 2022
10 + 2 . 0 • 5 + 3 •	439 7 43 7 43 4 9 9 9 9 9 9 9 9 9 9 9 9 9	372 1 1 1 1 • • • • • • • • • • • • •	0 + 1 0 1 + 0			Pe Pea	ak-Hou k 15-M	In: 4:19	S PM	- 5:15 I 4:30	PM PM			0 + 0 0 0 + 0			0 + 0 0 +	0
o 1		→ [] → [3		-		∲				∰ • \$−	-		0 0 0			0	
+ 3 N/A + + 7	* N// • •		∧/A ◆		-	-\$ 	•			\$		-		N/A			N/A	
15-Min Count Period		Fairvie (North	w Ave bound)			Fairvie (South	w Ave bound)			Wesle (Eastb	ey Ave oound)			Wesl (West	ey Ave bound)		Total	Hourly Totals
	Left	Inru 83	Right	0	Left	Inru	Right	0	Left	Inru	Right	0	Left	Inru	Right	0	176	
4:15 PM	1	100	0	0	0	105	3	0	0	0	3	0	0	0	0	0	212	
4:30 PM 4:45 PM	0	100 82	0	0	0	100 104	0	0	1 1	0	0	0	0	0	0	0	201 193	782
5:00 PM	1	87	0	0	0	122	1	0	0	0	0	0	0	0	0	0	211	817
5:30 PM	1	84	0	1	0	85	0	0	2	0	0	0	0	0	2 0	0	192	770
5:45 PM 6:00 PM	0	73 79	1	0	1	80 76	3 1	0	4	0	1 1	0	0	0	0	0	163 159	740 688
6:15 PM	1	82	0	0	1	71	Ō	Ő	0	0	Ō	Ő	0	0	1	Ő	156	652
6:30 PM 6:45 PM	1 0	66 107	U 0	0	0	59 80	0 1	0 0	2	0	0	0	0	0	0 0	0	128 188	606 631
Peak 15-Min		North	bound			South	bound			Eastb	ound			West	bound		т.,	hal
Flowrates	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	10	ldi
All Vehicles Heavy Trucks	4 0	400 4	0 0	0	0 0	420 4	12 0	0	0 0	0 0	12 0	0	0 0	0 0	0 0	0	84 8	8
Pedestrians Bicycles Scooters	0	0 0	0		0	0 0	0		0	0 0	0		0	4 0	0		4	
Comments:																		

LOCATION: F CITY/STATE:	airvie Mana	w Ave ssas, V	Richn A	nond A	Ave										QC DATE:	: <mark>JOB</mark> i Tue, I	#: 1583 May 24	37607 2022
$9 \leftarrow 2$ $1 \leftarrow 4$ $6 \leftarrow 3$	236 3 22: 4 4 09 4 46: 305	486 1 12 4 • • 4 • • 2 107 573	22 + 105 2 81 + 120			Pe Pea	ak-Hou k 15-M Qua	r: 7:30 in: 8:0		- 8:30 / 8:15	AM AM			11.1 + 100 0 33.3 + 0	3 0 2 	4.7 → 16.7 → 4.7 → 4.5	t 4.5 + 1 + 0 c 25 + 1	29
1		→	5		-		∲				ᡂ +\$	-		0 0 0			€ 0 ● 0 F 0	
+ 3 N/A → + 3	N/2		↓ N/A ↓		-	-\$	•					-		N/A			N/A	
15-Min Count Period		Fairvie (North	w Ave bound)			Fairvie (South	w Ave bound)			Richmo (Eastb	ond Ave bound)			Richmo (West	ond Ave bound)		Total	Hourly
Beginning At	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U		TULAIS
6:00 AM 6:15 AM	0	41 34	11 9	0	1	19 20	1 0	0	0	0	0	0 0	7 13	0	1 3	0	81 81	
6:30 AM	1	49	22	0	2	20	0	0	0	0	0	0	9	0	2	0	105	117
7:00 AM	1	74	34	0	1	48	0	0	2	1	0	0	24	0	2	0	187	523
7:15 AM 7:30 AM	0	82 105	33 29	0	1	69 46	1	0	0	0	0	0	31 30	0	5	0	222 223	664 782
7:45 AM	1	117	31	0	5	37	2	0	0	0	1	0	15	0	4	0	213	845
8:15 AM	1	136	25	0	2	76	1	0	1	0	2	0	21	1	5	0	239	920
8:30 AM	1	114	20	0	2	49	1	0	0	0	1	0	19	0	3	0	210	907
Dook 15 Min	U	North	bound	U	1	South	bound	0	1	Fasth	ound	0	12	West	+ bound	U	212	505
Flowrates	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	To	tal
All Vehicles Heavy Trucks	0 0	544 8	88 0	0	8 4	248 4	0 0	0	4 4	0 0	0 0	0	60 0	0 0	28 0	0	98 2	30 0
Buses Pedestrians Bicycles Scooters	0	0 0	0		0	28 0	0		0	0 0	0		0	8 0	0		3	6)
Comments:																		

Location: F City/state:	airvie Mana	w Ave ssas, V	Richn A	nond A	Ave										QC DATE:	C JOB i Tue, I	#: 1583 May 24	37608 2022
12 + 3 = 0	431 4 409 • • • 7 364 561	376 18 18 4 5 5 5 376 4 18 4 5 18 5 18 5 18 5 18 5 18 5 18 5 18 5 18 5 18 18 18 18 18 18 18 18 18 18	9 🔶 159 2 148 🔸 177			Pe Pea	ak-Hou k 15-N Qua			- 5:15 5:15	PM PM			0 + 0 0 + 0			• 0 • 0 • 2.7 •	25
1		• •] • [2		-		↓				∰ + \$	-		0 1 0			■ 0 ■ 0	
• • • N/A + • •	* N/4 • *	+ - - - - - +	N/A ≯		-	-\$ \$100	→ }			\$		-		N/A			⊾ ► N/A	
15-Min Count Period		Fairvie (North	w Ave bound)			Fairvie (South	w Ave bound)			Richmo (Eastb	ond Ave oound)			Richm (West	ond Ave bound)		Total	Hourly Totals
Beginning At	Left	Thru 76	Right	U	Left	Thru 29	Right	U	Left	Thru	Right	U 0	Left	Thru	Right	U 0	230	
4:15 PM	2	102	42	0	7	98	2	0	0	0	1	0	29	0	0	0	283	
4:30 PM 4:45 PM	1	98 78	58 42	1	4	97 98	0	0	1	0	0	0	38	1	2	0	282	1063
5:00 PM 5:15 PM	2	86 73	37 42	0	5	116	1	0	0	0	0	0	40 31	1	3	0	293 260	1126 1103
5:30 PM	1	82	52	0	2	82 81	1	0	0	Ō	1	0	36	1	5	0	263	1084
6:00 PM	0	81	38	1	3	75	0	0	1	0	0	0	31	0	2	0	232	979
6:15 PM 6:30 PM	1 1	82 60	34 29	0	2	68 54	2	0	1	0	2	0 0	37 27	0	1	0	230 179	949 865
6:45 PM	4	95	37	õ	9	74	0	Ő	1	0	Ő	õ	20	0	5	0	245	886
Peak 15-Min	4.1	North	bound		4.1	South	bound		1.4	Eastb	ound		1.4	West	bound		То	tal
	Left	1 nru 3//	right	U	Left	1 nru 464	Right	U	Left	inru	Right	U	Left	Inru		0	11	72
Heavy Trucks	0	0	0	U	0	8	0	0	0	0	0	0	8	4	0	0	11	6
Buses Pedestrians Bicycles Scooters	0	0 0	0		0	0 0	0		0	0 0	0		0	4 0	0		2	L)
Comments:																		

Location: F City/state:	airvie Mana	w Ave - ssas, V/	- Welli A	ngton	Rd										QC DATE:	: JOB 	‡: 1583 ∕lay 24	37609 2022
542 + 239 311 550 + 0	308 163 0 4 5 6 0 0 0 163 0 163 0	571 145 145 5 6 6 7 7 7 7 7 7 7 7 7 7 7 7 7	332 ← 711 79 0 ← 456			Pe Pea	ak-Hou k 15-M DATA TH	r: 7:30 in: 8:0		- 8:30 / 8:15	AM AM		2	4.1 ← 5 6.8 6 ← 0			4.7 ■ 4.7	44
2		→ [] → [0		-	3	, l	Ļ				-		0 0 0			0 0 0	
+ 3 N/A + + 3	* N/3 - * * - *		N/A ≁		-		* * *			[\$	-		N/A			N/A	
15-Min Count Period		Fairvie (Northl	w Ave bound)			Fairvie (South	w Ave bound)			Welling (Eastb	gton Rd oound)			Welling (West	gton Rd bound)		Total	Hourly Totals
6:00 AM 6:15 AM 6:30 AM 6:45 AM 7:00 AM 7:15 AM 7:30 AM 7:45 AM 8:00 AM 8:15 AM 8:30 AM 8:30 AM 8:30 AM	Left 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	Thru 0	Right 0	U 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	Left 13 18 12 15 20 28 21 28 21 28 38 58 40 26	Thru 0	Right 10 19 17 22 52 70 56 32 33 31	U 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	Left 33 25 45 71 68 67 64 64 64 55 56 46 66	Thru 59 68 61 56 78 85 79 67 80 85 73 64	Right 0	U 0 0 1 0 1 0 0 0 0 0 0 0 0 0 0	Left 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	Thru 32 37 44 72 63 65 80 105 103 91 90 99	Right 18 18 28 35 42 82 72 82 105 73 90 87	U 0 0 0 0 1 0 0 0 0 0 0 0 0 0 0	165 185 207 272 323 365 372 378 414 405 372 373	829 987 1167 1332 1438 1529 1569 1569 1564
Peak 15-Min Flowrates	Left	North Thru	ound Right	U	Left	South Thru	bound Right	U	Left	Eastb Thru	ound Right	U	Left	Westl Thru	oound Right	U	Tot	tal
All Vehicles	0	0	0	0	152	0	132	0	220 4	320	0	0	0	412	420	0	16. 3	56 2
Buses Pedestrians Bicycles Scooters	0	4 0	0		0	4 0	0		0	0 0	0		0	0 0	0		8 0	<u>-</u>

LOCATION: F	airvie Manas	w Ave ssas, V	Welli A	ngton	Rd										QC DATE:	C <mark>JOB</mark> 4 Tue, N	#: 1583 May 24	37610 2022
733 ← 259 526 785 ← 0	570 279 0 279 0 4 5 6 0 0 0	527 * 291 • • • • • • • • • • • • •	269 ← 722 453 0 ← 817			Pe Pea	ak-Hou k 15-M DATA TH		5 PM	- 5:15 5:15	PM PM			23 ★ 15 08 1 ★ 0			15 + 18 7 0 +	17 09
0		• [•] • [0		-		↓ ↓	Ļ				-		0 0 0			0 • 3 • 0	
+ 3 N/A + + 3	* N/2 • • • • •	+ 	N/A ✦		-		≁				1	-		N/A			► N/A	
15-Min Count Period Reginning At		Fairvie (North	w Ave bound)			Fairvie (South	w Ave bound)			Wellin (Eastk	gton Rd bound)			Wellin (West	gton Rd bound)		Total	Hourly Totals
4:00 PM 4:15 PM 4:30 PM 4:45 PM	Left 0 0 0 0	Thru 0 0 0 0	Right 0 0 0 0	0 0 0 0	Left 62 69 61 72	Thru 0 0 0 0	Right 48 58 75 69	0 0 0 0	Left 53 69 65 64	Thru 140 117 124 141	Right 0 0 0 0	0 1 0 0	Left 0 0 0 0	Thru 98 130 111 108	Right 52 76 72 59	0 0 0 0	453 520 508 513	1994
5:15 PM 5:30 PM 5:45 PM 6:00 PM 6:15 PM 6:30 PM 6:45 PM	0 0 0 0 0 0 0	0 0 0 0 0 0 0	0 0 0 0 0 0 0	0 0 0 0 0 0 0	71 61 51 48 35 47	0 0 0 0 0 0 0	57 56 60 57 57 44 46	0 0 0 0 0 0 0	61 75 58 56 57 41 71	144 105 130 121 101 108 85 75	0 0 0 0 0 0 0	0 0 0 0 0 0 0	0 0 0 0 0 0 0	97 90 104 110 85 104 117	51 61 49 63 59 48 65	0 0 0 0 0 0 0	442 473 443 438 414 357 421	1999 1964 1894 1796 1768 1652 1630
Peak 15-Min Flowrates	Left	North Thru	bound Right	U	Left	South Thru	bound Right	U	Left	Eastb Thru	oound Right	U	Left	West Thru	bound Right	U	То	tal
All Vehicles Heavy Trucks Buses	0	0	0	0	356 4	0	308 16	0	240 0	576 0	0	0	0	416 0	248 4	0	21 2	44 4
Pedestrians Bicycles Scooters Comments:	0	0 0	0		0	0 0	0		0	0 0	0		0	0 0	0		()

Table: Phase Timing Plans [Timing Plan: 1]												
Phase	1	2	3	4	5	6						
Walk	0	7	7	7	0	7						
Ped Clear	0	5	12	11	0	16						
Steady Don't Walk	0	0	0	0	0	0						
Min Green	8	10	8	8	8	10						
Min Green2	0	0	0	0	0	0						
Passage	3	5	4	3	3	5						
Passage 2	0	0	0	0	0	0						
Max 1	20	40	30	30	20	40						
Max 2	20	40	25	25	20	40						
Max 3	0	0	0	0	0	0						
Conditional Max	0	0	0	0	0	0						
Yellow Change	4	4	4	4	4	4						
Red Clear	2.5	2.5	2.1	2.3	2.5	2.5						



	0	-	0			
Phase	1	2	3	4	5	6
Walk	0	0	0	0	0	0
Ped Clear	0	0	0	0	0	0
Steady Don't Walk	0	0	0	0	0	0
Min Green	0	10	8	0	10	10
Min Green2	0	0	0	0	0	0
Passage	0	4	3.5	1	3.5	4
Passage 2	0	0	0	0	0	0
Max 1	0	55	30	0	25	55
Max 2	0	50	30	0	45	50
Max 3	0	0	0	0	0	0
Conditional Max	0	0	0	0	0	0
Yellow Change	0	4.2	4	4	4.2	4.2
Red Clear	0	2.9	2.9	2	2.9	2.9

Table: Phase Timing Plans [Timing Plan: 1]



Table: P	attern Parameter	5																	
Pattern	Cycle Time Offset 1	Offset 2	Offset 3	Sp	lit NumberSeq Num	nber Ref Point	Coord Mode Force Off	Max Mode	Trans Pe	d McMin Permis	si Correction M	Single Perm SPhase Plan	Overl	ap Plan Veh D	etecto: Veh	Detector Ped D	etector Ped [etector Pri/Pr	e Deter Description
1	11 60	42	0	0	11	1 Green	Actuated Coc Fixed	Max 2	Phase	Phase Only	Shortway (Au	0	1	1	1	1	1	1	1 AM Peak
2	21 60	37	0	0	21	1 Green	Actuated Coc Fixed	Max 2	Phase	Phase Only	Shortway (Au	0	1	1	1	1	1	1	1 Mid Peak
3	31 75	13	0	0	31	1 Green	Actuated Coc Fixed	Max 2	Phase	Phase Only	Shortway (Au	0	1	1	1	1	1	1	1 PM Peak Lite
4	41 70	67	0	0	41	1 Green	Actuated Coc Fixed	Max 2	Phase	Phase Only	Shortway (Au	0	1	1	1	1	1	1	1 Weekend

Table: Coordination Parameters

Operational I Coordination Max Mode	Force Mode	Correction M Max Cyc	Limi Min Cyc L	imi Max Dwell	Transition Ped Mode
Actuated Coc Maximum 2	Fixed	Shortway (Au	25	25	Pattern

Table:	Split Para	ameters [S	Split: 11]									
Phase	Time	Min	Max	С	oord Pha	ise Ref Point	Tran	s Cover FForce Off	Mo Mode	Pri Min	Pri Max	Pri Force Off Mode
	2	36	0	0	TRUE	TRUE		0 Fixed	None		0	0 Float
	3	24	0	0		0	0	0 Fixed	None		0	0 Float
	5	18	0	0		0	0	0 Fixed	None		0	0 Float
	6	18	0	0	TRUE	TRUE		0 Fixed	None		0	0 Float

Table. Split Parameters (Split. 21	Table: S	plit Pa	rameters	[S	plit:	21	1
------------------------------------	----------	---------	----------	----	-------	----	---

Table:	Split Para	ameters [S	Split: 21]									
Phase	Time	Min	Max	С	oord Pha	ise Ref Point	Tran	ns Cover FForce Off	Mo Mode	Pri Min	Pri Max	Pri Force Off Mode
	2	36	0	0	TRUE	TRUE		0 Fixed	None		0	0 Float
	3	24	0	0		0	0	0 Fixed	None		0	0 Float
	5	18	0	0		0	0	0 Fixed	None		0	0 Float
	6	18	0	0	TRUE	TRUE		0 Fixed	None		0	0 Float

Table:	Split Para	ameters [S	Split: 31]									
Phase	Time	Min	Max	С	oord Pha	ise Ref Point	Tran	s Cover FForce Off	Mo Mode	Pri Min	Pri Max	Pri Force Off Mode
	2	40	0	0	TRUE	TRUE		0 Fixed	None		0	0 Float
	3	35	0	0		0	0	0 Fixed	None		0	0 Float
	5	18	0	0		0	0	0 Fixed	None		0	0 Float
	6	22	0	0	TRUE	TRUE		0 Fixed	None		0	0 Float

Table: Split Parameters (Spl

Table:	Split Para	ameters [Split: 41]									
Phase	Time	Min	Max	С	oord Pha	ase Ref Point	Trar	ns Cover I Force Off	Mc Mode	Pri Min	Pri Max	Pri Force Off Mode
	2	49	0	0	TRUE	TRUE		0 Fixed	None		0	0 Float
	3	21	0	0		0	0	0 Fixed	None		0	0 Float
	5	21	0	0		0	0	0 Fixed	None		0	0 Float
	6	28	0	0	TRUE	TRUE		0 Fixed	None		0	0 Float

Event Hour Minute Action Description										
Event	Hour	Minute	Action	Description						
	1	8	0	11 AM						
	2	10	0	21 MID						
	3	16	0	31 PM						
	4	19	0	50 Free						

Table: Day Plan Events [Day Plan: 1]

Table:	Day Plan	Events [Da	iy Plan: Z		
Event	Hour	Minute	Action	Description	
	1	0	0	50 Free	

Table: Day Plan Events [Day Plan: 2]

C. VDOT Crash Data

VDOT Crash Data Summary Table

Crash Data for the Intersection of Fairview Avenue and Tudor Lane (2014 - 2019)											
Document Number	Date	Crash Severity	Collsion Type	Pedestrain Injury	Persons Injured	Fatalities	Direction of Travel	Work Zone Related	Adverse Weather Conditions	Distracted Driver	
160845085	3/23/2016	PDO.Property Damage Only	2. Angle	0	0	0	South	no	no	no	
180165266	1/11/2018	PDO.Property Damage Only	9. Fixed Object - Off Road	0	0	0		yes	no	no	
181565196	5/30/2018	C.Nonvisible Injury	12. Ped	0	1	0		no	no	no	
183515272	12/15/2018	PDO.Property Damage Only	1. Rear End	0	0	0		no	yes	no	
	Crash	n Data for th	ne Intersec	tion of Fa	airview Ave	nue and	Signal Hill Road ((2014 - 20 ⁻	19)		
--------------------	------------	-----------------------------	-------------------------------	----------------------	-----------------	------------	---------------------	-------------------------	----------------------------------	----------------------	
Document Number	Date	Crash Severity	Collsion Type	Pedestrain Injury	Persons Injured	Fatalities	Direction of Travel	Work Zone Related	Adverse Weather Conditions	Distracted Driver	
141255096	5/3/2014	A.Severe Injury	2. Angle	0	2	0		no	no	yes	
141345201	5/14/2014	PDO.Property Damage Only	1. Rear End	0	0	0		no	no	no	
163215208	10/28/2016	PDO.Property Damage Only	9. Fixed Object - Off Road	0	0	0		no	no	no	

	Crash	n Data for th	ne Interse	ction of Fa	airview Av	enue and	Wesley Avenue (2014 - 20 1	9)	
Document Number	Date	Crash Severity	Collsion Type	Pedestrain Injury	Persons Injured	Fatalities	Direction of Travel	Work Zone Related	Adverse Weather Conditions	Distracted Driver
143425209	12/6/2014	B.Visible Injury	8. Non-Collision	0	1	0		no	yes	yes

	Crash	Data for the	Intersect	ion of Fai	irview Aver	nue and F	Richmond Avenue	(2014 - 2	019)	
Document Number	Date	Crash Severity	Collsion Type	Pedestrain Injury	Persons Injured	Fatalities	Direction of Travel	Work Zone Related	Adverse Weather Conditions	Distracted Driver
172055176	7/19/2017	C.Nonvisible Injury	2. Angle	0	1	0		no	no	no
183175462	10/26/2018	C.Nonvisible Injury	2. Angle	0	2	0		no	yes	no

	Crash	Data for th	e Intersec	tion of Fa	airview Ave	nue and	Wellington Road	(2014 - 20	19)	
Document Number	Date	Crash Severity	Collsion Type	Pedestrain Injury	Persons Injured	Fatalities	Direction of Travel	Work Zone Related	Adverse Weather Conditions	Distracted Driver
140915058	3/25/2014	C.Nonvisible Injury	9. Fixed Object - Off Road	0	1	0		no	yes	no
143445162	12/9/2014	C.Nonvisible Injury	2. Angle	0	1	0		no	yes	no
150305084	1/29/2015	PDO.Property Damage Only	1. Rear End	0	0	0		no	yes	no
151135165	4/22/2015	C.Nonvisible Injury	1. Rear End	0	1	0		no	no	no
152015216	7/16/2015	PDO.Property Damage Only	1. Rear End	0	0	0		no	no	no
152165097	7/29/2015	A.Severe Injury	1. Rear End	0	1	0		no	yes	no
152995140	10/25/2015	C.Nonvisible Injury	2. Angle	0	2	0		no	no	no
153075130	11/2/2015	C.Nonvisible Injury	2. Angle	0	1	0	West	no	no	no
160475269	2/15/2016	C.Nonvisible Injury	6. Fixed Object in Road	0	1	0		no	yes	no
161675176	6/14/2016	PDO.Property Damage Only	2. Angle	0	0	0		no	no	no
163655184	12/29/2016	C.Nonvisible Injury	1. Rear End	0	2	0		no	no	yes
171005284	3/22/2017	C.Nonvisible Injury	2. Angle	0	3	0		no	no	no
171875229	7/4/2017	PDO.Property Damage Only	3. Head On	0	0	0		no	no	no
182995170	10/25/2018	B.Visible Injury	1. Rear End	0	1	0		no	no	no
190565158	2/22/2019	B.Visible Injury	1. Rear End	0	2	0		no	no	no

D. HCM Level of Service Definitions



TECHNICAL MEMORANDUM

Subject: Level of Service Definitions

Introduction

The purpose of this memorandum is to define the level of service (LOS) metric that commonly used as a measure of effectiveness (MOE) for traffic operations.

All capacity analyses are based on the procedures specified by the Transportation Research Board's (TRB) <u>Highway Capacity</u> <u>Manual</u> (HCM), which is currently on its sixth edition. Level of service ranges from A to F. A brief description of each level of service for signalized and unsignalized intersections is provided below.

Signalized Intersections

Level of service is based upon the traffic volume present in each lane on the roadway, the capacity of each lane at the intersection and the delay associated with each directional movement. The levels of service for signalized intersections are defined below:

- <u>Level of Service A</u> describes operations with very low average delay per vehicle, i.e., less than 10.0 seconds. This occurs when progression is extremely favorable, and most vehicles arrive during the green phase. Most vehicles do not stop. Short signal cycle lengths may also contribute to low delay.
- <u>Level of Service B</u> describes operations with average delay in the range of 10.1 to 20.0 seconds per vehicle. This generally occurs with good progression and/or short cycle lengths. More vehicles stop than for LOS A, causing higher levels of average delay.
- <u>Level of Service C</u> describes operations with delay in the range of 20.1 to 35.0 seconds per vehicle. These higher delays may result from fair progression and/or longer cycle lengths. Individual cycle failures may begin to appear at this level. The number of vehicles stopping is significant at this level although many still pass through the intersection without stopping. This is generally considered the lower end of the range of the acceptable level of service in rural areas.
- Level of Service D describes operations with delay in the range of 35.1 to 55.0 seconds per vehicle. At LOS D, the influence of congestion becomes more noticeable. Longer delays may result from some combination of unfavorable progression, long cycle lengths, and/or high traffic volumes as compared to the roadway capacity. Many vehicles are required to stop and the number of vehicles that do not have to stop declines. Individual signal cycle failures, where all waiting vehicles do not clear the intersection during a single green time, are noticeable. This is generally considered the lower end of the range of the acceptable level of service in urban areas.
- <u>Level of Service E</u> describes operations with delay in the range of 55.1 to 80.0 seconds per vehicle. These higher delay values generally indicate poor progression, long cycle lengths, and high traffic volumes. Individual cycle failures are frequent occurrences. LOS E has been set as the limit of acceptable conditions.
- Level of Service F describes operations with average delay in excess of 80.0 seconds per vehicle. This is considered to be unacceptable to most drivers. This condition often occurs with over-saturation, i.e., when traffic arrives at a flow rate that exceeds the capacity of the intersection. It may also occur at high volumes with many individual cycle failures. Poor progression and long cycle lengths may also contribute to such delays.

Unsignalized Intersections

At an unsignalized intersection, the major street through traffic and right-turns are assumed to operate unimpeded and therefore receive no level of service rating. The level of service for the minor street and the major street left-turn traffic is dependent on the volume and capacity of the available lanes, and, the number and frequency of acceptable gaps in the major street traffic to make a conflicting turn. The level of service grade is provided for each conflicting movement at an unsignalized intersection and is based on the total average delay experienced by each vehicle. The delay includes the time it takes a vehicle to move from the back of a queue through the intersection.

The unsignalized intersection level of service analysis does not account for variations in driver behavior or the effects of nearby traffic signals. Therefore, the results from this analysis usually indicate worse levels of service than may be experienced in the field. The unsignalized intersection level of service descriptions are provided below:

- <u>Level of Service A</u> describes operations where there is very little to no conflicting traffic for a minor side street movement, i.e., an average total delay of less than 10.0 seconds per vehicle.
- Level of Service B describes operations with average total delay in the range of 10.1 to 15.0 seconds per vehicle.
- Level of Service C describes operations with average total delay in the range of 15.1 to 25.0 second per vehicle.
- Level of Service D describes operations with average total delay in the range of 25.1 to 35.0 seconds per vehicle.
- <u>Level of Service E</u> describes operations with average total delay in the range of 35.1 to 50.0 seconds per vehicle.
- Level of Service F describes operations with average total delay of 50 seconds per vehicle. LOS F exists when there are insufficient gaps of suitable size to allow a side street demand to cross safely through or enter a major street traffic stream. This level of service is generally evident from extremely long total delays experienced by side street traffic and by queuing on the minor approaches. It is important to note that LOS F may not always result in long queues but may result in adjustments to normal driver behavior.

E. Intersection Capacity Analysis Worksheets – 2022 Existing Conditions

5

Intersection

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		\$			\$		۳	et -			\$	
Traffic Vol, veh/h	21	1	160	0	1	5	258	473	1	1	222	50
Future Vol, veh/h	21	1	160	0	1	5	258	473	1	1	222	50
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None									
Storage Length	-	-	-	-	-	-	280	-	-	-	-	-
Veh in Median Storage,	# -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	92	92	92	92	92	92	92	92	92	92	92	92
Heavy Vehicles, %	0	0	6	0	0	20	3	3	0	0	2	6
Mvmt Flow	23	1	174	0	1	5	280	514	1	1	241	54

Major/Minor	Minor2		ľ	Minor1			Major1			M	ajor2			
Conflicting Flow All	1348	1345	268	1433	1372	515	295	0	C)	515	0	0	
Stage 1	270	270	-	1075	1075	-	-	-	-	-	-	-	-	
Stage 2	1078	1075	-	358	297	-	-	-	-	-	-	-	-	
Critical Hdwy	7.1	6.5	6.26	7.1	6.5	6.4	4.13	-	-	-	4.1	-	-	
Critical Hdwy Stg 1	6.1	5.5	-	6.1	5.5	-	-	-	-	-	-	-	-	
Critical Hdwy Stg 2	6.1	5.5	-	6.1	5.5	-	-	-	-	-	-	-	-	
Follow-up Hdwy	3.5	4	3.354	3.5	4	3.48	2.227	-	-	-	2.2	-	-	
Pot Cap-1 Maneuver	129	153	761	113	147	526	1261	-	-	- '	1061	-	-	
Stage 1	740	690	-	268	298	-	-	-	-	-	-	-	-	
Stage 2	267	298	-	664	671	-	-	-	-	-	-	-	-	
Platoon blocked, %								-	-	-		-	-	
Mov Cap-1 Maneuver	105	119	761	72	114	526	1261	-	-	- '	1061	-	-	
Mov Cap-2 Maneuver	105	119	-	72	114	-	-	-	-	-	-	-	-	
Stage 1	576	689	-	209	232	-	-	-	-	-	-	-	-	
Stage 2	205	232	-	511	670	-	-	-	-	-	-	-	-	

Approach	EB	WB	NB	SB	
HCM Control Delay, s	20	16.2	3.1	0	
HCM LOS	С	С			

Minor Lane/Major Mvmt	NBL	NBT	NBR	EBLn1V	/BLn1	SBL	SBT	SBR	
Capacity (veh/h)	1261	-	-	435	328	1061	-	-	
HCM Lane V/C Ratio	0.222	-	-	0.455	0.02	0.001	-	-	
HCM Control Delay (s)	8.7	-	-	20	16.2	8.4	0	-	
HCM Lane LOS	А	-	-	С	С	А	А	-	
HCM 95th %tile Q(veh)	0.9	-	-	2.3	0.1	0	-	-	

	-	+	•	1	1	Ŧ
Lane Group	EBT	WBT	WBR	NBT	SBL	SBT
Lane Group Flow (vph)	6	32	317	504	182	222
v/c Ratio	0.03	0.13	0.65	0.65	0.39	0.18
Control Delay	35.2	31.3	11.1	23.1	8.8	6.6
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	35.2	31.3	11.1	23.1	8.8	6.6
Queue Length 50th (ft)	2	12	0	143	21	25
Queue Length 95th (ft)	16	43	75	#445	89	106
Internal Link Dist (ft)	120	611		282		588
Turn Bay Length (ft)			110		260	
Base Capacity (vph)	613	597	746	855	542	1218
Starvation Cap Reductn	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0
Reduced v/c Ratio	0.01	0.05	0.42	0.59	0.34	0.18
Intersection Summary						

Intersection Summary

95th percentile volume exceeds capacity, queue may be longer. Queue shown is maximum after two cycles. #

HCM Signalized Intersection Capacity Analysis 2: Fairview Ave & Manassas PD Driveway/Signal Hill Rd

	₫	٭	-	\rightarrow	1	-	*	1	1	1	1	ŧ
Movement	EBU	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT
Lane Configurations			4			र्स	1	ሻ	f,		5	î,
Traffic Volume (vph)	1	4	1	0	29	1	301	0	436	43	173	205
Future Volume (vph)	1	4	1	0	29	1	301	0	436	43	173	205
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)			6.3			6.1	6.1		6.5		6.5	6.5
Lane Util. Factor			1.00			1.00	1.00		1.00		1.00	1.00
Frt			1.00			1.00	0.85		0.99		1.00	1.00
Flt Protected			0.96			0.95	1.00		1.00		0.95	1.00
Satd. Flow (prot)			1824			1761	1583		1795		1719	1856
Flt Permitted			0.96			0.95	1.00		1.00		0.25	1.00
Satd. Flow (perm)			1824			1761	1583		1795		458	1856
Peak-hour factor, PHF	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Adj. Flow (vph)	1	4	1	0	31	1	317	0	459	45	182	216
RTOR Reduction (vph)	0	0	0	0	0	0	277	0	2	0	0	0
Lane Group Flow (vph)	0	0	6	0	0	32	40	0	502	0	182	222
Heavy Vehicles (%)	0%	0%	0%	0%	3%	0%	2%	0%	4%	9%	5%	2%
Turn Type	Split	Split	NA		Split	NA	Perm	pm+pt	NA		pm+pt	NA
Protected Phases	4	4	4		3	3		5	2		1	6
Permitted Phases							3	2			6	
Actuated Green, G (s)			1.2			9.8	9.8		31.4		47.4	47.4
Effective Green, g (s)			1.2			9.8	9.8		31.4		47.4	47.4
Actuated g/C Ratio			0.02			0.13	0.13		0.41		0.61	0.61
Clearance Time (s)			6.3			6.1	6.1		6.5		6.5	6.5
Vehicle Extension (s)			3.0			4.0	4.0		5.0		3.0	5.0
Lane Grp Cap (vph)			28			223	200		729		435	1138
v/s Ratio Prot			c0.00			0.02			c0.28		c0.05	0.12
v/s Ratio Perm							c0.03				0.20	
v/c Ratio			0.21			0.14	0.20		0.69		0.42	0.19
Uniform Delay, d1			37.6			30.0	30.2		18.9		9.1	6.6
Progression Factor			1.00			1.00	1.00		1.00		1.00	1.00
Incremental Delay, d2			3.8			0.4	0.7		3.5		0.7	0.2
Delay (s)			41.4			30.4	30.9		22.4		9.7	6.7
Level of Service			D			С	С		С		А	A
Approach Delay (s)			41.4			30.9			22.4			8.1
Approach LOS			D			С			С			A
Intersection Summary												
HCM 2000 Control Delay			20.3	Н	CM 2000	Level of	Service		С			
HCM 2000 Volume to Capacit	ty ratio		0.54									
Actuated Cycle Length (s)			77.3	S	um of los	t time (s)			25.4			
Intersection Capacity Utilization	on		66.6%	IC	CU Level	of Service	<u>;</u>		С			
Analysis Period (min)			15									
c Critical Lane Group												

Timing Plan: EX AM

1	
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Movement	SBR
Lane Configurations	
Traffic Volume (vph)	6
Future Volume (vph)	6
Ideal Flow (vphpl)	1900
Total Lost time (s)	
Lane Util. Factor	
Frt	
Flt Protected	
Satd. Flow (prot)	
Flt Permitted	
Satd. Flow (perm)	
Peak-hour factor, PHF	0.95
Adj. Flow (vph)	6
RTOR Reduction (vph)	0
Lane Group Flow (vph)	0
Heavy Vehicles (%)	0%
Turn Type	
Protected Phases	
Permitted Phases	
Actuated Green, G (s)	
Effective Green, g (s)	
Actuated g/C Ratio	
Clearance Time (s)	
Vehicle Extension (s)	
Lane Grp Cap (vph)	
v/s Ratio Prot	
v/s Ratio Perm	
v/c Ratio	
Uniform Delay, d1	
Progression Factor	
Incremental Delay, d2	
Delay (s)	
Level of Service	
Approach Delay (s)	
Approach LOS	
Intersection Summary	

HCM 2010 analysis expects strict NEMA phasing.

Intersection

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			र्च			eî 👘	
Traffic Vol, veh/h	4	0	1	2	1	2	1	473	2	1	231	3
Future Vol, veh/h	4	0	1	2	1	2	1	473	2	1	231	3
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None									
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage,	# -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	85	85	85	85	85	85	85	85	85	85	85	85
Heavy Vehicles, %	0	0	100	0	0	0	0	5	0	0	3	0
Mvmt Flow	5	0	1	2	1	2	1	556	2	1	272	4

Major/Minor	Minor2		Ν	1inor1		ſ	Major1		Ν	/lajor2			
Conflicting Flow All	837	836	274	836	837	557	276	0	0	558	0	0	
Stage 1	276	276	-	559	559	-	-	-	-	-	-	-	
Stage 2	561	560	-	277	278	-	-	-	-	-	-	-	
Critical Hdwy	7.1	6.5	7.2	7.1	6.5	6.2	4.1	-	-	4.1	-	-	
Critical Hdwy Stg 1	6.1	5.5	-	6.1	5.5	-	-	-	-	-	-	-	
Critical Hdwy Stg 2	6.1	5.5	-	6.1	5.5	-	-	-	-	-	-	-	
Follow-up Hdwy	3.5	4	4.2	3.5	4	3.3	2.2	-	-	2.2	-	-	
Pot Cap-1 Maneuver	288	305	579	289	305	534	1299	-	-	1023	-	-	
Stage 1	735	685	-	517	514	-	-	-	-	-	-	-	
Stage 2	516	514	-	734	684	-	-	-	-	-	-	-	
Platoon blocked, %								-	-		-	-	
Mov Cap-1 Maneuver	285	304	579	288	304	534	1299	-	-	1023	-	-	
Mov Cap-2 Maneuver	· 285	304	-	288	304	-	-	-	-	-	-	-	
Stage 1	734	684	-	516	513	-	-	-	-	-	-	-	
Stage 2	512	513	-	732	683	-	-	-	-	-	-	-	

Approach	EB	WB	NB	SB	
HCM Control Delay, s	16.6	15.2	0	0	
HCM LOS	С	С			

Minor Lane/Major Mvmt	NBL	NBT	NBR	EBLn1\	WBLn1	SBL	SBT	SBR
Capacity (veh/h)	1299	-	-	317	358	1023	-	-
HCM Lane V/C Ratio	0.001	-	-	0.019	0.016	0.001	-	-
HCM Control Delay (s)	7.8	0	-	16.6	15.2	8.5	-	-
HCM Lane LOS	А	А	-	С	С	А	-	-
HCM 95th %tile Q(veh)	0	-	-	0.1	0.1	0	-	-

Intersection

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		\$			÷			\$			÷	
Traffic Vol, veh/h	2	1	3	81	2	22	4	462	107	12	221	3
Future Vol, veh/h	2	1	3	81	2	22	4	462	107	12	221	3
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None									
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage,	# -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	94	94	94	94	94	94	94	94	94	94	94	94
Heavy Vehicles, %	100	0	0	2	0	5	25	4	5	17	2	0
Mvmt Flow	2	1	3	86	2	23	4	491	114	13	235	3

Major/Minor	Minor2		ľ	Minor1			Major1		N	lajor2			
Conflicting Flow All	832	876	237	821	820	548	238	0	0	605	0	0	
Stage 1	263	263	-	556	556	-	-	-	-	-	-	-	
Stage 2	569	613	-	265	264	-	-	-	-	-	-	-	
Critical Hdwy	8.1	6.5	6.2	7.12	6.5	6.25	4.35	-	-	4.27	-	-	
Critical Hdwy Stg 1	7.1	5.5	-	6.12	5.5	-	-	-	-	-	-	-	
Critical Hdwy Stg 2	7.1	5.5	-	6.12	5.5	-	-	-	-	-	-	-	
Follow-up Hdwy	4.4	4	3.3	3.518	4	3.345	2.425	-	-	2.353	-	-	
Pot Cap-1 Maneuver	200	290	807	293	312	530	1205	-	-	904	-	-	
Stage 1	570	694	-	515	516	-	-	-	-	-	-	-	
Stage 2	370	486	-	740	694	-	-	-	-	-	-	-	
Platoon blocked, %								-	-		-	-	
Mov Cap-1 Maneuver	187	284	807	286	305	530	1205	-	-	904	-	-	
Mov Cap-2 Maneuver	187	284	-	286	305	-	-	-	-	-	-	-	
Stage 1	567	682	-	512	513	-	-	-	-	-	-	-	
Stage 2	350	484	-	723	682	-	-	-	-	-	-	-	

Approach	EB	WB	NB	SB	
HCM Control Delay, s	16	22.4	0.1	0.5	
HCM LOS	С	С			

Minor Lane/Major Mvmt	NBL	NBT	NBR	EBLn1\	WBLn1	SBL	SBT	SBR
Capacity (veh/h)	1205	-	-	335	317	904	-	-
HCM Lane V/C Ratio	0.004	-	-	0.019	0.352	0.014	-	-
HCM Control Delay (s)	8	0	-	16	22.4	9	0	-
HCM Lane LOS	А	А	-	С	С	А	А	-
HCM 95th %tile Q(veh)	0	-	-	0.1	1.5	0	-	-

Queues 5: Wellington Rd & Fairview Ave

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Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
Lane Group Flow (vph)	252	327	399	349	153	172
v/c Ratio	0.42	0.16	0.36	0.33	0.29	0.11
Control Delay	8.0	5.5	17.3	1.7	23.5	0.1
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	8.0	5.5	17.3	1.7	23.5	0.1
Queue Length 50th (ft)	34	22	55	0	26	0
Queue Length 95th (ft)	73	42	97	27	46	0
Internal Link Dist (ft)		1061	854		222	
Turn Bay Length (ft)	225			235		
Base Capacity (vph)	606	2065	1104	1221	978	1583
Starvation Cap Reductn	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0
Reduced v/c Ratio	0.42	0.16	0.36	0.29	0.16	0.11
Intersection Summary						

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Movement	EBL	EBT	WBT	WBR	SBL	SBR		
Lane Configurations	ň	^	^	1	ሻሻ	1		
Traffic Volume (vph)	239	311	379	332	145	163		
Future Volume (vph)	239	311	379	332	145	163		
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900		
Total Lost time (s)	7.1	7.1	7.1	6.9	6.9	4.0		
Lane Util. Factor	1.00	0.95	0.95	1.00	0.97	1.00		
Frt	1.00	1.00	1.00	0.85	1.00	0.85		
Flt Protected	0.95	1.00	1.00	1.00	0.95	1.00		
Satd. Flow (prot)	1719	3374	3438	1553	3433	1583		
Flt Permitted	0.38	1.00	1.00	1.00	0.95	1.00		
Satd. Flow (perm)	683	3374	3438	1553	3433	1583		
Peak-hour factor, PHF	0.95	0.95	0.95	0.95	0.95	0.95		
Adj. Flow (vph)	252	327	399	349	153	172		
RTOR Reduction (vph)	0	0	0	183	0	0		
Lane Group Flow (vph)	252	327	399	166	153	172		
Heavy Vehicles (%)	5%	7%	5%	4%	2%	2%		
Turn Type	pm+pt	NA	NA	pm+ov	Prot	Free		
Protected Phases	5	2	6	. 3	3			
Permitted Phases	2			6		Free		
Actuated Green, G (s)	36.7	36.7	19.2	28.5	9.3	60.0		
Effective Green, g (s)	36.7	36.7	19.2	28.5	9.3	60.0		
Actuated g/C Ratio	0.61	0.61	0.32	0.48	0.16	1.00		
Clearance Time (s)	7.1	7.1	7.1	6.9	6.9			
Vehicle Extension (s)	3.5	4.0	4.0	3.5	3.5			
Lane Grp Cap (vph)	597	2063	1100	737	532	1583		
v/s Ratio Prot	c0.07	0.10	0.12	0.03	c0.04			
v/s Ratio Perm	c0.18			0.07		0.11		
v/c Ratio	0.42	0.16	0.36	0.22	0.29	0.11		
Uniform Delay, d1	5.9	5.0	15.7	9.3	22.4	0.0		
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00		
Incremental Delay, d2	0.6	0.2	0.9	0.2	0.4	0.1		
Delay (s)	6.4	5.2	16.6	9.4	22.8	0.1		
Level of Service	A	А	В	А	С	A		
Approach Delay (s)		5.7	13.3		10.8			
Approach LOS		А	В		В			
Intersection Summary								
HCM 2000 Control Delay			10.1	Н	CM 2000	Level of Servic	e	
HCM 2000 Volume to Capa	acity ratio		0.44		2.00			
Actuated Cycle Length (s)			60.0	S	um of lost	time (s)		
Intersection Capacity Utiliza	ation		48.0%		CU Level o	of Service		
Analysis Period (min)			15					
c Critical Lane Group								

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Movement	EBL	EBT	WBT	WBR	SBL	SBR	
Lane Configurations	5	**	**	1	ካካ	1	
Traffic Volume (veh/h)	239	311	379	332	145	163	
Future Volume (veh/h)	239	311	379	332	145	163	
Number	5	2	6	16	3	18	
Initial O (Ob), veh	0	0	0	0	0	0	
Ped-Bike Adi(A pbT)	1.00			1.00	1.00	1.00	
Parking Bus, Adi	1.00	1.00	1.00	1.00	1.00	1.00	
Adi Sat Flow, veh/h/ln	1810	1776	1810	1827	1863	1863	
Adi Flow Rate, veh/h	252	327	399	349	153	0	
Adi No. of Lanes	1	2	2	1	2	1	
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95	
Percent Heavy Veh %	5.75	7	5.75	4	2	2	
Cap. veh/h	594	2172	1242	752	423	195	
Arrive On Green	0.16	0.64	0.36	0.36	0.12	0.00	
Sat Flow, veh/h	1723	3463	3529	1553	3442	1583	
Grn Volume(v) veh/h	252	277	200	3/10	152	0	
Grn Sat Flow(s) veh/h/ln	1723	1687	1710	1553	1721	1583	
$O \operatorname{Serve}(\mathfrak{a}, \mathfrak{s}) = \mathfrak{s}$	1725	23	5.0	9.0	2.4	0.0	
Cycle O Clear(a, c) s	4.5	2.5	5.0	9.0	2.4	0.0	
Pron In Lane	1.0	2.5	5.0	1.00	1.00	1.00	
Lane Grn Can(c) veh/h	594	2172	1242	752	423	1.00	
V/C Ratio(X)	0.42	0.15	0.32	0.46	0.36	0.00	
Avail Can(c_a) veh/h	625	2172	1242	752	981	451	
HCM Platoon Ratio	1.00	1 00	1 00	1 00	1 00	1 00	
Instream Filter(I)	1.00	1.00	1.00	1.00	1.00	0.00	
Uniform Delay (d) s/yeh	7.5	4.2	13.8	10.3	24.2	0.00	
Incr Delay (d2) s/veh	0.6	4.2 0.1	0.7	2.1	0.6	0.0	
Initial \cap Delay(d3) s/veh	0.0	0.1	0.7	0.0	0.0	0.0	
%ile BackOfO(50%) veh/ln	2.2	1 1	2.5	4.2	1.2	0.0	
InGrn Delay(d) s/veh	8.1	<u> </u>	14 5	12 4	24.8	0.0	
InGrn LOS	Δ	Δ	R	12.4 R	24.0	0.0	
Approach Vol. veh/h		570	7/8	D	153		
Approach Delay s/yeh		60	13 5		24.8		
Approach LOS		0.0 A	B		2 1.0 C		
Timor	1	C	2	1	5	6	7
		2	3	4	5	6	0
Physician (C V Pa)					כ 17 0	200	0
Change Deried (V, De) e		40.7 71			17.0	20.Ŏ 7 1	14.0
Max Croop Sotting (Cmax)		7.1 20.0			10.0	10.0	0.9
Wax Green Setting (Griax), S		20.9			10.9	10.9	17.1
riviax Q Clear Time (g_C+II), S		4.3			0.5	11.0	4.4
Green Ext Time (p_c), s		J.I			0.4	0.0	0.0
Intersection Summary							
HCM 2010 Ctrl Delay			11.7				
HCM 2010 LOS			В				

4

Intersection

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		\$			\$		1	et 👘			\$	
Traffic Vol, veh/h	16	2	153	0	0	5	177	374	6	3	505	49
Future Vol, veh/h	16	2	153	0	0	5	177	374	6	3	505	49
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None									
Storage Length	-	-	-	-	-	-	280	-	-	-	-	-
Veh in Median Storage,	# -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	100	100	100	100	100	100	100	100	100	100	100	100
Heavy Vehicles, %	0	0	1	0	0	20	1	2	0	0	2	0
Mvmt Flow	16	2	153	0	0	5	177	374	6	3	505	49

Major/Minor	Minor2		ſ	Minor1			Major1			Major2			
Conflicting Flow All	1270	1270	530	1344	1291	377	554	0	0	380	0	0	
Stage 1	536	536	-	731	731	-	-	-	-	-	-	-	
Stage 2	734	734	-	613	560	-	-	-	-	-	-	-	
Critical Hdwy	7.1	6.5	6.21	7.1	6.5	6.4	4.11	-	-	4.1	-	-	
Critical Hdwy Stg 1	6.1	5.5	-	6.1	5.5	-	-	-	-	-	-	-	
Critical Hdwy Stg 2	6.1	5.5	-	6.1	5.5	-	-	-	-	-	-	-	
Follow-up Hdwy	3.5	4	3.309	3.5	4	3.48	2.209	-	-	2.2	-	-	
Pot Cap-1 Maneuver	146	170	551	130	165	632	1021	-	-	1190	-	-	
Stage 1	532	527	-	416	430	-	-	-	-	-	-	-	
Stage 2	415	429	-	483	514	-	-	-	-	-	-	-	
Platoon blocked, %								-	-		-	-	
Mov Cap-1 Maneuver	· 125	140	551	80	136	632	1021	-	-	1190	-	-	
Mov Cap-2 Maneuver	· 125	140	-	80	136	-	-	-	-	-	-	-	
Stage 1	440	525	-	344	356	-	-	-	-	-	-	-	
Stage 2	340	355	-	346	512	-	-	-	-	-	-	-	

Approach	EB	WB	NB	SB	
HCM Control Delay, s	20.1	10.7	2.9	0	
HCM LOS	С	В			

Minor Lane/Major Mvmt	NBL	NBT	NBR E	BLn1	VBLn1	SBL	SBT	SBR
Capacity (veh/h)	1021	-	-	407	632	1190	-	-
HCM Lane V/C Ratio	0.173	-	-	0.42	0.008	0.003	-	-
HCM Control Delay (s)	9.3	-	-	20.1	10.7	8	0	-
HCM Lane LOS	А	-	-	С	В	А	А	-
HCM 95th %tile Q(veh)	0.6	-	-	2	0	0	-	-

Queues	
2: Fairview Ave & Manassas PD Driveway/Signal Hill Rd	

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Lane Group	EBT	WBT	WBR	NBL	NBT	SBL	SBT
Lane Group Flow (vph)	4	42	217	1	394	275	415
v/c Ratio	0.02	0.16	0.51	0.00	0.68	0.49	0.39
Control Delay	31.8	29.3	9.9	8.0	25.7	9.6	11.3
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	31.8	29.3	9.9	8.0	25.7	9.6	11.3
Queue Length 50th (ft)	1	13	0	0	112	34	55
Queue Length 95th (ft)	13	53	63	2	282	123	271
Internal Link Dist (ft)	120	611			282		588
Turn Bay Length (ft)			110	260		260	
Base Capacity (vph)	739	705	779	721	1039	621	1119
Starvation Cap Reductn	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0
Reduced v/c Ratio	0.01	0.06	0.28	0.00	0.38	0.44	0.37
Intersection Summary							

HCM Signalized Intersection Capacity Analysis 2: Fairview Ave & Manassas PD Driveway/Signal Hill Rd

Timing Plan: EX PM

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			र्स	1	ሻ	ĥ		ሻ	ĥ	
Traffic Volume (vph)	2	2	0	40	0	206	1	340	34	261	393	1
Future Volume (vph)	2	2	0	40	0	206	1	340	34	261	393	1
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)		6.3			6.1	6.1	6.5	6.5		6.5	6.5	
Lane Util. Factor		1.00			1.00	1.00	1.00	1.00		1.00	1.00	
Frt		1.00			1.00	0.85	1.00	0.99		1.00	1.00	
Flt Protected		0.98			0.95	1.00	0.95	1.00		0.95	1.00	
Satd. Flow (prot)		1854			1752	1615	1805	1841		1770	1862	
Flt Permitted		0.98			0.95	1.00	0.52	1.00		0.31	1.00	
Satd. Flow (perm)		1854			1752	1615	987	1841		580	1862	
Peak-hour factor, PHF	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Adj. Flow (vph)	2	2	0	42	0	217	1	358	36	275	414	1
RTOR Reduction (vph)	0	0	0	0	0	189	0	3	0	0	0	0
Lane Group Flow (vph)	0	4	0	0	42	28	1	391	0	275	415	0
Heavy Vehicles (%)	0%	0%	0%	3%	0%	0%	0%	2%	0%	2%	2%	0%
Turn Type	Split	NA		Split	NA	Perm	pm+pt	NA		pm+pt	NA	
Protected Phases	4	4		3	3		5	2		1	6	
Permitted Phases						3	2			6		
Actuated Green, G (s)		1.1			9.3	9.3	26.7	25.6		43.5	35.9	
Effective Green, g (s)		1.1			9.3	9.3	26.7	25.6		43.5	35.9	
Actuated g/C Ratio		0.02			0.13	0.13	0.37	0.35		0.60	0.49	
Clearance Time (s)		6.3			6.1	6.1	6.5	6.5		6.5	6.5	
Vehicle Extension (s)		3.0			4.0	4.0	3.0	5.0		3.0	5.0	
Lane Grp Cap (vph)		28			223	206	374	647		532	918	
v/s Ratio Prot		c0.00			c0.02		0.00	c0.21		c0.08	0.22	
v/s Ratio Perm						0.02	0.00			0.23		
v/c Ratio		0.14			0.19	0.13	0.00	0.60		0.52	0.45	
Uniform Delay, d1		35.4			28.4	28.2	14.6	19.4		8.6	12.0	
Progression Factor		1.00			1.00	1.00	1.00	1.00		1.00	1.00	
Incremental Delay, d2		2.3			0.6	0.4	0.0	2.4		0.9	0.7	
Delay (s)		37.7			28.9	28.6	14.6	21.8		9.5	12.8	
Level of Service		D			С	С	В	С		А	В	
Approach Delay (s)		37.7			28.6			21.8			11.5	
Approach LOS		D			С			С			В	
Intersection Summary												
HCM 2000 Control Delay			17.9	Н	CM 2000	Level of	Service		В			
HCM 2000 Volume to Capacity	ratio		0.51									
Actuated Cycle Length (s)			72.8	S	um of los	t time (s)			25.4			
Intersection Capacity Utilization	1		57.2%	IC	U Level	of Service	Э		В			
Analysis Period (min)			15									
c Critical Lane Group												

HCM 2010 analysis expects strict NEMA phasing.

Intersection

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		\$			\$			\$			\$	
Traffic Vol, veh/h	2	0	3	1	0	0	3	369	0	1	431	7
Future Vol, veh/h	2	0	3	1	0	0	3	369	0	1	431	7
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None									
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage,	# -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	96	96	96	96	96	96	96	96	96	96	96	96
Heavy Vehicles, %	0	0	0	0	0	0	0	1	0	0	2	0
Mvmt Flow	2	0	3	1	0	0	3	384	0	1	449	7

Major/Minor	Minor2		Ν	1inor1		ſ	Major1		Ν	/lajor2			
Conflicting Flow All	845	845	453	846	848	384	456	0	0	384	0	0	
Stage 1	455	455	-	390	390	-	-	-	-	-	-	-	
Stage 2	390	390	-	456	458	-	-	-	-	-	-	-	
Critical Hdwy	7.1	6.5	6.2	7.1	6.5	6.2	4.1	-	-	4.1	-	-	
Critical Hdwy Stg 1	6.1	5.5	-	6.1	5.5	-	-	-	-	-	-	-	
Critical Hdwy Stg 2	6.1	5.5	-	6.1	5.5	-	-	-	-	-	-	-	
Follow-up Hdwy	3.5	4	3.3	3.5	4	3.3	2.2	-	-	2.2	-	-	
Pot Cap-1 Maneuver	285	302	611	284	301	668	1115	-	-	1186	-	-	
Stage 1	589	572	-	638	611	-	-	-	-	-	-	-	
Stage 2	638	611	-	588	570	-	-	-	-	-	-	-	
Platoon blocked, %								-	-		-	-	
Mov Cap-1 Maneuver	284	301	611	282	300	668	1115	-	-	1186	-	-	
Mov Cap-2 Maneuver	284	301	-	282	300	-	-	-	-	-	-	-	
Stage 1	587	571	-	636	609	-	-	-	-	-	-	-	
Stage 2	636	609	-	584	569	-	-	-	-	-	-	-	

Approach	EB	WB	NB	SB	
HCM Control Delay, s	13.7	17.8	0.1	0	
HCM LOS	В	С			

Minor Lane/Major Mvmt	NBL	NBT	NBR	EBLn1\	VBLn1	SBL	SBT	SBR
Capacity (veh/h)	1115	-	-	418	282	1186	-	-
HCM Lane V/C Ratio	0.003	-	-	0.012	0.004	0.001	-	-
HCM Control Delay (s)	8.2	0	-	13.7	17.8	8	0	-
HCM Lane LOS	А	А	-	В	С	А	А	-
HCM 95th %tile Q(veh)	0	-	-	0	0	0	-	-

Intersection

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Vol, veh/h	3	0	3	150	2	9	7	364	159	18	415	4
Future Vol, veh/h	3	0	3	150	2	9	7	364	159	18	415	4
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None									
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage, a	# -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	96	96	96	96	96	96	96	96	96	96	96	96
Heavy Vehicles, %	0	0	0	3	0	0	0	1	3	0	2	0
Mvmt Flow	3	0	3	156	2	9	7	379	166	19	432	4

Major/Minor	Minor2		ſ	Minor1		N	Major1		Ν	Major2			
Conflicting Flow All	954	1031	434	950	950	462	436	0	0	545	0	0	
Stage 1	472	472	-	476	476	-	-	-	-	-	-	-	
Stage 2	482	559	-	474	474	-	-	-	-	-	-	-	
Critical Hdwy	7.1	6.5	6.2	7.13	6.5	6.2	4.1	-	-	4.1	-	-	
Critical Hdwy Stg 1	6.1	5.5	-	6.13	5.5	-	-	-	-	-	-	-	
Critical Hdwy Stg 2	6.1	5.5	-	6.13	5.5	-	-	-	-	-	-	-	
Follow-up Hdwy	3.5	4	3.3	3.527	4	3.3	2.2	-	-	2.2	-	-	
Pot Cap-1 Maneuver	240	235	626	239	262	604	1134	-	-	1034	-	-	
Stage 1	576	562	-	568	560	-	-	-	-	-	-	-	
Stage 2	569	514	-	569	561	-	-	-	-	-	-	-	
Platoon blocked, %								-	-		-	-	
Mov Cap-1 Maneuver	229	227	626	232	253	604	1134	-	-	1034	-	-	
Mov Cap-2 Maneuver	229	227	-	232	253	-	-	-	-	-	-	-	
Stage 1	571	549	-	563	555	-	-	-	-	-	-	-	
Stage 2	553	509	-	553	548	-	-	-	-	-	-	-	

Approach	EB	WB	NB	SB	
HCM Control Delay, s	16	48.3	0.1	0.4	
HCM LOS	С	E			

Minor Lane/Major Mvmt	NBL	NBT	NBR	EBLn1V	WBLn1	SBL	SBT	SBR
Capacity (veh/h)	1134	-	-	335	241	1034	-	-
HCM Lane V/C Ratio	0.006	-	-	0.019	0.696	0.018	-	-
HCM Control Delay (s)	8.2	0	-	16	48.3	8.5	0	-
HCM Lane LOS	А	А	-	С	Ε	А	А	-
HCM 95th %tile Q(veh)	0	-	-	0.1	4.6	0.1	-	-

Queues 5: Wellington Rd & Fairview Ave

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Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
Lane Group Flow (vph)	267	542	467	277	300	288
v/c Ratio	0.43	0.24	0.32	0.24	0.51	0.18
Control Delay	8.6	6.4	16.7	1.2	30.7	0.3
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	8.6	6.4	16.7	1.2	30.7	0.3
Queue Length 50th (ft)	44	47	75	0	66	0
Queue Length 95th (ft)	93	84	124	21	94	0
Internal Link Dist (ft)		1061	854		222	
Turn Bay Length (ft)	225			235		
Base Capacity (vph)	621	2296	1443	1436	1298	1568
Starvation Cap Reductn	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0
Reduced v/c Ratio	0.43	0.24	0.32	0.19	0.23	0.18
Intersection Summary						

Movement EBL EBT WBT WBR SBL SBR Lane Configurations 1
Lane ConfigurationsImage: ConfigurationsImage: ConfigurationsTraffic Volume (vph)259526453269291279Future Volume (vph)259526453269291279Ideal Flow (vphp)190019001900190019001900Total Lost time (s)7.17.17.16.94.0Lane Util. Factor1.000.950.951.000.971.00Frt1.001.001.001.000.951.00Satd. Flow (prot)177035743539159934671568Flt Permitted0.391.001.000.951.000.951.00Satd. Flow (perm)73235743539159934671568Peak-hour factor, PHF0.970.970.970.970.970.97Adj. Flow (vph)267542467277300288RTOR Reduction (vph)00011700Lane Group Flow (vph)267542467160300288Heavy Vehicles (%)2%1%2%1%3%Turn Typepm+ptNANApm+ovProtePreeProtected Phases526FreeActuated Green, G (s)48.248.230.643.412.875.0Effective Green, G (s)48.248.230.643.412.875.0Effe
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Turn Type pm+pt NA NA pm+ov Prot Free Protected Phases 5 2 6 3 3 Permitted Phases 2 6 Free Actuated Green, G (s) 48.2 48.2 30.6 43.4 12.8 75.0 Effective Green, g (s) 48.2 48.2 30.6 43.4 12.8 75.0 Actuated g/C Ratio 0.64 0.64 0.41 0.58 0.17 1.00 Clearance Time (s) 7.1 7.1 7.1 6.9 6.9 Vehicle Extension (s) 3.5 4.0 4.0 3.5 3.5 Lane Grp Cap (vph) 615 2296 1443 925 591 1568 v/s Ratio Perm c0.22 0.07 0.18 0.18 0.17 0.18 v/c Ratio 0.43 0.24 0.32 0.17 0.51 0.18 Uniform Delay, d1 6.1 5.6 15.1 7.4 28.2 0.0
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Permitted Phases 2 6 Free Actuated Green, G (s) 48.2 48.2 30.6 43.4 12.8 75.0 Effective Green, g (s) 48.2 48.2 30.6 43.4 12.8 75.0 Actuated g/C Ratio 0.64 0.64 0.41 0.58 0.17 1.00 Clearance Time (s) 7.1 7.1 7.1 6.9 6.9 Vehicle Extension (s) 3.5 4.0 4.0 3.5 3.5 Lane Grp Cap (vph) 615 2296 1443 925 591 1568 v/s Ratio Prot c0.06 0.15 0.13 0.03 c0.09 0.18 v/c Ratio 0.43 0.24 0.32 0.17 0.51 0.18 Uniform Delay, d1 6.1 5.6 15.1 7.4 28.2 0.0 Progression Factor 1.00 1.00 1.00 1.00 1.00 1.00
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Actuated g/C Ratio 0.64 0.64 0.41 0.58 0.17 1.00 Clearance Time (s) 7.1 7.1 7.1 6.9 6.9 Vehicle Extension (s) 3.5 4.0 4.0 3.5 3.5 Lane Grp Cap (vph) 615 2296 1443 925 591 1568 v/s Ratio Prot c0.06 0.15 0.13 0.03 c0.09 0.18 v/c Ratio 0.43 0.24 0.32 0.17 0.51 0.18 Uniform Delay, d1 6.1 5.6 15.1 7.4 28.2 0.0 Progression Factor 1.00 1.00 1.00 1.00 1.00 1.00
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v/s Ratio perm c0.22 0.07 0.18 v/c Ratio 0.43 0.24 0.32 0.17 0.51 0.18 Uniform Delay, d1 6.1 5.6 15.1 7.4 28.2 0.0 Progression Factor 1.00 1.00 1.00 1.00 1.00 1.00
v/c Ratio 0.43 0.24 0.32 0.17 0.51 0.18 Uniform Delay, d1 6.1 5.6 15.1 7.4 28.2 0.0 Progression Factor 1.00 1.00 1.00 1.00 1.00 1.00
Uniform Delay, d1 6.1 5.6 15.1 7.4 28.2 0.0 Progression Factor 1.00 1.00 1.00 1.00 1.00 1.00
Progression Factor 1.00 1.00 1.00 1.00 1.00 1.00
Incremental Delay d2 0.6 0.2 0.6 0.1 0.8 0.3
Delay (s) 67 59 157 75 291 0.3
Level of Service A A B A C A
Approach Delay (s) 62 127 149
Approach LOS A B B
Intersection Summary
HCM 2000 Control Delay 10.8 HCM 2000 Level of Service R
HCM 2000 Volume to Capacity ratio 0.49
$\begin{array}{ccc} \text{Actuated Cycle Length (s)} & 75.0 & \text{Sum of lost time (c)} & 21.1 \\ \end{array}$
Intersection Canacity Itilization 52.8% ICIT Evel of Service
Analysis Period (min) 15
c Critical Lane Group

	≯	-	+	•	1	~	
Movement	EBL	EBT	WBT	WBR	SBL	SBR	
Lane Configurations	5	**	**	1	55	1	
Traffic Volume (veh/h)	259	526	453	269	291	279	
Future Volume (veh/h)	259	526	453	269	291	279	
Number	5	2	6	16	3	18	
Initial O (Ob), veh	0	0	0	0	0	0	
Ped-Bike Adi(A pbT)	1.00			1.00	1.00	1.00	
Parking Bus, Adi	1.00	1.00	1.00	1.00	1.00	1.00	
Adj Sat Flow, veh/h/ln	1863	1881	1863	1881	1881	1845	
Adj Flow Rate, veh/h	267	542	467	277	300	0	
Adj No. of Lanes	1	2	2	1	2	1	
Peak Hour Factor	0.97	0.97	0.97	0.97	0.97	0.97	
Percent Heavy Veh, %	2	1	2	1	1	3	
Cap, veh/h	601	2457	1627	937	438	198	
Arrive On Green	0.13	0.69	0.46	0.46	0.13	0.00	
Sat Flow, veh/h	1774	3668	3632	1599	3476	1568	
Grp Volume(v), veh/h	267	542	467	277	300	0	
Grp Sat Flow(s), veh/h/ln	1774	1787	1770	1599	1738	1568	
Q Serve(g_s), s	5.1	4.2	6.2	6.5	6.2	0.0	
Cycle Q Clear(q_c), s	5.1	4.2	6.2	6.5	6.2	0.0	
Prop In Lane	1.00			1.00	1.00	1.00	
Lane Grp Cap(c), veh/h	601	2457	1627	937	438	198	
V/C Ratio(X)	0.44	0.22	0.29	0.30	0.68	0.00	
Avail Cap(c_a), veh/h	623	2457	1627	937	1302	587	
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	
Upstream Filter(I)	1.00	1.00	1.00	1.00	1.00	0.00	
Uniform Delay (d), s/veh	7.1	4.3	12.6	7.8	31.3	0.0	
Incr Delay (d2), s/veh	0.6	0.2	0.4	0.8	2.3	0.0	
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	
%ile BackOfQ(50%),veh/In	2.5	2.1	3.1	3.1	3.1	0.0	
LnGrp Delay(d),s/veh	7.7	4.5	13.1	8.6	33.6	0.0	
LnGrp LOS	А	А	В	А	С		
Approach Vol, veh/h		809	744		300		
Approach Delay, s/veh		5.6	11.4		33.6		
Approach LOS		А	В		С		
Timer	1	2	3	4	5	6	7 8
Assigned Phs		2			5	6	8
Phs Duration (G+Y+Rc), s		58.6			17.1	41.6	16.4
Change Period (Y+Rc), s		7.1			7.1	7.1	6.9
Max Green Setting (Gmax), s		32.9			10.9	14.9	28.1
Max Q Clear Time (g_c+I1), s		6.2			7.1	8.5	8.2
Green Ext Time (p_c), s		5.6			0.4	2.9	1.3
Intersection Summary							
HCM 2010 Ctrl Delay			12.5				
HCM 2010 LOS			В				

F. Intersection Capacity Analysis Worksheets – 2027 Future without Development

Intersection

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4		٦	eî 👘			4	
Traffic Vol, veh/h	21	1	164	0	1	5	259	510	1	1	262	50
Future Vol, veh/h	21	1	164	0	1	5	259	510	1	1	262	50
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None									
Storage Length	-	-	-	-	-	-	280	-	-	-	-	-
Veh in Median Storage,	# -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	92	92	92	92	92	92	92	92	92	92	92	92
Heavy Vehicles, %	0	0	6	0	0	20	3	3	0	0	2	6
Mvmt Flow	23	1	178	0	1	5	282	554	1	1	285	54

Major/Minor	Minor2		ľ	Minor1			Major1		Ν	lajor2			
Conflicting Flow All	1436	1433	312	1523	1460	555	339	0	0	555	0	0	
Stage 1	314	314	-	1119	1119	-	-	-	-	-	-	-	
Stage 2	1122	1119	-	404	341	-	-	-	-	-	-	-	
Critical Hdwy	7.1	6.5	6.26	7.1	6.5	6.4	4.13	-	-	4.1	-	-	
Critical Hdwy Stg 1	6.1	5.5	-	6.1	5.5	-	-	-	-	-	-	-	
Critical Hdwy Stg 2	6.1	5.5	-	6.1	5.5	-	-	-	-	-	-	-	
Follow-up Hdwy	3.5	4	3.354	3.5	4	3.48	2.227	-	-	2.2	-	-	
Pot Cap-1 Maneuver	112	135	719	98	130	498	1215	-	-	1026	-	-	
Stage 1	701	660	-	253	285	-	-	-	-	-	-	-	
Stage 2	252	285	-	627	642	-	-	-	-	-	-	-	
Platoon blocked, %								-	-		-	-	
Mov Cap-1 Maneuver	· 90	104	719	60	100	498	1215	-	-	1026	-	-	
Mov Cap-2 Maneuver	· 90	104	-	60	100	-	-	-	-	-	-	-	
Stage 1	538	659	-	194	219	-	-	-	-	-	-	-	
Stage 2	190	219	-	470	641	-	-	-	-	-	-	-	

Approach	EB	WB	NB	SB	
HCM Control Delay, s	23.3	17.3	3	0	
HCM LOS	С	С			

Minor Lane/Major Mvmt	NBL	NBT	NBR	EBLn1V	VBLn1	SBL	SBT	SBR
Capacity (veh/h)	1215	-	-	395	299	1026	-	-
HCM Lane V/C Ratio	0.232	-	-	0.512	0.022	0.001	-	-
HCM Control Delay (s)	8.9	-	-	23.3	17.3	8.5	0	-
HCM Lane LOS	А	-	-	С	С	А	А	-
HCM 95th %tile Q(veh)	0.9	-	-	2.8	0.1	0	-	-

Queues	
2: Fairview Ave & Manassas PD Driveway/Signal Hill Rd	

	-	-	•	1	1	1	ŧ
Lane Group	EBT	WBT	WBR	NBL	NBT	SBL	SBT
Lane Group Flow (vph)	19	45	317	4	536	182	268
v/c Ratio	0.10	0.20	0.66	0.01	0.68	0.40	0.24
Control Delay	38.7	35.3	11.9	9.2	26.4	10.1	10.9
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	38.7	35.3	11.9	9.2	26.4	10.1	10.9
Queue Length 50th (ft)	7	18	0	1	158	21	32
Queue Length 95th (ft)	34	55	75	6	#494	90	176
Internal Link Dist (ft)	120	611			282		588
Turn Bay Length (ft)			110	260		260	
Base Capacity (vph)	565	560	711	809	785	522	1133
Starvation Cap Reductn	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0
Reduced v/c Ratio	0.03	0.08	0.45	0.00	0.68	0.35	0.24

Intersection Summary

95th percentile volume exceeds capacity, queue may be longer. Queue shown is maximum after two cycles. #

HCM Signalized Intersection Capacity Analysis 2: Fairview Ave & Manassas PD Driveway/Signal Hill Rd

Timing Plan: EX AM

	₫	≯	-	\mathbf{r}	•	-	•	1	1	۲	1	Ŧ
Movement	EBU	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT
Lane Configurations			4			ર્સ	1	ሻ	4Î		٦	f)
Traffic Volume (vph)	1	12	5	0	29	13	301	4	466	43	173	223
Future Volume (vph)	1	12	5	0	29	13	301	4	466	43	173	223
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)			6.3			6.1	6.1	6.5	6.5		6.5	6.5
Lane Util. Factor			1.00			1.00	1.00	1.00	1.00		1.00	1.00
Frt			1.00			1.00	0.85	1.00	0.99		1.00	0.98
Flt Protected			0.96			0.97	1.00	0.95	1.00		0.95	1.00
Satd. Flow (prot)			1832			1800	1583	1805	1797		1719	1833
Flt Permitted			0.96			0.97	1.00	0.59	1.00		0.26	1.00
Satd. Flow (perm)			1832			1800	1583	1129	1797		471	1833
Peak-hour factor, PHF	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Adj. Flow (vph)	1	13	5	0	31	14	317	4	491	45	182	235
RTOR Reduction (vph)	0	0	0	0	0	0	281	0	2	0	0	3
Lane Group Flow (vph)	0	0	19	0	0	45	36	4	534	0	182	265
Heavy Vehicles (%)	0%	0%	0%	0%	3%	0%	2%	0%	4%	9%	5%	2%
Turn Type	Split	Split	NA		Split	NA	Perm	pm+pt	NA		pm+pt	NA
Protected Phases	4	4	4		3	3		5	2		1	6
Permitted Phases							3	2			6	
Actuated Green, G (s)			2.7			10.0	10.0	41.1	39.8		56.2	48.4
Effective Green, g (s)			2.7			10.0	10.0	41.1	39.8		56.2	48.4
Actuated g/C Ratio			0.03			0.11	0.11	0.47	0.45		0.64	0.55
Clearance Time (s)			6.3			6.1	6.1	6.5	6.5		6.5	6.5
Vehicle Extension (s)			3.0			4.0	4.0	3.0	5.0		3.0	5.0
Lane Grp Cap (vph)			56			205	180	538	814		442	1010
v/s Ratio Prot			c0.01			c0.03		0.00	c0.30		c0.05	0.14
v/s Ratio Perm							0.02	0.00			0.22	
v/c Ratio			0.34			0.22	0.20	0.01	0.66		0.41	0.26
Uniform Delay, d1			41.7			35.4	35.3	12.4	18.7		9.4	10.3
Progression Factor			1.00			1.00	1.00	1.00	1.00		1.00	1.00
Incremental Delay, d2			3.6			0.7	0.8	0.0	2.6		0.6	0.3
Delay (s)			45.3			36.1	36.0	12.5	21.2		10.0	10.6
Level of Service			D			D	D	В	С		А	В
Approach Delay (s)			45.3			36.0			21.2			10.4
Approach LOS			D			D			С			В
Intersection Summary												
HCM 2000 Control Delay			21.9	Н	CM 2000	Level of	Service		С			
HCM 2000 Volume to Capacit	iy ratio		0.54									
Actuated Cycle Length (s)			87.8	S	um of los	t time (s)			25.4			
Intersection Capacity Utilization	on		68.2%	IC	CU Level	of Service)		С			
Analysis Period (min)			15									
c Critical Lane Group												

1

Movement	SBR	
Lane Configurations		
Traffic Volume (vph)	31	
Future Volume (vph)	31	
Ideal Flow (vphpl)	1900	
Total Lost time (s)		
Lane Util. Factor		
Frt		
Flt Protected		
Satd. Flow (prot)		
Flt Permitted		
Satd. Flow (perm)		
Peak-hour factor, PHF	0.95	
Adj. Flow (vph)	33	
RTOR Reduction (vph)	0	
Lane Group Flow (vph)	0	
Heavy Vehicles (%)	0%	
Turn Type		
Protected Phases		
Permitted Phases		
Actuated Green, G (s)		
Effective Green, g (s)		
Actuated g/C Ratio		
Clearance Time (s)		
Vehicle Extension (s)		
Lane Grp Cap (vph)		
v/s Ratio Prot		
v/s Ratio Perm		
v/c Ratio		
Uniform Delay, d1		
Progression Factor		
Incremental Delay, d2		
Delay (s)		
Level of Service		
Approach Delay (s)		
Approach LOS		
Intersection Summary		

HCM 2010 analysis expects strict NEMA phasing.

Intersection

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		÷			÷			ا			el el	
Traffic Vol, veh/h	4	0	1	2	1	2	1	542	2	1	259	3
Future Vol, veh/h	4	0	1	2	1	2	1	542	2	1	259	3
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage,	# -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	92	92	92	92	92	92	92	92	92	92	92	92
Heavy Vehicles, %	0	0	100	0	0	0	0	5	0	0	3	0
Mvmt Flow	4	0	1	2	1	2	1	589	2	1	282	3

Major/Minor	Minor2		Ν	1inor1		ſ	Major1		N	lajor2			
Conflicting Flow All	880	879	284	878	879	590	285	0	0	591	0	0	
Stage 1	286	286	-	592	592	-	-	-	-	-	-	-	
Stage 2	594	593	-	286	287	-	-	-	-	-	-	-	
Critical Hdwy	7.1	6.5	7.2	7.1	6.5	6.2	4.1	-	-	4.1	-	-	
Critical Hdwy Stg 1	6.1	5.5	-	6.1	5.5	-	-	-	-	-	-	-	
Critical Hdwy Stg 2	6.1	5.5	-	6.1	5.5	-	-	-	-	-	-	-	
Follow-up Hdwy	3.5	4	4.2	3.5	4	3.3	2.2	-	-	2.2	-	-	
Pot Cap-1 Maneuver	270	288	571	271	288	511	1289	-	-	995	-	-	
Stage 1	726	679	-	496	497	-	-	-	-	-	-	-	
Stage 2	495	497	-	726	678	-	-	-	-	-	-	-	
Platoon blocked, %								-	-		-	-	
Mov Cap-1 Maneuver	· 268	287	571	270	287	511	1289	-	-	995	-	-	
Mov Cap-2 Maneuver	268	287	-	270	287	-	-	-	-	-	-	-	
Stage 1	725	678	-	496	497	-	-	-	-	-	-	-	
Stage 2	491	497	-	724	677	-	-	-	-	-	-	-	

Approach	EB	WB	NB	SB	
HCM Control Delay, s	17.2	15.8	0	0	
HCM LOS	С	С			

Minor Lane/Major Mvmt	NBL	NBT	NBR	EBLn1\	VBLn1	SBL	SBT	SBR
Capacity (veh/h)	1289	-	-	300	338	995	-	-
HCM Lane V/C Ratio	0.001	-	-	0.018	0.016	0.001	-	-
HCM Control Delay (s)	7.8	0	-	17.2	15.8	8.6	-	-
HCM Lane LOS	А	А	-	С	С	А	-	-
HCM 95th %tile Q(veh)	0	-	-	0.1	0	0	-	-

Intersection

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Vol, veh/h	2	1	3	81	2	26	4	527	107	13	248	3
Future Vol, veh/h	2	1	3	81	2	26	4	527	107	13	248	3
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None									
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage,	# -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	94	94	94	94	94	94	94	94	94	94	94	94
Heavy Vehicles, %	100	0	0	2	0	5	25	4	5	17	2	0
Mvmt Flow	2	1	3	86	2	28	4	561	114	14	264	3

Major/Minor	Minor2		N	Ainor1			Major1			N	lajor2			
Conflicting Flow All	935	977	266	922	921	618	267	0	(0	675	0	0	
Stage 1	294	294	-	626	626	-	-	-		-	-	-	-	
Stage 2	641	683	-	296	295	-	-	-		-	-	-	-	
Critical Hdwy	8.1	6.5	6.2	7.12	6.5	6.25	4.35	-		-	4.27	-	-	
Critical Hdwy Stg 1	7.1	5.5	-	6.12	5.5	-	-	-		-	-	-	-	
Critical Hdwy Stg 2	7.1	5.5	-	6.12	5.5	-	-	-		-	-	-	-	
Follow-up Hdwy	4.4	4	3.3	3.518	4	3.345	2.425	-		- 2	2.353	-	-	
Pot Cap-1 Maneuver	167	253	778	251	273	484	1175	-		-	850	-	-	
Stage 1	545	673	-	472	480	-	-	-		-	-	-	-	
Stage 2	333	452	-	712	673	-	-	-		-	-	-	-	
Platoon blocked, %								-		-		-	-	
Mov Cap-1 Maneuver	· 153	247	778	244	266	484	1175	-		-	850	-	-	
Mov Cap-2 Maneuver	⁻ 153	247	-	244	266	-	-	-		-	-	-	-	
Stage 1	542	660	-	469	477	-	-	-		-	-	-	-	
Stage 2	311	449	-	694	660	-	-	-		-	-	-	-	

Approach	EB	WB	NB	SB	
HCM Control Delay, s	17.9	27	0.1	0.5	
HCM LOS	С	D			

Minor Lane/Major Mvmt	NBL	NBT	NBR	EBLn1\	VBLn1	SBL	SBT	SBR
Capacity (veh/h)	1175	-	-	286	277	850	-	-
HCM Lane V/C Ratio	0.004	-	-	0.022	0.419	0.016	-	-
HCM Control Delay (s)	8.1	0	-	17.9	27	9.3	0	-
HCM Lane LOS	А	А	-	С	D	А	А	-
HCM 95th %tile Q(veh)	0	-	-	0.1	2	0.1	-	-

Queues 5: Wellington Rd & Fairview Ave

	∕	-+	-	•	×	-		
		-						
Lane Group	EBL	EBT	WBT	WBR	SBL	SBR		
Lane Group Flow (vph)	277	344	419	392	168	184		
v/c Ratio	0.47	0.17	0.39	0.37	0.31	0.12		
Control Delay	8.9	5.8	18.0	2.5	23.2	0.1		
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0		
Total Delay	8.9	5.8	18.0	2.5	23.2	0.1		
Queue Length 50th (ft)	38	23	59	9	28	0		
Queue Length 95th (ft)	85	46	105	40	48	0		
Internal Link Dist (ft)		1061	854		222			
Turn Bay Length (ft)	225			235				
Base Capacity (vph)	596	2046	1081	1207	978	1583		
Starvation Cap Reductn	0	0	0	0	0	0		
Spillback Cap Reductn	0	0	0	0	0	0		
Storage Cap Reductn	0	0	0	0	0	0		
Reduced v/c Ratio	0.46	0.17	0.39	0.32	0.17	0.12		
Intersection Summary								
Movement EBL EBT WBT WBR SBL SBR Lane Configurations ↑		≯	-	-	•	1	<	
--	--	------	-------------	------	------	------	------	------------
Lane Configurations Image: Configurations <	Movement	EBL	EBT	WBT	WBR	SBL	SBR	
Traffic Volume (veh/h) 263 327 398 372 160 175 Future Volume (veh/h) 263 327 398 372 160 175 Number 5 2 6 16 3 18 Initial C (Qb), veh 0 0 0 0 0 0 Ped-Bike Adj(A, pbT) 1.00 1.00 1.00 1.00 1.00 1.00 Adj Sat Flow, veh/h/n 1810 177 344 419 392 168 0 Adj Ko, of Lanes 1 2 1 2 1 2 1 Peak Hour Factor 0.95 0.95 0.95 0.95 0.95 0.95 Peak Hour Factor 0.77 344 419 392 168 0 Grav, veh/h 1723 3463 3529 1553 3442 1583 Graveline (wei/whith/in 1723 1687 1719 1553 1721 1583 Graveline (wei/whith/in 1723 1687 1751 431 198 V/C Ratic(X) <	Lane Configurations	3	**	**	1	ካካ	1	
Future Volume (veh/h) 263 327 398 372 160 175 Number 5 2 6 16 3 18 Initial Q (Qb), veh 0 0 0 0 0 0 Ped-Bike Adj(A_pbT) 1.00 1.00 1.00 1.00 1.00 1.00 Parking Bus, Adj 1.00 1.00 1.00 1.00 1.00 1.00 Adj Flow Rate, veh/h 1810 1776 1810 1827 1863 1863 Adj Flow Rate, veh/h 27 344 419 392 168 0 Adj No. of Lanes 1 2 1 2 1 2 Cap, veh/h 579 2164 1231 751 431 198 Arrive On Green 0.17 0.64 0.36 0.36 0.13 0.00 Sat Flow, veh/h 1723 1687 1719 1553 1221 1583 Q Serve(g_s), s 5.1 2.4 5.3 10.5 2.7 0.0 Cycle O Clear(g_c), veh/h <	Traffic Volume (veh/h)	263	327	398	372	160	175	
Number 5 2 6 16 3 18 Initial Q (Db), veh 0 </td <td>Future Volume (veh/h)</td> <td>263</td> <td>327</td> <td>398</td> <td>372</td> <td>160</td> <td>175</td> <td></td>	Future Volume (veh/h)	263	327	398	372	160	175	
Initial Q (Qb), veh 0 0 0 0 0 0 Ped-Bike Adj(A, pbT) 1.00 1.00 1.00 1.00 1.00 1.00 Parking Bus, Adj 1.00 1.00 1.00 1.00 1.00 1.00 Adj Sat Flow, veh/h 277 344 419 392 168 0 Adj No of Lanes 1 2 2 1 2 1 Peak Hour Factor 0.95 0.95 0.95 0.95 0.95 Percent Heavy Veh, % 5 7 5 4 2 2 Cap, weh/h 579 2164 1231 751 431 198 Arrive On Green 0.17 0.64 0.36 0.36 0.13 0.00 Sat Flow, veh/h 1723 3463 3529 1553 3442 1583 Grp Volume(v), veh/h 277 344 419 392 168 0 Cycle Q Clarg(g_c), s 5.1 2.4 5.3 10.5 2.7 0.0 Prop In Lane 1.00	Number	5	2	6	16	3	18	
Ped-Bike Adj(A_pbT) 1.00 1.00 1.00 1.00 1.00 Parking Bus, Adj 1.00 1.00 1.00 1.00 1.00 1.00 Adj Sat Flow, veh/h/ln 1810 1776 1810 1827 1863 1863 Adj Flow Rate, veh/h 277 344 419 392 168 0 Adj Flow Rate, veh/h 277 344 419 392 168 0 Percent Heavy Veh, % 5 7 5 4 2 2 2 Cap, veh/h 579 2164 1231 751 431 198 419 392 1688 0 567 54 2 2 2 Cap, veh/h 579 2164 1231 751 431 198 419 392 1683 0 67 53 10.5 2.7 0.0 7 7 0 7 10.0 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1	Initial Q (Qb), veh	0	0	0	0	0	0	
Parking Bus, Adj 1.00 1.00 1.00 1.00 1.00 1.00 Adj Sat Flow, veh/h/ln 1810 1776 1810 1827 1863 1863 Adj Flow Rate, veh/h 277 344 419 392 168 0 Adj No. of Lanes 1 2 1 2 1 2 1 Peak Hour Factor 0.95 0.95 0.95 0.95 0.95 0.95 0.95 Percent Heavy Veh, % 5 7 5 4 2 2 2 Cap, veh/h 579 2164 1231 751 431 198 Arrive On Green 0.17 0.64 0.36 0.36 0.13 0.00 Sat Flow, veh/h 1723 3463 3529 1553 3442 1583 0 Grey Sat Flow(s), veh/h/ln 1723 1687 1719 1553 1721 1583 0 Sot Sat Flow (s), veh/h 1723 1687 1719 1553 10.5 2.7 0.0 Qrey escape, sot Sat Flow (s), veh/h 608 2164 1231 751 <td< td=""><td>Ped-Bike Adi(A pbT)</td><td>1.00</td><td></td><td></td><td>1.00</td><td>1.00</td><td>1.00</td><td></td></td<>	Ped-Bike Adi(A pbT)	1.00			1.00	1.00	1.00	
Adj Sat Flow, veh/h 1810 1776 1810 1827 1863 1863 Adj Kow Rate, veh/h 277 344 419 392 168 0 Adj No. of Lanes 1 2 2 1 2 1 Peak Hour Factor 0.95 0.95 0.95 0.95 0.95 Percent Heavy Veh, % 5 7 5 4 2 2 Cap, veh/h 579 2164 1231 751 431 198 Arrive On Green 0.17 0.64 0.36 0.36 0.13 0.00 Sat Flow, veh/h 1723 3463 3529 1553 3442 1583 Grp Volume(v), veh/h 277 344 419 392 168 0 Grp Volume(v), veh/h 1723 1687 1719 1553 1721 1583 Q Serve(g.s), s 5.1 2.4 5.3 10.5 2.7 0.0 Prop In Lane 1.00 1.00 1.00 1.00 1.00 1.00 Lane Grp Cap(c), veh/h 6	Parking Bus, Adi	1.00	1.00	1.00	1.00	1.00	1.00	
Adj Flow Rate, veh/h 277 344 419 392 168 0 Adj No. of Lanes 1 2 2 1 2 1 Peak Hour Factor 0.95 0.95 0.95 0.95 0.95 Percent Heavy Veh, % 5 7 5 4 2 2 Cap, veh/h 579 2164 1231 751 431 198 Arrive On Green 0.17 0.64 0.36 0.36 0.13 0.00 Sat Flow, veh/h 1723 3463 3529 1553 3442 1583 Grp Volume(v), veh/h 277 344 419 392 168 0 Grest Flow(s), veh/h 1723 1583 0 Serve(g_s), s 5.1 2.4 5.3 10.5 2.7 0.0 Cycle Q Clear(g_c, s 5.1 2.4 5.3 10.5 2.7 0.0 Cycle Q Clear(g_c), veh/h 507 2164 1231 751 431 198 V/C Ratio(X) 0.48 0.16 0.34 0.52 0.39 0.00 Avait C	Adi Sat Flow, veh/h/ln	1810	1776	1810	1827	1863	1863	
Adj No. of Lanes 1 2 2 1 2 1 Peak Hour Factor 0.95 0.95 0.95 0.95 0.95 0.95 Percent Heavy Veh, % 5 7 5 4 2 2 Cap, veh/h 579 2164 1231 751 431 198 Arrive On Green 0.17 0.64 0.36 0.13 0.00 Sat Flow, veh/h 1723 3463 3529 1553 3442 1583 Grp Volume(v), veh/h 1723 1687 1719 1553 1721 1583 Q Serve(g_s), s 5.1 2.4 5.3 10.5 2.7 0.0 Cycle Q Clear(g_c), s 5.1 2.4 5.3 10.5 2.7 0.0 Prop In Lane 1.00 1.00 1.00 1.00 1.00 1.00 1.00 V/C Ratio(X) 0.48 0.16 0.34 0.52 0.39 0.00 Avail Cap(C,a), veh/h 608 2164 1231 751 981 451 HCM Platoon Rati	Adi Flow Rate, veh/h	277	344	419	392	168	0	
Peak Hour Factor 0.95 0.95 0.95 0.95 0.95 0.95 Percent Heavy Veh, % 5 7 5 4 2 2 Cap, veh/h 579 2164 1231 751 431 198 Arrive On Green 0.17 0.64 0.36 0.13 0.00 Sat Flow, veh/h 1723 3463 3529 1553 3442 1583 Grp Volume(v), veh/h 277 344 419 392 168 0 Greg Sat Flow(s), veh/h/ln 1723 1687 1719 1553 1721 1583 Q Serve(g_s), s 5.1 2.4 5.3 10.5 2.7 0.0 Prop In Lane 1.00 <td>Adi No. of Lanes</td> <td>1</td> <td>2</td> <td>2</td> <td>1</td> <td>2</td> <td>1</td> <td></td>	Adi No. of Lanes	1	2	2	1	2	1	
Percent Heavy Veh, % 5 7 5 4 2 2 Cap, veh/h 579 2164 1231 751 431 198 Arrive On Green 0.17 0.64 0.36 0.36 0.13 0.00 Sat Flow, veh/h 1723 3463 3529 1553 3442 1583 Grp Volume(v), veh/h 277 344 419 392 168 0 Grp Sat Flow(s), veh/h/ln 1723 1687 1719 1553 1721 1583 Q Serve(g_s), s 5.1 2.4 5.3 10.5 2.7 0.0 Cycle Q Clear(g_c), s 5.1 2.4 5.3 10.5 2.7 0.0 Prop In Lane 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 Lane Grp Cap(c), veh/h 579 2164 1231 751 431 198 V/C Ratio(X) 0.48 0.16 0.34 0.52 0.39 0.00 Avail Cap(c_a), veh/h 608 2164 1231 751 431 451	Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95	
Cap, veh/h 579 2164 1231 751 431 198 Arrive On Green 0.17 0.64 0.36 0.36 0.13 0.00 Sat Flow, veh/h 1723 3463 3529 1553 3442 1583 Grp Volume(v), veh/h 177 344 419 392 168 0 Grp Sat Flow(s), veh/h/ln 1723 1687 1719 1553 1721 1583 O Serve(g.s), s 5.1 2.4 5.3 10.5 2.7 0.0 Cycle Q Clear(g_c), s 5.1 2.4 5.3 10.5 2.7 0.0 Prop In Lane 1.00 1.00 1.00 1.00 1.00 1.00 Avail Cap(c_a), veh/h 608 2164 1231 751 431 198 V/C Ratio(X) 0.48 0.16 0.34 0.52 0.39 0.00 Avail Cap(C_a), veh/h 608 2164 1231 751 981 451 HCM Platoon Ratio 1.00 1.00 1.00 1.00 1.00 1.00	Percent Heavy Veh. %	5	7	5	4	2	2	
Arrive On Green 0.17 0.64 0.36 0.13 0.00 Sat Flow, veh/h 1723 3463 3529 1553 3442 1583 Grp Volume(v), veh/h 277 344 419 392 168 0 Grp Sat Flow(s), veh/h 1723 1687 1719 1553 3442 1583 Q Serve(g_s), s 5.1 2.4 5.3 10.5 2.7 0.0 Cycle Q Clear(g_c), s 5.1 2.4 5.3 10.5 2.7 0.0 Prop In Lane 1.00 1.00 1.00 1.00 1.00 1.00 Lane Grp Cap(c), veh/h 579 2164 1231 751 431 198 V/C Ratio(X) 0.48 0.16 0.34 0.52 0.39 0.00 Avail Cap(c_a), veh/h 608 2164 1231 751 981 451 HCM Platoon Ratio 1.00 1.00 1.00 1.00 1.00 1.00 Upstream Filter(I) 1.00 1.00 1.00 1.00 0.0 0.0	Cap, veh/h	579	2164	1231	751	431	198	
Sat Flow, veh/h 1723 3463 3529 1533 3442 1583 Grp Volume(v), veh/h 277 344 419 392 168 0 Grp Sat Flow(s), veh/h/ln 1723 1687 1719 1553 31721 1583 Q Serve(g, s), s 5.1 2.4 5.3 10.5 2.7 0.0 Cycle Q Clear(g_c), s 5.1 2.4 5.3 10.5 2.7 0.0 Prop In Lane 1.00 1.00 1.00 1.00 1.00 1.00 Lane Grp Cap(c), veh/h 579 2164 1231 751 981 451 HCM Platoon Ratio 1.00 1.00 1.00 1.00 1.00 1.00 Upstream Filter(I) 1.00 1.00 1.00 1.00 1.00 1.00 Indr Delay (d2), s/veh 0.7 0.2 0.8 2.6 0.7 0.0 Initial Q Delay(d3), s/veh 0.7 0.2 0.8 2.6 0.7 0.0 Incr Delay (d2), s/veh 8.6 4.5 14.8 13.3 24.8	Arrive On Green	0.17	0.64	0.36	0.36	0.13	0.00	
Grp Volume(v), veh/h 277 344 419 392 168 0 Grp Sat Flow(s), veh/h/ln 1723 1687 1719 1553 1721 1583 Q Serve(g_s), s 5.1 2.4 5.3 10.5 2.7 0.0 Cycle Q Clear(g_c), s 5.1 2.4 5.3 10.5 2.7 0.0 Prop In Lane 1.00 1.00 1.00 1.00 1.00 1.00 Lane Grp Cap(c), veh/h 579 2164 1231 751 431 198 V/C Ratio(X) 0.48 0.16 0.34 0.52 0.39 0.00 Avail Cap(c_a), veh/h 608 2164 1231 751 981 451 HCM Platoon Ratio 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 Upstream Filter(I) 1.00 1.00 1.00 1.00 0.00 0.0 0.0 Infor Delay (d2), s/veh 0.7 0.2 0.8 2.6 0.7 0.0 Incr Delay (d2), s/veh 0.7 0.2 0	Sat Flow, veh/h	1723	3463	3529	1553	3442	1583	
Intervention Intervention Intervention Intervention Intervention Grp Sat Flow(s), veh/h/in 1723 1687 1719 1553 1721 1583 Q Serve(g_s), s 5.1 2.4 5.3 10.5 2.7 0.0 Cycle Q Clear(g_c), s 5.1 2.4 5.3 10.5 2.7 0.0 Prop In Lane 1.00 1.00 1.00 1.00 1.00 1.00 Lane Grp Cap(c), veh/h 579 2164 1231 751 431 198 V/C Ratio(X) 0.48 0.16 0.34 0.52 0.39 0.00 Avail Cap(c_a), veh/h 608 2164 1231 751 981 451 HCM Platoon Ratio 1.00 1.00 1.00 1.00 1.00 1.00 Upstream Filter(I) 1.00 1.00 1.00 1.00 1.00 1.00 Inftia Q Delay(d), s/veh 7.8 4.3 14.1 10.7 24.1 0.0 Incr Delay (d/2), s/veh 0.6 4.5 14.8 13.3 24.8 0.	Grp Volume(v), veh/h	277	344	419	392	168	0	
C Serve(g_s), s 5.1 2.4 5.3 10.5 2.7 0.0 Cycle Q Clear(g_c), s 5.1 2.4 5.3 10.5 2.7 0.0 Prop In Lane 1.00 1.00 1.00 1.00 1.00 1.00 Lane Grp Cap(c), veh/h 579 2164 1231 751 431 198 V/C Ratio(X) 0.48 0.16 0.34 0.52 0.39 0.00 Avail Cap(c_a), veh/h 608 2164 1231 751 981 451 HCM Platoon Ratio 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 Upstream Filter(I) 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 Incr Delay (d2), s/veh 0.7 0.2 0.8 2.6 0.7 0.0 Incr Delay (d2), s/veh 0.7 0.2 0.8 2.6 0.7 0.0 Incr Delay (d2), s/veh 0.7 0.2 0.8 2.6 0.7 0.0 LnGrp Delay(d3), s/veh 0.0 0.0<	Grp Sat Flow(s) veh/h/ln	1723	1687	1719	1553	1721	1583	
Drive Drive <th< td=""><td>O Serve(a, s), s</td><td>5.1</td><td>2.4</td><td>5.3</td><td>10.5</td><td>2.7</td><td>0.0</td><td></td></th<>	O Serve(a, s), s	5.1	2.4	5.3	10.5	2.7	0.0	
Line	Cycle O Clear(g_c), s	5.1	2.4	5.3	10.5	2.7	0.0	
Lane Grp Cap(c), veh/h 579 2164 1231 751 431 198 V/C Ratio(X) 0.48 0.16 0.34 0.52 0.39 0.00 Avail Cap(c_a), veh/h 608 2164 1231 751 981 451 HCM Platoon Ratio 1.00 1.00 1.00 1.00 1.00 1.00 Upstream Filter(I) 1.00 1.00 1.00 1.00 0.00 0.00 Uniform Delay (d), s/veh 7.8 4.3 14.1 10.7 24.1 0.0 Inct Delay (d2), s/veh 0.7 0.2 0.8 2.6 0.7 0.0 India Q Delay(d3), s/veh 0.4 0.0 0.0 0.0 0.0 0.0 Mile BackOfQ(50%), veh/ln 2.4 1.1 2.6 5.0 1.3 0.0 LnGrp Delay(d), s/veh 8.6 4.5 14.8 13.3 24.8 0.0 LnGrp LOS A B B C C Timer 1 2 3 4 5 6 7 8 Appro	Prop In Lane	1.00		5.6	1.00	1.00	1.00	
V/C Ratio(X) 0.48 0.16 0.34 0.52 0.39 0.00 Avail Cap(c_a), veh/h 608 2164 1231 751 981 451 HCM Platon Ratio 1.00 1.00 1.00 1.00 1.00 1.00 Upstream Filter(I) 1.00 1.00 1.00 1.00 1.00 0.00 Uniform Delay (d), s/veh 7.8 4.3 14.1 10.7 24.1 0.0 Incr Delay (d2), s/veh 0.7 0.2 0.8 2.6 0.7 0.0 Initial Q Delay(d3), s/veh 0.0 0.0 0.0 0.0 0.0 0.0 InGrp Delay(d), s/veh 8.6 4.5 14.8 13.3 24.8 0.0 LnGrp Delay(d), s/veh 8.6 4.5 14.8 13.3 24.8 0.0 LnGrp LOS A A B C	Lane Grp Cap(c), veh/h	579	2164	1231	751	431	198	
Avail Cap(c_a), veh/h 608 2164 1231 751 981 451 HCM Platoon Ratio 1.00 1.00 1.00 1.00 1.00 1.00 Upstream Filter(I) 1.00 1.00 1.00 1.00 1.00 0.00 Uniform Delay (d), s/veh 7.8 4.3 14.1 10.7 24.1 0.0 Incr Delay (d2), s/veh 0.7 0.2 0.8 2.6 0.7 0.0 Initial Q Delay(d3),s/veh 0.0 0.0 0.0 0.0 0.0 0.0 InGrp Delay(d),s/veh 8.6 4.5 14.8 13.3 24.8 0.0 LnGrp Delay(d),s/veh 8.6 4.5 14.8 13.3 24.8 0.0 LnGrp LOS A A B C Approach Vol, veh/h 6.3 14.1 24.8 Approach LOS A B C C Timer 1 2 3 4 5 6 7 8 Assigned Phs 2 5 6 8 P 1.0 1.4 2 <td>V/C Ratio(X)</td> <td>0.48</td> <td>0.16</td> <td>0.34</td> <td>0.52</td> <td>0.39</td> <td>0.00</td> <td></td>	V/C Ratio(X)	0.48	0.16	0.34	0.52	0.39	0.00	
HCM Platoon Ratio 1.00 1.00 1.00 1.00 1.00 1.00 Upstream Filter(I) 1.00 1.00 1.00 1.00 1.00 0.00 Uniform Delay (d), s/veh 7.8 4.3 14.1 10.7 24.1 0.0 Incr Delay (d2), s/veh 0.7 0.2 0.8 2.6 0.7 0.0 Initial Q Delay(d3), s/veh 0.0 0.0 0.0 0.0 0.0 0.0 Sile BackOfQ(50%), veh/ln 2.4 1.1 2.6 5.0 1.3 0.0 LnGrp Delay(d), s/veh 8.6 4.5 14.8 13.3 24.8 0.0 LnGrp LOS A A B C Approach Vol, veh/h 621 811 168 Approach LOS A B C C Timer 1 2 3 4 5 6 7 8 Assigned Phs 2 5 6 8 Phs Duration (G+Y+Rc), s 7.1 7.1 7.1 6.9 Max Green Setting (Gmax), s 28.9 10.9 10.9	Avail Cap(c_a), veh/h	608	2164	1231	751	981	451	
Upstream Filter(I) 1.00 1.00 1.00 1.00 1.00 0.00 Uniform Delay (d), s/veh 7.8 4.3 14.1 10.7 24.1 0.0 Incr Delay (d2), s/veh 0.7 0.2 0.8 2.6 0.7 0.0 Initial Q Delay(d3), s/veh 0.0 0.0 0.0 0.0 0.0 0.0 %ile BackOfQ(50%), veh/ln 2.4 1.1 2.6 5.0 1.3 0.0 LnGrp Delay(d), s/veh 8.6 4.5 14.8 13.3 24.8 0.0 LnGrp LOS A A B C	HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	
Uniform Delay (d), s/veh 7.8 4.3 14.1 10.7 24.1 0.0 Incr Delay (d2), s/veh 0.7 0.2 0.8 2.6 0.7 0.0 Initial Q Delay(d3), s/veh 0.0 0.0 0.0 0.0 0.0 0.0 %ile BackOfQ(50%), veh/ln 2.4 1.1 2.6 5.0 1.3 0.0 LnGrp Delay(d), s/veh 8.6 4.5 14.8 13.3 24.8 0.0 LnGrp Delay(d), s/veh 8.6 4.5 14.8 13.3 24.8 0.0 LnGrp DOS A A B C	Upstream Filter(I)	1.00	1.00	1.00	1.00	1.00	0.00	
Incr Delay (d2), s/veh 0.7 0.2 0.8 2.6 0.7 0.0 Initial Q Delay(d3), s/veh 0.0 0.0 0.0 0.0 0.0 0.0 %ile BackOfQ(50%), veh/ln 2.4 1.1 2.6 5.0 1.3 0.0 LnGrp Delay(d), s/veh 8.6 4.5 14.8 13.3 24.8 0.0 LnGrp Delay(d), s/veh 8.6 4.5 14.8 13.3 24.8 0.0 LnGrp DOS A A B B C 7 7 Approach Vol, veh/h 6.3 14.1 24.8 7 8 Approach DoS A B C 7 8 Assigned Phs 2 5 6 8 Phs Duration (G+Y+Rc), s 7.1 7.1 7.1 6.9 Max Green Setting (Gmax), s 28.9 10.9 10.9 17.1 Max Q Clear Time (p_c, s 3.3 0.4 0.0 0.5 Intersection Summary 12.2 4 4 4 4 HCM 2010 Ctrl Delay 12	Uniform Delay (d), s/veh	7.8	4.3	14.1	10.7	24.1	0.0	
Initial Q Delay(d3),s/veh 0.0 0.0 0.0 0.0 0.0 %ile BackOfQ(50%),veh/ln 2.4 1.1 2.6 5.0 1.3 0.0 LnGrp Delay(d),s/veh 8.6 4.5 14.8 13.3 24.8 0.0 LnGrp Delay(d),s/veh 8.6 4.5 14.8 13.3 24.8 0.0 LnGrp LOS A A B B C 7 Approach Vol, veh/h 6.3 14.1 24.8 7 7 Approach Delay, s/veh 6.3 14.1 24.8 7 8 Approach LOS A B C 7 8 Assigned Phs 2 5 6 8 Phs Duration (G+Y+Rc), s 7.1 7.1 7.1 6.9 Max Green Setting (Gmax), s 28.9 10.9 10.9 17.1 Max Q Clear Time (g_c+I1), s 4.4 7.1 12.5 4.7 Green Ext Time (p_c), s 3.3 0.4 0.0 0.5 Intersection Summary 12.2 9 10.9 10.5	Incr Delay (d2), s/veh	0.7	0.2	0.8	2.6	0.7	0.0	
%ile BackOfQ(50%),veh/ln 2.4 1.1 2.6 5.0 1.3 0.0 LnGrp Delay(d),s/veh 8.6 4.5 14.8 13.3 24.8 0.0 LnGrp LOS A A B B C Approach Vol, veh/h 621 811 168 Approach Delay, s/veh 6.3 14.1 24.8 Approach LOS A B C Timer 1 2 3 4 5 6 7 8 Assigned Phs 2 5 6 8 Phs Duration (G+Y+Rc), s 45.6 17.0 28.6 14.4 Change Period (Y+Rc), s 7.1 7.1 7.1 6.9 Max Green Setting (Gmax), s 28.9 10.9 10.9 17.1 Max Q Clear Time (g_c+11), s 4.4 7.1 12.5 4.7 Green Ext Time (p_c), s 3.3 0.4 0.0 0.5 Intersection Summary 12.2 4.4 4.4 4.4 4.4 HCM 2010 Ctrl Delay 12.2 4.2 4.4 4.4	Initial Q Delay(d3), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	
LnGrp Delay(d),s/veh 8.6 4.5 14.8 13.3 24.8 0.0 LnGrp LOS A A B B C Approach Vol, veh/h 621 811 168 Approach Delay, s/veh 6.3 14.1 24.8 Approach LOS A B C Timer 1 2 3 4 5 6 7 8 Assigned Phs 2 3 4 5 6 7 8 Assigned Phs 2 5 6 8 Phs Duration (G+Y+Rc), s 45.6 17.0 28.6 14.4 Change Period (Y+Rc), s 7.1 7.1 7.1 6.9 Max Green Setting (Gmax), s 28.9 10.9 10.9 17.1 Max Q Clear Time (g_c+I1), s 4.4 7.1 12.5 4.7 Green Ext Time (p_c), s 3.3 0.4 0.0 0.5 Intersection Summary HCM 2010 Ctrl Delay 12.2 12.2 12.2 12.2 12.2	%ile BackOfQ(50%),veh/ln	2.4	1.1	2.6	5.0	1.3	0.0	
LnGrp LOS A A B B C Approach Vol, veh/h 621 811 168 Approach Delay, s/veh 6.3 14.1 24.8 Approach LOS A B C Timer 1 2 3 4 5 6 7 8 Assigned Phs 2 5 6 8 Phs Duration (G+Y+Rc), s 45.6 17.0 28.6 14.4 Change Period (Y+Rc), s 7.1 7.1 7.1 6.9 Max Green Setting (Gmax), s 28.9 10.9 10.9 17.1 Max Q Clear Time (g_c+11), s 4.4 7.1 12.5 4.7 Green Ext Time (p_c), s 3.3 0.4 0.0 0.5 Intersection Summary 12.2 4 4 4 4	LnGrp Delay(d).s/veh	8.6	4.5	14.8	13.3	24.8	0.0	
Approach Vol, veh/h 621 811 168 Approach Delay, s/veh 6.3 14.1 24.8 Approach LOS A B C Timer 1 2 3 4 5 6 7 8 Assigned Phs 2 3 4 5 6 7 8 Assigned Phs 2 5 6 8 Phs Duration (G+Y+Rc), s 45.6 17.0 28.6 14.4 Change Period (Y+Rc), s 7.1 7.1 7.1 6.9 Max Green Setting (Gmax), s 28.9 10.9 10.9 17.1 Max Q Clear Time (g_c+I1), s 4.4 7.1 12.5 4.7 Green Ext Time (p_c), s 3.3 0.4 0.0 0.5 Intersection Summary HCM 2010 Ctrl Delay 12.2 12.2 12.2 12.2 12.2 12.2	LnGrp LOS	A	A	В	В	C	5.0	
Approach Delay, s/veh 6.3 14.1 24.8 Approach LOS A B C Timer 1 2 3 4 5 6 7 8 Assigned Phs 2 5 6 8 Phs Duration (G+Y+Rc), s 45.6 17.0 28.6 14.4 Change Period (Y+Rc), s 7.1 7.1 7.1 6.9 Max Green Setting (Gmax), s 28.9 10.9 10.9 17.1 Max Q Clear Time (g_c+I1), s 4.4 7.1 12.5 4.7 Green Ext Time (p_c), s 3.3 0.4 0.0 0.5 Intersection Summary 12.2 4 4 4	Approach Vol. veh/h		621	811		168		
Approach LOS A B C Timer 1 2 3 4 5 6 7 8 Assigned Phs 2 3 4 5 6 7 8 Assigned Phs 2 5 6 8 Physical Control Contro Control Control Control Control Contro Control Contro C	Approach Delay, s/veh		6.3	14.1		24.8		
Timer 1 2 3 4 5 6 7 8 Assigned Phs 2 5 6 8 Phs Duration (G+Y+Rc), s 45.6 17.0 28.6 14.4 Change Period (Y+Rc), s 7.1 7.1 7.1 6.9 Max Green Setting (Gmax), s 28.9 10.9 10.9 17.1 Max Q Clear Time (g_c+11), s 4.4 7.1 12.5 4.7 Green Ext Time (p_c), s 3.3 0.4 0.0 0.5 Intersection Summary 12.2 12.2 12.2	Approach LOS		A	В		C		
Assigned Phs 2 5 6 8 Phs Duration (G+Y+Rc), s 45.6 17.0 28.6 14.4 Change Period (Y+Rc), s 7.1 7.1 7.1 6.9 Max Green Setting (Gmax), s 28.9 10.9 10.9 17.1 Max Q Clear Time (g_c+I1), s 4.4 7.1 12.5 4.7 Green Ext Time (p_c), s 3.3 0.4 0.0 0.5 Intersection Summary 12.2 12.2 12.2	Timer	1	2	3	4	5	6	7 8
Phs Duration (G+Y+Rc), s 45.6 17.0 28.6 14.4 Change Period (Y+Rc), s 7.1 7.1 7.1 6.9 Max Green Setting (Gmax), s 28.9 10.9 10.9 17.1 Max Q Clear Time (g_c+I1), s 4.4 7.1 12.5 4.7 Green Ext Time (p_c), s 3.3 0.4 0.0 0.5 Intersection Summary 12.2 12.2 12.2	Assigned Phs		2	5	т	5	6	ρ
Change Period (Y+Rc), s 7.1 7.1 7.1 6.9 Max Green Setting (Gmax), s 28.9 10.9 10.9 17.1 Max Q Clear Time (g_c+I1), s 4.4 7.1 12.5 4.7 Green Ext Time (p_c), s 3.3 0.4 0.0 0.5 Intersection Summary 12.2 4.2 4.2 4.2	Phy Duration $(G_{\pm}V_{\pm}R_{C})$ s		<u>45</u> 6			17.0	28.6	1/ /
Max Green Setting (Gmax), s 28.9 10.9 10.9 17.1 Max Q Clear Time (g_c+I1), s 4.4 7.1 12.5 4.7 Green Ext Time (p_c), s 3.3 0.4 0.0 0.5 Intersection Summary 12.2 HCM 2010 Ctrl Delay 12.2	Change Period ($V_{\pm}P_{C}$) s		4J.0 7 1			7.0	20.0	6.0
Max Q Clear Time (g_c+11), s 4.4 7.1 12.5 4.7 Green Ext Time (p_c), s 3.3 0.4 0.0 0.5 Intersection Summary 12.2 12.2 12.2	Max Green Setting (Gmax) s		28.9			10.0	10.9	17 1
Intersection Summary 12.2 HCM 2010 Ctrl Delay 12.2	Max O Clear Time $(a, c+11)$		20.7 A A			71	12.5	17.1
Intersection Summary HCM 2010 Ctrl Delay 12.2 HCM 2010 L OS	Green Ext Time (n c) s		4.4			0.1	0.0	4.7 0 5
HCM 2010 Ctrl Delay 12.2			J.J			0.4	0.0	0.0
HCM 2010 CILI DEIAY I2.2	Intersection Summary			10.0				
	HUM 2010 UN Delay			12.2				

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Intersection

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		\$			\$		1	et 👘			\$	
Traffic Vol, veh/h	16	2	154	0	0	5	179	412	6	3	537	49
Future Vol, veh/h	16	2	154	0	0	5	179	412	6	3	537	49
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None									
Storage Length	-	-	-	-	-	-	280	-	-	-	-	-
Veh in Median Storage,	# -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	100	100	100	100	100	100	100	100	100	100	100	100
Heavy Vehicles, %	0	0	1	0	0	20	1	2	0	0	2	0
Mvmt Flow	16	2	154	0	0	5	179	412	6	3	537	49

Major/Minor	Minor2		Ν	Ainor1			Major1			N	lajor2			
Conflicting Flow All	1344	1344	562	1419	1365	415	586	0	(0	418	0	0	
Stage 1	568	568	-	773	773	-	-	-		-	-	-	-	
Stage 2	776	776	-	646	592	-	-	-		-	-	-	-	
Critical Hdwy	7.1	6.5	6.21	7.1	6.5	6.4	4.11	-		-	4.1	-	-	
Critical Hdwy Stg 1	6.1	5.5	-	6.1	5.5	-	-	-		-	-	-	-	
Critical Hdwy Stg 2	6.1	5.5	-	6.1	5.5	-	-	-		-	-	-	-	
Follow-up Hdwy	3.5	4	3.309	3.5	4	3.48	2.209	-		-	2.2	-	-	
Pot Cap-1 Maneuver	130	153	528	115	149	601	994	-		-	1152	-	-	
Stage 1	511	510	-	395	412	-	-	-		-	-	-	-	
Stage 2	393	410	-	464	497	-	-	-		-	-	-	-	
Platoon blocked, %								-		-		-	-	
Mov Cap-1 Maneuver	· 111	125	528	69	122	601	994	-		-	1152	-	-	
Mov Cap-2 Maneuver	· 111	125	-	69	122	-	-	-		-	-	-	-	
Stage 1	419	508	-	324	338	-	-	-		-	-	-	-	
Stage 2	320	336	-	326	495	-	-	-		-	-	-	-	

Approach	EB	WB	NB	SB	
HCM Control Delay, s	22	11	2.8	0	
HCM LOS	С	В			

Minor Lane/Major Mvmt	NBL	NBT	NBR	EBLn1\	VBLn1	SBL	SBT	SBR	
Capacity (veh/h)	994	-	-	381	601	1152	-	-	
HCM Lane V/C Ratio	0.18	-	-	0.451	0.008	0.003	-	-	
HCM Control Delay (s)	9.4	-	-	22	11	8.1	0	-	
HCM Lane LOS	А	-	-	С	В	А	А	-	
HCM 95th %tile Q(veh)	0.7	-	-	2.3	0	0	-	-	

Queues	
2: Fairview Ave & Manassas PD Driveway/Signal Hill Rd	

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Long Crown	ГОТ					CDI	• CDT
Lane Group	EDI	VVDI	WDK	INDL	IND I	SBL	SRI
Lane Group Flow (vph)	22	44	217	2	423	275	449
v/c Ratio	0.10	0.18	0.53	0.00	0.71	0.50	0.42
Control Delay	36.2	33.9	10.8	9.0	28.7	11.1	12.9
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	36.2	33.9	10.8	9.0	28.7	11.1	12.9
Queue Length 50th (ft)	7	14	0	0	127	34	61
Queue Length 95th (ft)	36	56	63	3	309	123	300
Internal Link Dist (ft)	120	611			282		588
Turn Bay Length (ft)			110	260		260	
Base Capacity (vph)	665	643	727	681	944	577	1084
Starvation Cap Reductn	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0
Reduced v/c Ratio	0.03	0.07	0.30	0.00	0.45	0.48	0.41
Intersection Summary							

HCM Signalized Intersection Capacity Analysis 2: Fairview Ave & Manassas PD Driveway/Signal Hill Rd

Timing Plan: EX PM

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			र्स	1	5	ĥ		ሻ	ţ,	
Traffic Volume (vph)	14	7	0	40	2	206	2	368	34	261	422	5
Future Volume (vph)	14	7	0	40	2	206	2	368	34	261	422	5
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)		6.3			6.1	6.1	6.5	6.5		6.5	6.5	
Lane Util. Factor		1.00			1.00	1.00	1.00	1.00		1.00	1.00	
Frt		1.00			1.00	0.85	1.00	0.99		1.00	1.00	
Flt Protected		0.97			0.95	1.00	0.95	1.00		0.95	1.00	
Satd. Flow (prot)		1837			1763	1615	1805	1842		1770	1860	
Flt Permitted		0.97			0.95	1.00	0.50	1.00		0.29	1.00	
Satd. Flow (perm)		1837			1763	1615	956	1842		537	1860	
Peak-hour factor, PHF	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Adj. Flow (vph)	15	7	0	42	2	217	2	387	36	275	444	5
RTOR Reduction (vph)	0	0	0	0	0	190	0	3	0	0	0	0
Lane Group Flow (vph)	0	22	0	0	44	27	2	420	0	275	449	0
Heavy Vehicles (%)	0%	0%	0%	3%	0%	0%	0%	2%	0%	2%	2%	0%
Turn Type	Split	NA		Split	NA	Perm	pm+pt	NA		pm+pt	NA	
Protected Phases	. 4	4		3	3		5	2		1	6	
Permitted Phases						3	2			6		
Actuated Green, G (s)		2.5			9.6	9.6	29.6	28.4		47.5	39.8	
Effective Green, g (s)		2.5			9.6	9.6	29.6	28.4		47.5	39.8	
Actuated g/C Ratio		0.03			0.12	0.12	0.38	0.36		0.61	0.51	
Clearance Time (s)		6.3			6.1	6.1	6.5	6.5		6.5	6.5	
Vehicle Extension (s)		3.0			4.0	4.0	3.0	5.0		3.0	5.0	
Lane Grp Cap (vph)		58			215	197	373	666		522	943	
v/s Ratio Prot		c0.01			c0.02		0.00	c0.23		c0.08	0.24	
v/s Ratio Perm						0.02	0.00			0.23		
v/c Ratio		0.38			0.20	0.13	0.01	0.63		0.53	0.48	
Uniform Delay, d1		37.2			31.0	30.7	15.2	20.7		9.4	12.6	
Progression Factor		1.00			1.00	1.00	1.00	1.00		1.00	1.00	
Incremental Delay, d2		4.1			0.6	0.4	0.0	2.7		1.0	0.8	
Delay (s)		41.4			31.7	31.2	15.3	23.4		10.3	13.4	
Level of Service		D			С	С	В	С		В	В	
Approach Delay (s)		41.4			31.3			23.4			12.2	
Approach LOS		D			С			С			В	
Intersection Summary												
HCM 2000 Control Delay			19.4	Н	CM 2000	Level of	Service		В			
HCM 2000 Volume to Capacity	ratio		0.53									
Actuated Cycle Length (s)			78.5	S	um of los	t time (s)			25.4			
Intersection Capacity Utilization	1		59.8%	IC	U Level	of Service	Э		В			
Analysis Period (min)			15									
c Critical Lane Group												

HCM 2010 analysis expects strict NEMA phasing.

Intersection

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		\$			÷			÷			÷	
Traffic Vol, veh/h	2	0	3	1	0	0	3	401	0	1	478	7
Future Vol, veh/h	2	0	3	1	0	0	3	401	0	1	478	7
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None									
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage,	# -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	96	96	96	96	96	96	96	96	96	96	96	96
Heavy Vehicles, %	0	0	0	0	0	0	0	1	0	0	2	0
Mvmt Flow	2	0	3	1	0	0	3	418	0	1	498	7

Major/Minor	Minor2		Ν	1inor1		ſ	Major1		Ν	/lajor2			
Conflicting Flow All	928	928	502	929	931	418	505	0	0	418	0	0	
Stage 1	504	504	-	424	424	-	-	-	-	-	-	-	
Stage 2	424	424	-	505	507	-	-	-	-	-	-	-	
Critical Hdwy	7.1	6.5	6.2	7.1	6.5	6.2	4.1	-	-	4.1	-	-	
Critical Hdwy Stg 1	6.1	5.5	-	6.1	5.5	-	-	-	-	-	-	-	
Critical Hdwy Stg 2	6.1	5.5	-	6.1	5.5	-	-	-	-	-	-	-	
Follow-up Hdwy	3.5	4	3.3	3.5	4	3.3	2.2	-	-	2.2	-	-	
Pot Cap-1 Maneuver	250	270	573	250	269	639	1070	-	-	1152	-	-	
Stage 1	554	544	-	612	590	-	-	-	-	-	-	-	
Stage 2	612	590	-	553	543	-	-	-	-	-	-	-	
Platoon blocked, %								-	-		-	-	
Mov Cap-1 Maneuver	· 249	269	573	248	268	639	1070	-	-	1152	-	-	
Mov Cap-2 Maneuver	· 249	269	-	248	268	-	-	-	-	-	-	-	
Stage 1	552	543	-	610	588	-	-	-	-	-	-	-	
Stage 2	610	588	-	549	542	-	-	-	-	-	-	-	

Approach	EB	WB	NB	SB	
HCM Control Delay, s	14.7	19.6	0.1	0	
HCM LOS	В	С			

Minor Lane/Major Mvmt	NBL	NBT	NBR	EBLn1\	NBLn1	SBL	SBT	SBR
Capacity (veh/h)	1070	-	-	377	248	1152	-	-
HCM Lane V/C Ratio	0.003	-	-	0.014	0.004	0.001	-	-
HCM Control Delay (s)	8.4	0	-	14.7	19.6	8.1	0	-
HCM Lane LOS	А	А	-	В	С	А	А	-
HCM 95th %tile Q(veh)	0	-	-	0	0	0	-	-

Intersection

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		\$			\$			\$			\$	
Traffic Vol, veh/h	3	0	3	150	2	9	7	396	159	20	460	4
Future Vol, veh/h	3	0	3	150	2	9	7	396	159	20	460	4
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None									
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage,	# -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	96	96	96	96	96	96	96	96	96	96	96	96
Heavy Vehicles, %	0	0	0	3	0	0	0	1	3	0	2	0
Mvmt Flow	3	0	3	156	2	9	7	413	166	21	479	4

Major/Minor	Minor2		[Vinor1		ſ	Major1		l	Major2			
Conflicting Flow All	1039	1116	481	1035	1035	496	483	0	0	579	0	0	
Stage 1	523	523	-	510	510	-	-	-	-	-	-	-	
Stage 2	516	593	-	525	525	-	-	-	-	-	-	-	
Critical Hdwy	7.1	6.5	6.2	7.13	6.5	6.2	4.1	-	-	4.1	-	-	
Critical Hdwy Stg 1	6.1	5.5	-	6.13	5.5	-	-	-	-	-	-	-	
Critical Hdwy Stg 2	6.1	5.5	-	6.13	5.5	-	-	-	-	-	-	-	
Follow-up Hdwy	3.5	4	3.3	3.527	4	3.3	2.2	-	-	2.2	-	-	
Pot Cap-1 Maneuver	211	209	589	209	234	578	1090	-	-	1005	-	-	
Stage 1	541	534	-	544	541	-	-	-	-	-	-	-	
Stage 2	546	497	-	534	533	-	-	-	-	-	-	-	
Platoon blocked, %								-	-		-	-	
Mov Cap-1 Maneuver	200	201	589	202	225	578	1090	-	-	1005	-	-	
Mov Cap-2 Maneuver	200	201	-	202	225	-	-	-	-	-	-	-	
Stage 1	536	519	-	539	536	-	-	-	-	-	-	-	
Stage 2	530	492	-	516	518	-	-	-	-	-	-	-	

Approach	EB	WB	NB	SB	
HCM Control Delay, s	17.3	67.5	0.1	0.4	
HCM LOS	С	F			

Minor Lane/Major Mvmt	NBL	NBT	NBR	EBLn1\	VBLn1	SBL	SBT	SBR
Capacity (veh/h)	1090	-	-	299	210	1005	-	-
HCM Lane V/C Ratio	0.007	-	-	0.021	0.799	0.021	-	-
HCM Control Delay (s)	8.3	0	-	17.3	67.5	8.7	0	-
HCM Lane LOS	А	А	-	С	F	А	А	-
HCM 95th %tile Q(veh)	0	-	-	0.1	5.7	0.1	-	-

Queues 5: Wellington Rd & Fairview Ave

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Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
Lane Group Flow (vph)	281	570	491	295	327	307
v/c Ratio	0.47	0.25	0.35	0.25	0.52	0.20
Control Delay	9.5	6.8	17.5	1.4	30.3	0.3
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	9.5	6.8	17.5	1.4	30.3	0.3
Queue Length 50th (ft)	51	54	82	4	71	0
Queue Length 95th (ft)	101	91	132	26	102	0
Internal Link Dist (ft)		1061	854		222	
Turn Bay Length (ft)	225			235		
Base Capacity (vph)	598	2260	1405	1421	1298	1568
Starvation Cap Reductn	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0
Reduced v/c Ratio	0.47	0.25	0.35	0.21	0.25	0.20
Intersection Summary						

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Movement	EBL	EBT	WBT	WBR	SBL	SBR		
Lane Configurations	5	^	44	1	ሻሻ	1		
Traffic Volume (vph)	273	553	476	286	317	298		
Future Volume (vph)	273	553	476	286	317	298		
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900		
Total Lost time (s)	7.1	7.1	7.1	6.9	6.9	4.0		
Lane Util. Factor	1.00	0.95	0.95	1.00	0.97	1.00		
Frt	1.00	1.00	1.00	0.85	1.00	0.85		
Flt Protected	0.95	1.00	1.00	1.00	0.95	1.00		
Satd. Flow (prot)	1770	3574	3539	1599	3467	1568		
Flt Permitted	0.38	1.00	1.00	1.00	0.95	1.00		
Satd. Flow (perm)	700	3574	3539	1599	3467	1568		
Peak-hour factor, PHF	0.97	0.97	0.97	0.97	0.97	0.97		
Adj. Flow (vph)	281	570	491	295	327	307		
RTOR Reduction (vph)	0	0	0	113	0	0		
Lane Group Flow (vph)	281	570	491	182	327	307		
Heavy Vehicles (%)	2%	1%	2%	1%	1%	3%		
Turn Type	pm+pt	NA	NA	pm+ov	Prot	Free		
Protected Phases	5	2	6	3	3			
Permitted Phases	2			6		Free		
Actuated Green, G (s)	47.4	47.4	29.8	43.4	13.6	75.0		
Effective Green, g (s)	47.4	47.4	29.8	43.4	13.6	75.0		
Actuated g/C Ratio	0.63	0.63	0.40	0.58	0.18	1.00		
Clearance Time (s)	7.1	7.1	7.1	6.9	6.9			
Vehicle Extension (s)	3.5	4.0	4.0	3.5	3.5			
Lane Grp Cap (vph)	592	2258	1406	925	628	1568		
v/s Ratio Prot	c0.07	0.16	0.14	0.04	c0.09			
v/s Ratio Perm	c0.23			0.08		0.20		
v/c Ratio	0.47	0.25	0.35	0.20	0.52	0.20		
Uniform Delay, d1	6.6	6.0	15.8	7.5	27.8	0.0		
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00		
Incremental Delay, d2	0.7	0.3	0.7	0.1	0.9	0.3		
Delay (s)	7.3	6.3	16.5	7.6	28.6	0.3		
Level of Service	А	А	В	А	С	A		
Approach Delay (s)		6.6	13.2		14.9			
Approach LOS		А	В		В			
Intersection Summary								
HCM 2000 Control Delay			11.2	Н	CM 2000	Level of Servic	9	В
HCM 2000 Volume to Capa	city ratio		0.52					
Actuated Cycle Length (s)			75.0	S	um of lost	time (s)		21.1
Intersection Capacity Utiliza	tion		54.9%	IC	CU Level of	of Service		А
Analysis Period (min)			15					
c Critical Lane Group								

Movement EBL EBT WBT WBR SBL SBR Lane Configurations 1 1 1 7 1 7 Traffic Volume (veh/h) 273 553 476 286 317 298 Number 5 2 6 16 3 18 Initial Q (Qb), veh 0 0 0 0 0 0 Ped-Bike Adj(A_pbT) 1.00 1.00 1.00 1.00 1.00 1.00 Ped-Bike Adj(A_pbT) 1.00 1.00 1.00 1.00 1.00 1.00 Adj Ioo Rice, veh/h 281 570 491 295 327 0 Adj Ioo of Lanes 1 2 1 1 3 2 Star Flow, veh/h 580 2424 1595 937 469 212 Arrive On Green 0.13 0.68 0.45 0.45 0.46 0.67 0.0 Grp Sat Flow, veh/h 1774		≯	-	+	*	1	~	
Lane Configurations T T T T T Traffic Volume (veh/h) 273 553 476 286 317 298 Future Volume (veh/h) 273 553 476 286 317 298 Number 5 2 6 16 3 18 Initial O (Db), veh 0 0 0 0 0 0 Parking Bus, Adj 1.00 1.00 1.00 1.00 1.00 Adj No. of Lanes 2 2 1 2 1 2 1 2 1 2 1 2 1 2 1 2 1 3 Cap, veh/h 580 2424 1595 937 469 212 2 1 3 Cap, veh/h 580 2424 1595 937 669 212 2 1 3 3 3 3 3 3 3 5 5 7 0 67 0.0 <	Movement	EBL	EBT	WBT	WBR	SBL	SBR	
Traffic Volume (veh/h) 273 553 476 286 317 298 Future Volume (veh/h) 273 553 476 286 317 298 Number 5 2 6 16 3 18 Initial Q (Qb), veh 0 0 0 0 0 Ped-Bike Adj(A, pbT) 1.00 1.00 1.00 1.00 1.00 Adj Sat Flow, veh/h 1881 1863 1881 1845 Adj Flow Rate, veh/h 281 570 491 295 327 0 Adj No. of Lanes 1 2 1 1 3 Cap. veh/h 281 570 491 295 327 0 Adj No. of Lanes 1 2 1 1 3 Cap. veh/h 0.00 Sat Flow, veh/h 174 3668 3621 1599 3476 1568 Grp Volume(v), veh/h 174 376 1568 527 0 Grp Sat Flow (s), veh/h/n 1774 1787 1799 1738 1568 Qster (og.), s 5.5	Lane Configurations	5	**	**	1	55	1	
Future Volume (veh/h) 273 553 476 286 317 298 Number 5 2 6 16 3 18 Initial Q (2b), veh 0 0 0 0 0 Ped-Bike Adj(A_pbT) 1.00 1.00 1.00 1.00 1.00 Parking Bus, Adj 1.00 1.00 1.00 1.00 1.00 Adj Sat Flow, veh/h/In 1863 1881 1881 1881 1881 1881 Adj Flow Rate, veh/h 281 570 491 295 327 0 Adj Flow Rate, veh/h 580 2424 1595 937 469 212 Arrive On Green 0.13 0.68 0.45 0.45 0.14 0.00 Sat Flow, veh/h 1774 3668 3632 1599 3476 1568 Grp Volume(y), veh/h 281 570 491 295 327 0 Grp Sat Flow, veh/h 1774 1787 1770 1599 1738 1568 Grp Volume(y), veh/h 281 570 </td <td>Traffic Volume (veh/h)</td> <td>273</td> <td>553</td> <td>476</td> <td>286</td> <td>317</td> <td>298</td> <td></td>	Traffic Volume (veh/h)	273	553	476	286	317	298	
Number 5 2 6 16 3 18 Initial Q (Qb), veh 0 0 0 0 0 0 Ped-Bike Adj(A.pbT) 1.00 1.00 1.00 1.00 1.00 1.00 Adj Sat Flow, veh/h/ln 1863 1881 1863 1881 1881 1845 Adj No. of Lanes 1 2 1 2 1 2 1 Peak Hour Factor 0.97 0.97 0.97 0.97 0.97 0.97 Percent Heavy Veh, % 2 1 2 1 3 3 Cap, veh/h 580 2424 1595 937 469 212 Arrive On Green 0.13 0.68 0.45 0.14 0.00 Sat Flow, veh/h 1774 1787 1770 1599 1738 1568 O Serve(g.s), s 5.5 4.6 6.6 7.0 0.7 0.0 Cycle O Clear(g_c.), veh/h 502 2424	Future Volume (veh/h)	273	553	476	286	317	298	
Initial Q (Qb), veh 0	Number	5	2	6	16	3	18	
Ped-Bike Adj(A_pbT) 1.00 1.00 1.00 1.00 1.00 Parking Bus, Adj 1.00 1.00 1.00 1.00 1.00 1.00 Adj Sat Flow, veh/h/ln 1863 1881 1881 1881 1845 Adj Flow Rate, veh/h 281 570 491 295 327 0 Adj No. of Lanes 1 2 2 1 2 1 3 Cap, veh/h 580 2424 1595 937 469 212 Arrive On Green 0.13 0.68 0.45 0.45 0.14 0.00 Sat Flow, veh/h 1774 3668 3632 1599 3476 1568 Grp Volume(v), veh/h 281 570 491 295 327 0 Grp Sat Flow(s), veh/h 281 570 491 295 327 0 Grp Sat Flow(s), veh/h 281 570 467 0.0 0.00 1.00 1.00 1.00 Q Clear(g_c), s 5.5 4.6 6.6 7.0 6.7 0.0 0	Initial O (Ob), veh	0	0	0	0	0	0	
Parking Bus, Adj 1.00 1.00 1.00 1.00 1.00 1.00 1.00 Adj Sat Flow, veh/h/ln 1863 1881 1863 1881 1881 1885 Adj Flow Rate, veh/h 281 570 491 295 327 0 Adj No. of Lanes 1 2 2 1 2 1 Peak Hour Factor 0.97 0.97 0.97 0.97 0.97 0.97 Percent Heavy Veh, % 2 1 2 1 1 3 Cap, veh/h 580 2424 1595 937 469 212 Arrive On Green 0.13 0.68 0.45 0.45 0.14 0.00 Sat Flow, veh/h 1774 3668 3632 1599 3476 1568 Grp Volume(v), veh/h 1774 3668 3632 1599 3476 1568 Grp Volume(v), veh/h 1774 1787 1770 1599 1738 1568 O Serve(g.s), s 5.5 4.6 6.6 7.0 6.7 0.0 Cycle Q Clear(g.c), s 5.5 4.6 6.6 7.0 6.7 0.0 Cycle Q Clear(g.c), s 5.5 4.6 6.6 7.0 6.7 0.0 Cycle Q Clear(g.c), s 5.5 4.6 6.6 7.0 6.7 0.0 Cycle Q Clear(g.c), s 5.5 4.6 0.2 410 1.00 1.00 1.00 Lane Grp Cap(c), veh/h 580 2424 1595 937 1302 587 HCM Platoon Ratio 1.00 1.00 1.00 1.00 1.00 Upstream Filter(i) 1.00 1.00 1.00 1.00 1.00 Unform Delay (d), s/veh 7.6 4.6 131 7.9 31.0 0.0 Incr Delay (d), s/veh 7.6 4.6 131 7.9 31.0 0.0 Incr Delay (d), s/veh 7.6 4.6 131 7.9 31.0 0.0 Incr Delay (d), s/veh 7.6 4.6 131 7.9 31.0 0.0 Incr Delay (d), s/veh 7.6 4.6 131 7.9 31.0 0.0 Incr Delay (d), s/veh 7.6 4.6 131 7.9 31.0 0.0 Incr Delay (d), s/veh 7.6 4.6 131 7.9 31.0 0.0 Incr Delay (d), s/veh 7.6 4.6 131 7.9 31.0 0.0 Incr Delay (d), s/veh 7.6 4.6 131 7.9 31.0 0.0 Incr Delay (d), s/veh 7.6 4.6 131 7.9 31.0 0.0 Incr Delay (d), s/veh 7.6 4.6 131 7.9 31.0 0.0 Incr Delay (d), s/veh 7.6 4.6 131 7.9 31.0 0.0 Incr Delay (d), s/veh 7.6 4.6 131 7.9 31.0 0.0 Incr Delay (d), s/veh 7.6 4.6 131 7.9 31.0 0.0 Incr Delay (d), s/veh 8.8 4 4.8 13.6 8.8 33.2 0.0 InGrp Delay(d), s/veh 8.4 4.8 13.6 8.8 33.2 0.0 InGrp Delay(d), s/veh 8.4 4.8 13.6 8.8 33.2 0.0 InGrp Delay (d), s/veh 8.4 4.8 13.6 8.8 33.2 0.0 InGrp Delay (d), s/veh 8.4 4.8 13.6 8.8 33.2 0.0 InGrp Delay (d), s/veh 8.4 4.8 13.6 8.8 33.2 0.0 InGrp Delay (d), s/veh 8.4 4.8 13.6 8.8 33.2 0.0 InGrp Delay (d), s/veh 8.4 4.8 13.6 8.8 33.2 0.0 InGrp Delay (d), s/veh 8.4 4.8 13.6 8.8 33.2 0.0 InGrp Delay (d), s/veh 8.4 4.8 13.6 8.8 33.2 0.0	Ped-Bike Adi(A pbT)	1.00	Ŭ	Ŭ	1.00	1.00	1.00	
Adj Sat Flow, veh/h/in 1863 1881 1863 1881 1881 1845 Adj Sat Flow, veh/h 281 570 491 295 327 0 Adj No. of Lanes 1 2 2 1 2 1 Peak Hour Factor 0.97 0.97 0.97 0.97 0.97 Percent Heavy Veh, % 2 1 2 1 1 3 Cap, veh/h 580 2424 1595 937 469 212 Arrive On Green 0.13 0.68 0.45 0.14 0.00 Sat Flow, veh/h 1774 3668 3632 1599 3476 1568 Grp Volume(v), veh/h 281 570 491 295 327 0 Grp Sat Flow(s), veh/h/ln 1774 1787 1770 1599 1738 1568 Q Serve(g_s), s 5.5 4.6 6.6 7.0 6.7 0.0 Prop In Lane 1.00 1.00 1.00 1.00 1.00 1.00 1.00 Avail Cap(c_a), veh/h	Parking Bus, Adi	1.00	1.00	1.00	1.00	1.00	1.00	
Adj Flow Rate, veh/h 281 570 491 295 327 0 Adj Flow Rate, veh/h 281 570 491 295 327 0 Adj No. of Lanes 1 2 2 1 2 1 2 1 Peak Hour Factor 0.97 0.97 0.97 0.97 0.97 0.97 0.97 Percent Heavy Veh, 2 1 2 1 1 3 3 3 4 69 212 Arrive On Green 0.13 0.68 0.45 0.14 0.00 3 568 57 4.66 6.7 0.45 0.14 0.00 Satt Flow, veh/h 1774 1787 1770 1599 1738 1568 Q Serve(g_s), s 5.5 4.6 6.6 7.0 6.7 0.0 7.0 Orp In Lane 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 Lane Grp Cap(c), veh/h 602 2424 1595 937 1302 587 HCM Platoon Ratio	Adi Sat Flow, veh/h/ln	1863	1881	1863	1881	1881	1845	
Adj No. of Lanes 1 2 1 2 1 2 1 Peak Hour Factor 0.97 0.97 0.97 0.97 0.97 0.97 0.97 Percent Heavy Veh, % 2 1 2 1 1 3 Cap, veh/h 580 2424 1595 937 469 212 Arrive On Green 0.13 0.68 0.45 0.45 0.14 0.00 Sat Flow, veh/h 1774 3668 3632 1599 3476 1568 Grp Volume(v), veh/h 281 570 491 295 327 0 Grp Sat Flow(s), veh/h/ln 1774 1787 1770 1599 1738 1568 Q Serve(g_s), s 5.5 4.6 6.6 7.0 6.7 0.0 Cycle Q Clear(g_c), s 5.5 4.6 6.6 7.0 0.7 0.0 Itane 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 V/C Ratio(X) 0.48 0.24 0.31 0.31 0.70 <td>Adj Flow Rate, veh/h</td> <td>281</td> <td>570</td> <td>491</td> <td>295</td> <td>327</td> <td>0</td> <td></td>	Adj Flow Rate, veh/h	281	570	491	295	327	0	
Peak Hour Factor 0.97 0.97 0.97 0.97 0.97 Percent Heavy Veh, % 2 1 2 1 1 3 Cap, veh/h 580 2424 1595 937 469 212 Arrive On Green 0.13 0.68 0.45 0.14 0.00 Sat Flow, veh/h 1774 3668 3632 1599 3476 1568 Grp Volume(v), veh/h 281 570 491 295 327 0 Grp Sat Flow(s), veh/h/ln 1774 1787 1770 1599 1738 1568 Q Serve(g_s), s 5.5 4.6 6.6 7.0 6.7 0.0 Prop In Lane 1.00 1.00 1.00 1.00 1.00 1.00 Lane Grp Cap(c), veh/h 580 2424 1595 937 469 212 V/C Ratio(X) 0.48 0.24 0.31 0.70 0.00 1.00 1.00 LAP Patoon Ratio 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 </td <td>Adi No. of Lanes</td> <td>1</td> <td>2</td> <td>2</td> <td>1</td> <td>2</td> <td>1</td> <td></td>	Adi No. of Lanes	1	2	2	1	2	1	
Percent Heavy Veh, % 2 1 2 1 1 3 Cap, veh/h 580 2424 1595 937 469 212 Arrive On Green 0.13 0.68 0.45 0.45 0.14 0.00 Sat Flow, veh/h 1774 3668 3632 1599 3476 1568 Grp Volume(v), veh/h 281 570 491 295 327 0 Gr Sat Flow(s), veh/h/ln 1774 1787 1770 1599 1738 1568 Q Serve(g_s), s 5.5 4.6 6.6 7.0 6.7 0.0 Prop In Lane 1.00 <t< td=""><td>Peak Hour Factor</td><td>0.97</td><td>0.97</td><td>0.97</td><td>0.97</td><td>0.97</td><td>0.97</td><td></td></t<>	Peak Hour Factor	0.97	0.97	0.97	0.97	0.97	0.97	
Cap, veh/h 580 2424 1595 937 469 212 Arrive On Green 0.13 0.68 0.45 0.45 0.14 0.00 Sat Flow, veh/h 1774 3668 3632 1599 3476 1568 Grp Volume(v), veh/h 1774 1770 1599 1738 1568 0 Grp Sat Flow(s), veh/h/ln 1774 1787 1770 1599 1738 1568 0 O Serve(g_s), s 5.5 4.6 6.6 7.0 6.7 0.0 0 Cycle Q Clear(g_c), s 5.5 4.6 6.6 7.0 6.7 0.0 Cycle Q Clear(g_c), s 5.5 4.6 6.6 7.0 6.7 0.0 Prop In Lane 1.00 1.00 1.00 1.00 1.00 1.00 1.00 V/C Ratio(X) 0.48 0.24 0.31 0.31 0.70 0.00 V/C Ratio(X) 0.48 0.2 0.42 1.00 1.00 1.00 1.00 1.00 Upstream Filter(I) 1.00 1.00	Percent Heavy Veh. %	2	1	2	1	1	3	
Arrive On Green 0.13 0.68 0.45 0.14 0.00 Sat Flow, veh/h 1774 3668 3632 1599 3476 1568 Grp Volume(v), veh/h 281 570 491 295 327 0 Grp Sat Flow(s), veh/h 1774 1787 1770 1599 1738 1568 Q Serve(g_s), s 5.5 4.6 6.6 7.0 6.7 0.0 Cycle Q Clear(g_c), s 5.5 4.6 6.6 7.0 0.0 Prop In Lane 1.00 1.00 1.00 1.00 Lane Grp Cap(c), veh/h 580 2424 1595 937 469 212 V/C Ratio(X) 0.48 0.24 0.31 0.31 0.70 0.00 Avail Cap(c_a), veh/h 602 2424 1595 937 1302 587 HCM Platoon Ratio 1.00 1.00 1.00 1.00 1.00 1.00 1.00 Upstream Filter(I) 1.00 1.00 1.00 1.00 0.0 0.0 0.0 Intia Q Delay(d), s/veh 0.8	Cap, veh/h	580	2424	1595	937	469	212	
Sat Flow, veh/h 1774 3668 3632 1599 3476 1568 Grp Volume(v), veh/h 281 570 491 295 327 0 Grp Sat Flow(s), veh/h/ln 1774 1787 1770 1599 1738 1568 Q Serve(g_s), s 5.5 4.6 6.6 7.0 6.7 0.0 Cycle Q Clear(g_c), s 5.5 4.6 6.6 7.0 6.7 0.0 Prop In Lane 1.00 1.00 1.00 1.00 1.00 1.00 Lane Grp Cap(C), veh/h 580 2424 1595 937 1302 587 HCM Platoon Ratio 1.00 1.00 1.00 1.00 1.00 1.00 1.00 Uniform Delay (d), s/veh 7.6 4.6 13.1 7.9 31.0 0.0 Incr Delay (d2), s/veh 0.8 0.2 0.5 0.9 2.3 0.0 Initial Q Delay(d3), s/veh 0.8 0.2 0.5 0.9 2.3 0.0 Incr Delay (d2), s/veh 8.4 4.8 13.6 8.8	Arrive On Green	0.13	0.68	0.45	0.45	0.14	0.00	
Grp Volume(v), veh/h 281 570 491 295 327 0 Grp Sat Flow(s), veh/h/ln 1774 1787 1770 1599 1738 1568 Q Serve(g_s), s 5.5 4.6 6.6 7.0 6.7 0.0 Cycle Q Clear(g_c), s 5.5 4.6 6.6 7.0 6.7 0.0 Prop In Lane 1.00 1.00 1.00 1.00 1.00 1.00 Lane Grp Cap(c), veh/h 580 2424 1595 937 469 212 V/C Ratio(X) 0.48 0.24 0.31 0.31 0.70 0.00 Avail Cap(c_a), veh/h 602 2424 1595 937 1302 587 HCM Platoon Ratio 1.00 1.00 1.00 1.00 1.00 1.00 1.00 Uniform Delay (d), s/veh 7.6 4.6 13.1 7.9 31.0 0.0 Initial O Delay(d3), s/veh 0.8 0.2 0.5 0.9 2.3 0.0 Initial O Delay(d3), s/veh 8.4 4.8 13.6 8.8	Sat Flow, veh/h	1774	3668	3632	1599	3476	1568	
arr Antholy, Shink 10, 1774 1787 1778 1778 1599 1738 1568 Q Serve(g_s), s 5.5 4.6 6.6 7.0 6.7 0.0 Cycle Q Clear(g_c), s 5.5 4.6 6.6 7.0 6.7 0.0 Prop In Lane 1.00 1.00 1.00 1.00 1.00 1.00 Lane Grp Cap(c), veh/h 580 2424 1595 937 469 212 V/C Ratio(X) 0.48 0.24 0.31 0.31 0.70 0.00 Avail Cap(c_a), veh/h 602 2424 1595 937 1302 587 HCM Platoon Ratio 1.00 1.00 1.00 1.00 1.00 1.00 1.00 Upstream Filter(I) 1.00 1.00 1.00 1.00 0.00 0.0 Uniform Delay (d2), s/veh 0.8 0.2 0.5 0.9 2.3 0.0 Intital Q Delay(d3), s/veh 0.8 0.2 0.5 0.9 2.3 0.0 Indirg Delay (dy), s/veh 8.4 4.8 13.6 8.8 <td>Grp Volume(v), veh/h</td> <td>281</td> <td>570</td> <td>491</td> <td>295</td> <td>327</td> <td>0</td> <td></td>	Grp Volume(v), veh/h	281	570	491	295	327	0	
C Serve(g_s), s 5.5 4.6 6.6 7.0 6.7 0.0 Cycle Q Clear(g_c), s 5.5 4.6 6.6 7.0 6.7 0.0 Prop In Lane 1.00 1.00 1.00 1.00 1.00 1.00 Lane Grp Cap(c), veh/h 580 2424 1595 937 469 212 V/C Ratio(X) 0.48 0.24 0.31 0.31 0.70 0.00 Avail Cap(c_a), veh/h 602 2424 1595 937 1302 587 HCM Platoon Ratio 1.00 1.00 1.00 1.00 1.00 1.00 1.00 Upstream Filter(I) 1.00 1.00 1.00 1.00 0.00 0.0 Uniform Delay (d2), s/veh 0.8 0.2 0.5 0.9 2.3 0.0 Initial Q Delay(d3), s/veh 0.8 0.2 0.5 0.9 2.3 0.0 Indifia Q Delay(d3), s/veh 8.4 4.8 13.6 8.8 33.2 0.0 InGrp Delay(d), s/veh 8.4 4.8 13.6 8.8	Grp Sat Flow(s).veh/h/ln	1774	1787	1770	1599	1738	1568	
Line Line <thline< th=""> Line Line</thline<>	O Serve(a, s), s	5.5	4.6	6.6	7.0	6.7	0.0	
Prop In Lane 1.00 1.00 1.00 1.00 Prop In Lane 1.00 1.00 1.00 1.00 Lane Grp Cap(C), veh/h 580 2424 1595 937 469 212 V/C Ratio(X) 0.48 0.24 0.31 0.31 0.70 0.00 Avail Cap(c_a), veh/h 602 2424 1595 937 1302 587 HCM Platoon Ratio 1.00 1.00 1.00 1.00 1.00 1.00 1.00 Upstream Filter(I) 1.00 1.00 1.00 1.00 1.00 0.00 Uniform Delay (d), s/veh 7.6 4.6 13.1 7.9 31.0 0.0 Incr Delay (d2), s/veh 0.8 0.2 0.5 0.9 2.3 0.0 Initial Q Delay(d3), s/veh 0.8 0.2 0.5 0.9 2.3 0.0 InGrp Delay (d2), s/veh 8.4 4.8 13.6 8.8 33.2 0.0 InGrp Delay, (s/veh 8.0 11.8 33.2 Approach LOS A B C	Cycle O Clear(a, c), s	5.5	4.6	6.6	7.0	6.7	0.0	
Lane Grp Cap(c), veh/h 580 2424 1595 937 469 212 V/C Ratio(X) 0.48 0.24 0.31 0.31 0.70 0.00 Avail Cap(c_a), veh/h 602 2424 1595 937 1302 587 HCM Platoon Ratio 1.00 1.00 1.00 1.00 1.00 1.00 1.00 Upstream Filter(I) 1.00 1.00 1.00 1.00 0.00 0.00 Uniform Delay (d), s/veh 7.6 4.6 13.1 7.9 31.0 0.0 Incr Delay (d2), s/veh 0.8 0.2 0.5 0.9 2.3 0.0 Initial Q Delay(d3), s/veh 0.8 0.2 0.5 0.9 2.3 0.0 InGr Delay (d2), s/veh 8.4 4.8 13.6 8.8 33.2 0.0 InGrp Delay(d3), s/veh 8.4 4.8 13.6 8.8 33.2 0.0 InGrp Delay, (s/veh 6.0 11.8 33.2 Approach LOS A B C Timer 1 2 3 4	Prop In Lane	1.00		0.0	1.00	1.00	1.00	
V/C Ratio(X) 0.48 0.24 0.31 0.31 0.70 0.00 Avail Cap(c_a), veh/h 602 2424 1595 937 1302 587 HCM Platoon Ratio 1.00 1.00 1.00 1.00 1.00 1.00 1.00 Upstream Filter(I) 1.00 1.00 1.00 1.00 1.00 0.00 Uniform Delay (d), s/veh 7.6 4.6 13.1 7.9 31.0 0.0 Incr Delay (d2), s/veh 0.8 0.2 0.5 0.9 2.3 0.0 Initial Q Delay(d3), s/veh 0.0 0.0 0.0 0.0 0.0 0.0 Inforp Delay(d), s/veh 8.4 4.8 13.6 8.8 33.2 0.0 Inforp Delay(d), s/veh 8.4 4.8 13.6 8.8 33.2 0.0 LnGrp Delay(d), s/veh 8.4 4.8 13.6 8.8 33.2 0.0 LnGrp Delay(d), s/veh 8.4 4.8 13.6 8.8 33.2 0.0 LnGrp Delay(d), s/veh 6.0 11.8 33.2 33.	Lane Grp Cap(c), veh/h	580	2424	1595	937	469	212	
Avail Cap(c_a), veh/h 602 2424 1595 937 1302 587 HCM Platoon Ratio 1.00 1.00 1.00 1.00 1.00 1.00 1.00 Upstream Filter(I) 1.00 1.00 1.00 1.00 1.00 0.00 Uniform Delay (d), s/veh 7.6 4.6 13.1 7.9 31.0 0.0 Incr Delay (d2), s/veh 0.8 0.2 0.5 0.9 2.3 0.0 Initial Q Delay(d3), s/veh 0.0 0.0 0.0 0.0 0.0 0.0 LnGrp Delay(d), s/veh 8.4 4.8 13.6 8.8 33.2 0.0 LnGrp Delay(d), s/veh 8.4 4.8 13.6 8.8 33.2 0.0 LnGrp Delay(d), s/veh 8.4 4.8 13.6 8.8 33.2 0.0 LnGrp LOS A A B C	V/C Ratio(X)	0.48	0.24	0.31	0.31	0.70	0.00	
HCM Platoon Ratio 1.00 1.00 1.00 1.00 1.00 Upstream Filter(I) 1.00 1.00 1.00 1.00 0.00 Uniform Delay (d), s/veh 7.6 4.6 13.1 7.9 31.0 0.0 Incr Delay (d2), s/veh 0.8 0.2 0.5 0.9 2.3 0.0 Initial Q Delay(d3), s/veh 0.0 0.0 0.0 0.0 0.0 0.0 %ile BackOfQ(50%), veh/ln 2.8 2.3 3.3 3.4 0.0 0.0 LnGrp Delay(d), s/veh 8.4 4.8 13.6 8.8 33.2 0.0 LnGrp LOS A A B A C Approach Vol, veh/h 851 786 327 Approach LOS A B C C C Signed Phs 2 5 6 8 Phs Duration (G+Y+Rc), s 58.0 17.1 40.9 17.0 Change Period (Y+Rc), s 7.1 7.1 6.9 Max Green Setting (Gmax), s 32.9 10.9 14.9 28.1 Max Q Clear Time (p	Avail Cap(c_a), veh/h	602	2424	1595	937	1302	587	
Upstream Filter(I) 1.00 1.00 1.00 1.00 0.00 Uniform Delay (d), s/veh 7.6 4.6 13.1 7.9 31.0 0.0 Incr Delay (d2), s/veh 0.8 0.2 0.5 0.9 2.3 0.0 Initial Q Delay(d3), s/veh 0.0 0.0 0.0 0.0 0.0 0.0 %ile BackOfQ(50%), veh/ln 2.8 2.3 3.3 3.3 3.4 0.0 LnGrp Delay(d), s/veh 8.4 4.8 13.6 8.8 33.2 0.0 LnGrp LOS A A B A C Approach Vol, veh/h 851 786 327 Approach LOS A B C C State	HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	
Uniform Delay (d), s/veh 7.6 4.6 13.1 7.9 31.0 0.0 Incr Delay (d2), s/veh 0.8 0.2 0.5 0.9 2.3 0.0 Initial Q Delay(d3), s/veh 0.0 0.0 0.0 0.0 0.0 %ile BackOfQ(50%), veh/ln 2.8 2.3 3.3 3.4 0.0 LnGrp Delay(d), s/veh 8.4 4.8 13.6 8.8 33.2 0.0 LnGrp Delay(d), s/veh 8.4 4.8 13.6 8.8 33.2 0.0 LnGrp Delay(d), s/veh 8.4 4.8 13.6 8.8 33.2 0.0 LnGrp DOS A A B A C	Upstream Filter(I)	1.00	1.00	1.00	1.00	1.00	0.00	
Incr Delay (d2), s/veh 0.8 0.2 0.5 0.9 2.3 0.0 Initial Q Delay(d3), s/veh 0.0 0.0 0.0 0.0 0.0 0.0 %ile BackOfQ(50%), veh/ln 2.8 2.3 3.3 3.4 0.0 LnGrp Delay(d), s/veh 8.4 4.8 13.6 8.8 33.2 0.0 LnGrp Delay(d), s/veh 8.4 4.8 13.6 8.8 33.2 0.0 LnGrp LOS A A B A C Approach Vol, veh/h 851 786 327 Approach Delay, s/veh 6.0 11.8 33.2 Approach LOS A B C Timer 1 2 3 4 5 6 7 8 Assigned Phs 2 5 6 8 8 9 17.0 0 17.0 Change Period (Y+Rc), s 7.1 7.1 7.1 6.9 8.7 0 8.7 Green Setting (Gmax), s 32.9 10.9 14.9 28.1 1.4 Max Q Clear Time (pC +I1)	Uniform Delay (d), s/veh	7.6	4.6	13.1	7.9	31.0	0.0	
Initial Q Delay(d3),s/veh 0.0 0.0 0.0 0.0 0.0 %ile BackOfQ(50%),veh/ln 2.8 2.3 3.3 3.4 0.0 LnGrp Delay(d),s/veh 8.4 4.8 13.6 8.8 33.2 0.0 LnGrp Delay(d),s/veh 8.4 4.8 13.6 8.8 33.2 0.0 LnGrp LOS A A B A C	Incr Delay (d2), s/veh	0.8	0.2	0.5	0.9	2.3	0.0	
%ile BackOfQ(50%),veh/ln 2.8 2.3 3.3 3.3 3.4 0.0 LnGrp Delay(d),s/veh 8.4 4.8 13.6 8.8 33.2 0.0 LnGrp LOS A A B A C Approach Vol, veh/h 851 786 327 Approach Delay, s/veh 6.0 11.8 33.2 Approach LOS A B C Timer 1 2 3 4 5 6 7 8 Assigned Phs 2 5 6 8 8 9 17.0 Change Period (Y+Rc), s 7.1 7.1 7.1 6.9 9 17.0 Change Period (Y+Rc), s 7.1 7.1 7.1 6.9 8.7 Max Green Setting (Gmax), s 32.9 10.9 14.9 28.1 Max Q Clear Time (g_c+I1), s 6.6 7.5 9.0 8.7 Green Ext Time (p_c), s 6.0 0.4 2.9 1.4 Intersection Summary 12.9 12.9 1.4	Initial Q Delay(d3), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	
LnGrp Delay(d),s/veh 8.4 4.8 13.6 8.8 33.2 0.0 LnGrp LOS A A B A C Approach Vol, veh/h 851 786 327 Approach Delay, s/veh 6.0 11.8 33.2 0.0 Timer 1 2 3 4 5 6 7 8 Approach LOS A B C 7 8 7 8 7 8 Approach LOS A B C 7 8 7 8 7 8 Assigned Phs 2 3 4 5 6 7 8 8 Assigned Phs 2 5 6 8 8 9 17.0 17.0 17.1 40.9 17.0 17.0 17.1 6.9 17.1 6.9 18.7 19.9 18.7 19.9 18.7 19.9 18.7 19.9 18.7 19.9 18.7 19.9 14.9 28.1 14.9 28.1 14.9 28.1 14.9 28.	%ile BackOfQ(50%),veh/In	2.8	2.3	3.3	3.3	3.4	0.0	
LnGrp LOS A A B A C Approach Vol, veh/h 851 786 327 Approach Delay, s/veh 6.0 11.8 33.2 Approach LOS A B C Timer 1 2 3 4 5 6 7 8 Assigned Phs 2 5 6 8 8 Phs Duration (G+Y+Rc), s 58.0 17.1 40.9 17.0 Change Period (Y+Rc), s 7.1 7.1 7.1 6.9 8 Max Green Setting (Gmax), s 32.9 10.9 14.9 28.1 Max Q Clear Time (g_c+I1), s 6.6 7.5 9.0 8.7 Green Ext Time (p_c), s 6.0 0.4 2.9 1.4 Intersection Summary 12.9 12.9 12.9	LnGrp Delay(d).s/veh	8.4	4.8	13.6	8.8	33.2	0.0	
Approach Vol, veh/h 851 786 327 Approach Delay, s/veh 6.0 11.8 33.2 Approach LOS A B C Timer 1 2 3 4 5 6 7 8 Assigned Phs 2 5 6 8 Phs Duration (G+Y+Rc), s 58.0 17.1 40.9 17.0 Change Period (Y+Rc), s 7.1 7.1 7.1 6.9 9 17.0 Change Period (Y+Rc), s 7.1 7.1 7.1 6.9 14.9 28.1 Max Green Setting (Gmax), s 32.9 10.9 14.9 28.1 Max Q Clear Time (g_c+I1), s 6.6 7.5 9.0 8.7 Green Ext Time (p_c), s 6.0 0.4 2.9 1.4 Intersection Summary 12.9 12.9 12.9 12.9	LnGrp LOS	A	A	В	A	С		
Approach Delay, s/veh 6.0 11.8 33.2 Approach LOS A B C Timer 1 2 3 4 5 6 7 8 Assigned Phs 2 5 6 8 Phs Duration (G+Y+Rc), s 58.0 17.1 40.9 17.0 Change Period (Y+Rc), s 7.1 7.1 7.1 6.9 Max Green Setting (Gmax), s 32.9 10.9 14.9 28.1 Max Q Clear Time (g_c+I1), s 6.6 7.5 9.0 8.7 Green Ext Time (p_c), s 6.0 0.4 2.9 1.4 Intersection Summary 12.9 12.9 12.9	Approach Vol, veh/h		851	786		327		
Approach LOS A B C Timer 1 2 3 4 5 6 7 8 Assigned Phs 2 5 6 8 Phs Duration (G+Y+Rc), s 58.0 17.1 40.9 17.0 Change Period (Y+Rc), s 7.1 7.1 7.1 6.9 Max Green Setting (Gmax), s 32.9 10.9 14.9 28.1 Max Q Clear Time (g_c+I1), s 6.6 7.5 9.0 8.7 Green Ext Time (p_c), s 6.0 0.4 2.9 1.4 Intersection Summary 12.9 12.9 12.9	Approach Delay, s/veh		6.0	11.8		33.2		
Timer 1 2 3 4 5 6 7 8 Assigned Phs 2 5 6 8 8 Phs Duration (G+Y+Rc), s 58.0 17.1 40.9 17.0 Change Period (Y+Rc), s 7.1 7.1 7.1 6.9 Max Green Setting (Gmax), s 32.9 10.9 14.9 28.1 Max Q Clear Time (g_c+11), s 6.6 7.5 9.0 8.7 Green Ext Time (p_c), s 6.0 0.4 2.9 1.4 Intersection Summary 12.9 12.9 14.9	Approach LOS		A	В		С		
Assigned Phs 2 5 6 8 Phs Duration (G+Y+Rc), s 58.0 17.1 40.9 17.0 Change Period (Y+Rc), s 7.1 7.1 7.1 6.9 Max Green Setting (Gmax), s 32.9 10.9 14.9 28.1 Max Q Clear Time (g_c+I1), s 6.6 7.5 9.0 8.7 Green Ext Time (p_c), s 6.0 0.4 2.9 1.4 Intersection Summary 12.9 12.9 14.9	Timer	1	2	3	4	5	6	7 8
Phs Duration (G+Y+Rc), s 58.0 17.1 40.9 17.0 Change Period (Y+Rc), s 7.1 7.1 7.1 6.9 Max Green Setting (Gmax), s 32.9 10.9 14.9 28.1 Max Q Clear Time (g_c+I1), s 6.6 7.5 9.0 8.7 Green Ext Time (p_c), s 6.0 0.4 2.9 1.4 Intersection Summary 12.9 12.9 12.9	Assigned Phs		2	Ŭ		5	6	8
Change Period (Y+Rc), s 7.1 7.1 7.1 6.9 Max Green Setting (Gmax), s 32.9 10.9 14.9 28.1 Max Q Clear Time (g_c+I1), s 6.6 7.5 9.0 8.7 Green Ext Time (p_c), s 6.0 0.4 2.9 1.4 Intersection Summary 12.9 12.9 12.9	Phy Duration $(G_+Y_+R_c)$ s					17 1	40.9	17 0
Max Green Setting (Gmax), s 32.9 10.9 14.9 28.1 Max Q Clear Time (g_c+I1), s 6.6 7.5 9.0 8.7 Green Ext Time (p_c), s 6.0 0.4 2.9 1.4 Intersection Summary 12.9 12.9 14.9 UCM 2010 Ctrl Delay 12.9 12.9 12.9	Change Period (V+Rc) s		7 1			7.1	71	6.9
Max Q Clear Time (g_c+11), s 6.6 7.5 9.0 8.7 Green Ext Time (p_c), s 6.0 0.4 2.9 1.4 Intersection Summary 12.9 1.4	Max Green Setting (Gmax) s		32.9			10.9	14.9	28.1
Intersection Summary 12.9 1.4 HCM 2010 Ctrl Delay 12.9	Max O Clear Time $(a, c+11)$ s		6.6			7.5	9.0	8.7
Intersection Summary HCM 2010 Ctrl Delay 12.9	Green Ext Time (p c), s		6.0			0.4	2.9	1.4
HCM 2010 Ctrl Delay 12.9	Intersection Summary							
	HCM 2010 Ctrl Dolay			12.0				
	HCM 2010 Cill Delay			12.7 R				

G. Intersection Capacity Analysis Worksheets - 2027 Future with Development

Intersection

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4		٦	eî 👘			4	
Traffic Vol, veh/h	21	1	168	0	1	5	262	530	1	1	287	50
Future Vol, veh/h	21	1	168	0	1	5	262	530	1	1	287	50
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None									
Storage Length	-	-	-	-	-	-	280	-	-	-	-	-
Veh in Median Storage,	# -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	92	92	92	92	92	92	92	92	92	92	92	92
Heavy Vehicles, %	0	0	6	0	0	20	3	3	0	0	2	6
Mvmt Flow	23	1	183	0	1	5	285	576	1	1	312	54

Major/Minor	Minor2		N	Ainor1			Major1			N	lajor2			
Conflicting Flow All	1491	1488	339	1580	1515	577	366	0	(0	577	0	0	
Stage 1	341	341	-	1147	1147	-	-	-		-	-	-	-	
Stage 2	1150	1147	-	433	368	-	-	-		-	-	-	-	
Critical Hdwy	7.1	6.5	6.26	7.1	6.5	6.4	4.13	-		-	4.1	-	-	
Critical Hdwy Stg 1	6.1	5.5	-	6.1	5.5	-	-	-		-	-	-	-	
Critical Hdwy Stg 2	6.1	5.5	-	6.1	5.5	-	-	-		-	-	-	-	
Follow-up Hdwy	3.5	4	3.354	3.5	4	3.48	2.227	-		-	2.2	-	-	
Pot Cap-1 Maneuver	103	125	694	89	121	484	1187	-		-	1006	-	-	
Stage 1	678	642	-	244	276	-	-	-		-	-	-	-	
Stage 2	243	276	-	605	625	-	-	-		-	-	-	-	
Platoon blocked, %								-		-		-	-	
Mov Cap-1 Maneuver	. 82	95	694	53	92	484	1187	-		-	1006	-	-	
Mov Cap-2 Maneuver	82	95	-	53	92	-	-	-		-	-	-	-	
Stage 1	515	641	-	185	210	-	-	-		-	-	-	-	
Stage 2	182	210	-	445	624	-	-	-		-	-	-	-	

Approach	EB	WB	NB	SB	
HCM Control Delay, s	25.9	18	3	0	
HCM LOS	D	С			

Minor Lane/Major Mvmt	NBL	NBT	NBR	EBLn1V	VBLn1	SBL	SBT	SBR
Capacity (veh/h)	1187	-	-	374	283	1006	-	-
HCM Lane V/C Ratio	0.24	-	-	0.552	0.023	0.001	-	-
HCM Control Delay (s)	9	-	-	25.9	18	8.6	0	-
HCM Lane LOS	А	-	-	D	С	А	А	-
HCM 95th %tile Q(veh)	0.9	-	-	3.2	0.1	0	-	-

Queues	
2: Fairview Ave & Manassas PD Driveway/Signal Hill Rd	

	-	-	•	1	†	1	Ŧ
Lane Group	EBT	WBT	WBR	NBL	NBT	SBL	SBT
Lane Group Flow (vph)	19	58	317	4	571	182	298
v/c Ratio	0.10	0.25	0.66	0.01	0.73	0.42	0.26
Control Delay	38.9	36.0	11.7	9.2	28.4	10.6	11.2
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	38.9	36.0	11.7	9.2	28.4	10.6	11.2
Queue Length 50th (ft)	8	23	0	1	176	22	37
Queue Length 95th (ft)	34	67	75	6	#544	90	198
Internal Link Dist (ft)	120	611			159		588
Turn Bay Length (ft)			110			260	
Base Capacity (vph)	563	555	709	794	781	497	1130
Starvation Cap Reductn	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0
Reduced v/c Ratio	0.03	0.10	0.45	0.01	0.73	0.37	0.26

Intersection Summary

95th percentile volume exceeds capacity, queue may be longer. Queue shown is maximum after two cycles. #

HCM Signalized Intersection Capacity Analysis 2: Fairview Ave & Manassas PD Driveway/Signal Hill Rd

Timing Plan: EX AM

	≤	۶	-	$\mathbf{\hat{z}}$	•	-	•	1	1	1	1	Ŧ
Movement	EBU	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT
Lane Configurations			4			ર્સ	1	۲.	4Î		۲	f)
Traffic Volume (vph)	1	12	5	0	42	13	301	4	489	53	173	252
Future Volume (vph)	1	12	5	0	42	13	301	4	489	53	173	252
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)			6.3			6.1	6.1	6.5	6.5		6.5	6.5
Lane Util. Factor			1.00			1.00	1.00	1.00	1.00		1.00	1.00
Frt			1.00			1.00	0.85	1.00	0.99		1.00	0.98
Flt Protected			0.96			0.96	1.00	0.95	1.00		0.95	1.00
Satd. Flow (prot)			1832			1790	1583	1805	1792		1719	1836
Flt Permitted			0.96			0.96	1.00	0.58	1.00		0.23	1.00
Satd. Flow (perm)			1832			1790	1583	1098	1792		420	1836
Peak-hour factor, PHF	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Adj. Flow (vph)	1	13	5	0	44	14	317	4	515	56	182	265
RTOR Reduction (vph)	0	0	0	0	0	0	280	0	3	0	0	2
Lane Group Flow (vph)	0	0	19	0	0	58	37	4	568	0	182	296
Heavy Vehicles (%)	0%	0%	0%	0%	3%	0%	2%	0%	4%	9%	5%	2%
Turn Type	Split	Split	NA		Split	NA	Perm	pm+pt	NA		pm+pt	NA
Protected Phases	. 4	4	4		3	3		5	2		1	6
Permitted Phases							3	2			6	
Actuated Green, G (s)			2.7			10.2	10.2	41.1	39.8		56.2	48.4
Effective Green, g (s)			2.7			10.2	10.2	41.1	39.8		56.2	48.4
Actuated g/C Ratio			0.03			0.12	0.12	0.47	0.45		0.64	0.55
Clearance Time (s)			6.3			6.1	6.1	6.5	6.5		6.5	6.5
Vehicle Extension (s)			3.0			4.0	4.0	3.0	5.0		3.0	5.0
Lane Grp Cap (vph)			56			207	183	523	810		414	1009
v/s Ratio Prot			c0.01			c0.03		0.00	c0.32		c0.05	0.16
v/s Ratio Perm							0.02	0.00			0.23	
v/c Ratio			0.34			0.28	0.20	0.01	0.70		0.44	0.29
Uniform Delay, d1			41.8			35.5	35.2	12.5	19.3		10.1	10.6
Progression Factor			1.00			1.00	1.00	1.00	1.00		1.00	1.00
Incremental Delay, d2			3.6			1.0	0.7	0.0	3.5		0.7	0.3
Delay (s)			45.4			36.6	36.0	12.5	22.8		10.8	11.0
Level of Service			D			D	D	В	С		В	В
Approach Delay (s)			45.4			36.0			22.7			10.9
Approach LOS			D			D			С			В
Intersection Summary												
HCM 2000 Control Delay			22.6	Н	CM 2000	Level of	Service		С			
HCM 2000 Volume to Capacit	y ratio		0.58									
Actuated Cycle Length (s)			88.0	S	um of los	t time (s)			25.4			
Intersection Capacity Utilization	n		70.0%	IC	CU Level	of Service)		С			
Analysis Period (min)			15									
c Critical Lane Group												

1

Movement	SBR	
Lane Configurations		
Traffic Volume (vph)	31	
Future Volume (vph)	31	
Ideal Flow (vphpl)	1900	
Total Lost time (s)		
Lane Util. Factor		
Frt		
Flt Protected		
Satd. Flow (prot)		
Flt Permitted		
Satd. Flow (perm)		
Peak-hour factor, PHF	0.95	
Adj. Flow (vph)	33	
RTOR Reduction (vph)	0	
Lane Group Flow (vph)	0	
Heavy Vehicles (%)	0%	
Turn Type		
Protected Phases		
Permitted Phases		
Actuated Green, G (s)		
Effective Green, g (s)		
Actuated g/C Ratio		
Clearance Time (s)		
Vehicle Extension (s)		
Lane Grp Cap (vph)		
v/s Ratio Prot		
v/s Ratio Perm		
v/c Ratio		
Uniform Delay, d1		
Progression Factor		
Incremental Delay, d2		
Delay (s)		
Level of Service		
Approach Delay (s)		
Approach LOS		
Intersection Summary		

HCM 2010 analysis expects strict NEMA phasing.

Intersection

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			र्च			ef 👘	
Traffic Vol, veh/h	8	0	17	2	1	2	33	552	2	1	277	11
Future Vol, veh/h	8	0	17	2	1	2	33	552	2	1	277	11
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None									
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage,	# -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	92	92	92	92	92	92	92	92	92	92	92	92
Heavy Vehicles, %	0	0	100	0	0	0	0	5	0	0	3	0
Mvmt Flow	9	0	18	2	1	2	36	600	2	1	301	12

Major/Minor	Minor2		Ν	1inor1		N	Najor1		N	lajor2			
Conflicting Flow All	984	983	307	991	988	601	313	0	0	602	0	0	
Stage 1	309	309	-	673	673	-	-	-	-	-	-	-	
Stage 2	675	674	-	318	315	-	-	-	-	-	-	-	
Critical Hdwy	7.1	6.5	7.2	7.1	6.5	6.2	4.1	-	-	4.1	-	-	
Critical Hdwy Stg 1	6.1	5.5	-	6.1	5.5	-	-	-	-	-	-	-	
Critical Hdwy Stg 2	6.1	5.5	-	6.1	5.5	-	-	-	-	-	-	-	
Follow-up Hdwy	3.5	4	4.2	3.5	4	3.3	2.2	-	-	2.2	-	-	
Pot Cap-1 Maneuver	229	251	552	227	249	504	1259	-	-	985	-	-	
Stage 1	705	663	-	448	457	-	-	-	-	-	-	-	
Stage 2	447	457	-	698	659	-	-	-	-	-	-	-	
Platoon blocked, %								-	-		-	-	
Mov Cap-1 Maneuver	220	240	552	212	238	504	1259	-	-	985	-	-	
Mov Cap-2 Maneuver	220	240	-	212	238	-	-	-	-	-	-	-	
Stage 1	675	662	-	429	437	-	-	-	-	-	-	-	
Stage 2	425	437	-	674	658	-	-	-	-	-	-	-	

Approach	EB	WB	NB	SB	
HCM Control Delay, s	15.4	17.9	0.4	0	
HCM LOS	С	С			

Minor Lane/Major Mvmt	NBL	NBT	NBR	EBLn1V	VBLn1	SBL	SBT	SBR
Capacity (veh/h)	1259	-	-	372	284	985	-	-
HCM Lane V/C Ratio	0.028	-	-	0.073	0.019	0.001	-	-
HCM Control Delay (s)	7.9	0	-	15.4	17.9	8.7	-	-
HCM Lane LOS	А	А	-	С	С	А	-	-
HCM 95th %tile Q(veh)	0.1	-	-	0.2	0.1	0	-	-

Intersection

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Vol, veh/h	2	1	3	81	2	30	4	565	107	16	279	3
Future Vol, veh/h	2	1	3	81	2	30	4	565	107	16	279	3
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None									
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage,	# -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	94	94	94	94	94	94	94	94	94	94	94	94
Heavy Vehicles, %	100	0	0	2	0	5	25	4	5	17	2	0
Mvmt Flow	2	1	3	86	2	32	4	601	114	17	297	3

Major/Minor	Minor2		[Vinor1			Major1		N	lajor2			
Conflicting Flow All	1016	1056	299	1001	1000	658	300	0	0	715	0	0	
Stage 1	333	333	-	666	666	-	-	-	-	-	-	-	
Stage 2	683	723	-	335	334	-	-	-	-	-	-	-	
Critical Hdwy	8.1	6.5	6.2	7.12	6.5	6.25	4.35	-	-	4.27	-	-	
Critical Hdwy Stg 1	7.1	5.5	-	6.12	5.5	-	-	-	-	-	-	-	
Critical Hdwy Stg 2	7.1	5.5	-	6.12	5.5	-	-	-	-	-	-	-	
Follow-up Hdwy	4.4	4	3.3	3.518	4	3.345	2.425	-	-	2.353	-	-	
Pot Cap-1 Maneuver	145	227	745	222	245	459	1141	-	-	820	-	-	
Stage 1	516	647	-	449	460	-	-	-	-	-	-	-	
Stage 2	314	434	-	679	647	-	-	-	-	-	-	-	
Platoon blocked, %								-	-		-	-	
Mov Cap-1 Maneuver	· 131	220	745	215	237	459	1141	-	-	820	-	-	
Mov Cap-2 Maneuver	· 131	220	-	215	237	-	-	-	-	-	-	-	
Stage 1	513	631	-	446	457	-	-	-	-	-	-	-	
Stage 2	289	431	-	658	631	-	-	-	-	-	-	-	

Approach	EB	WB	NB	SB	
HCM Control Delay, s	19.7	31.9	0	0.5	
HCM LOS	С	D			

Minor Lane/Major Mvmt	NBL	NBT	NBR	EBLn1\	VBLn1	SBL	SBT	SBR
Capacity (veh/h)	1141	-	-	252	251	820	-	-
HCM Lane V/C Ratio	0.004	-	-	0.025	0.479	0.021	-	-
HCM Control Delay (s)	8.2	0	-	19.7	31.9	9.5	0	-
HCM Lane LOS	А	А	-	С	D	А	А	-
HCM 95th %tile Q(veh)	0	-	-	0.1	2.4	0.1	-	-

Queues 5: Wellington Rd & Fairview Ave

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		-				
Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
Lane Group Flow (vph)	291	344	419	418	192	194
v/c Ratio	0.50	0.17	0.40	0.40	0.34	0.12
Control Delay	9.5	5.9	18.4	3.1	23.3	0.2
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	9.5	5.9	18.4	3.1	23.3	0.2
Queue Length 50th (ft)	42	24	61	16	32	0
Queue Length 95th (ft)	92	47	106	50	53	0
Internal Link Dist (ft)		1061	854		222	
Turn Bay Length (ft)	225			235		
Base Capacity (vph)	590	2030	1060	1193	978	1583
Starvation Cap Reductn	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0
Reduced v/c Ratio	0.49	0.17	0.40	0.35	0.20	0.12
Intersection Summary						

	∕	-	+	•	1	-			
Movement	EBL	EBT	WBT	WBR	SBL	SBR			
Lane Configurations	ሻ	* *	44	1	ሻሻ	1			
Traffic Volume (vph)	276	327	398	397	182	184			
Future Volume (vph)	276	327	398	397	182	184			
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900			
Total Lost time (s)	7.1	7.1	7.1	6.9	6.9	4.0			
Lane Util. Factor	1.00	0.95	0.95	1.00	0.97	1.00			
Frt	1.00	1.00	1.00	0.85	1.00	0.85			
Flt Protected	0.95	1.00	1.00	1.00	0.95	1.00			
Satd. Flow (prot)	1719	3374	3438	1553	3433	1583			
Flt Permitted	0.37	1.00	1.00	1.00	0.95	1.00			
Satd. Flow (perm)	663	3374	3438	1553	3433	1583			
Peak-hour factor, PHF	0.95	0.95	0.95	0.95	0.95	0.95			
Adj. Flow (vph)	291	344	419	418	192	194			
RTOR Reduction (vph)	0	0	0	161	0	0			
Lane Group Flow (vph)	291	344	419	257	192	194			
Heavy Vehicles (%)	5%	7%	5%	4%	2%	2%			
Turn Type	pm+pt	NA	NA	pm+ov	Prot	Free			
Protected Phases	5	2	6	3	3				
Permitted Phases	2			6		Free			
Actuated Green, G (s)	36.1	36.1	18.5	28.4	9.9	60.0			
Effective Green, g (s)	36.1	36.1	18.5	28.4	9.9	60.0			
Actuated g/C Ratio	0.60	0.60	0.31	0.47	0.17	1.00			
Clearance Time (s)	7.1	7.1	7.1	6.9	6.9				
Vehicle Extension (s)	3.5	4.0	4.0	3.5	3.5				
Lane Grp Cap (vph)	583	2030	1060	735	566	1583			
v/s Ratio Prot	c0.09	0.10	0.12	c0.06	0.06				
v/s Ratio Perm	c0.21			0.11		0.12			
v/c Ratio	0.50	0.17	0.40	0.35	0.34	0.12			
Uniform Delay, d1	6.3	5.3	16.3	10.0	22.2	0.0			
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00			
Incremental Delay, d2	0.8	0.2	1.1	0.3	0.4	0.2			
Delay (s)	7.1	5.5	17.4	10.3	22.6	0.2			
Level of Service	A	А	В	В	С	А			
Approach Delay (s)		6.2	13.9		11.3				
Approach LOS		А	В		В				
Intersection Summary									
HCM 2000 Control Delav			10.7	H	CM 2000	Level of Servi	се	B	
HCM 2000 Volume to Capa	acity ratio		0.52						
Actuated Cycle Length (s)	,		60.0	Si	um of lost	t time (s)	2	1.1	
Intersection Capacity Utiliz	ation		51.5%	IC	U Level o	of Service		А	
Analysis Period (min)			15						
c Critical Lane Group									

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Movement	EBL	EBT	WBT	WBR	SBL	SBR	
Lane Configurations	3	**	**	1	ካካ	1	
Traffic Volume (veh/h)	276	327	398	397	182	184	
Future Volume (veh/h)	276	327	398	397	182	184	
Number	5	2	6	16	3	18	
Initial O (Ob), veh	0	0	0	0	0	0	
Ped-Bike Adi(A pbT)	1.00			1.00	1.00	1.00	
Parking Bus, Adi	1.00	1.00	1.00	1.00	1.00	1.00	
Adi Sat Flow, veh/h/ln	1810	1776	1810	1827	1863	1863	
Adi Flow Rate, veh/h	291	344	419	418	192	0	
Adi No. of Lanes	1	2	2	1	2	1	
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95	
Percent Heavy Veh. %	5	7	5	4	2	2	
Cap, veh/h	574	2155	1221	750	440	203	
Arrive On Green	0.17	0.64	0.36	0.36	0.13	0.00	
Sat Flow, veh/h	1723	3463	3529	1553	3442	1583	
Grp Volume(v) veh/h	291	344	419	418	192	0	
Grp Sat Flow(s) veh/h/ln	1723	1687	1719	1553	1721	1583	
O Serve(a, s), s	5.4	2.5	5.4	11 4	31	0.0	
Cycle O Clear(q, c) s	5.4	2.5	5.4	11.1	3.1	0.0	
Prop In Lane	1 00	2.0	0.1	1 00	1 00	1 00	
Lane Grp Cap(c) veh/h	574	2155	1221	750	440	203	
V/C Ratio(X)	0.51	0.16	0.34	0.56	0.44	0.00	
Avail Cap(c, a) veh/h	602	2155	1221	750	981	451	
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	
Upstream Filter(I)	1.00	1.00	1.00	1.00	1.00	0.00	
Uniform Delay (d), s/veh	8.0	4.4	14.2	11.0	24.2	0.0	
Incr Delay (d2), s/veh	0.8	0.2	0.8	3.0	0.8	0.0	
Initial O Delay(d3), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	
%ile BackOfQ(50%).veh/ln	2.6	1.2	2.6	5.5	1.5	0.0	
LnGrp Delav(d).s/veh	8.9	4.5	15.0	14.0	25.0	0.0	
LnGrp LOS	A	A	B	В	C	5.0	
Approach Vol. veh/h		635	837		192		
Approach Delay s/veh		6.5	14.5		25.0		
Approach LOS		A	B		<u>C</u>		
Timer	1	2	2	1	5	6	7 8
Assigned Dhs	1	2	5	4	5	6	<u> </u>
Assigned Fils					ت 170	20 /	0
Chapter Derived (V, Pe)		40.4 7 1			17.U 7.1	20.4 7.1	6.0
May Croop Sotting (Cmay)		7.1			10.0	10.0	0.7
wax Green Setting (GridX), S Max O Clear Time (a. c. (1), c.		20.7 1 E			10.9	10.9	5.1
(y_{1}, y_{2}) (real time (y_t+1), S		4.0			1.4	13.4	0.6
		ა.ა			0.4	0.0	0.0
Intersection Summary			40.4				
HCM 2010 Ctrl Delay			12.6				
HCM 2010 LOS			В				

Intersection						
Int Delay, s/veh	0.6					
Movomont	FRI	FRD	MRI	NRT	CRT	SBD
IVIOVEITIETIL	LDL	LDK	NDL	NDT	301	JDK
Lane Configurations	- Y			-4†	4	
Traffic Vol, veh/h	17	8	18	564	281	25
Future Vol, veh/h	17	8	18	564	281	25
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	50	-	-	-
Veh in Median Storage	,# 0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	18	9	20	613	305	27

Major/Minor	Minor2		Major1	Maje	or2		
Conflicting Flow All	666	319	332	0	-	0	
Stage 1	319	-	-	-	-	-	
Stage 2	347	-	-	-	-	-	
Critical Hdwy	6.63	6.23	4.13	-	-	-	
Critical Hdwy Stg 1	5.43	-	-	-	-	-	
Critical Hdwy Stg 2	5.83	-	-	-	-	-	
Follow-up Hdwy	3.519	3.319	2.219	-	-	-	
Pot Cap-1 Maneuver	408	721	1226	-	-	-	
Stage 1	736	-	-	-	-	-	
Stage 2	688	-	-	-	-	-	
Platoon blocked, %				-	-	-	
Mov Cap-1 Maneuver	398	721	1226	-	-	-	
Mov Cap-2 Maneuver	398	-	-	-	-	-	
Stage 1	718	-	-	-	-	-	
Stage 2	688	-	-	-	-	-	

Approach	EB	NB	SB	
HCM Control Delay, s	13.2	0.3	0	
HCM LOS	В			

Minor Lane/Major Mvmt	NBL	NBT EBL	n1 SBT	SBR
Capacity (veh/h)	1226	- 4	55 -	-
HCM Lane V/C Ratio	0.016	- 0.0	- 86	-
HCM Control Delay (s)	8	0.1 13	.2 -	-
HCM Lane LOS	А	А	В -	-
HCM 95th %tile Q(veh)	0	- (.2 -	-

Intersection Int Delay, s/veh 0.6 Movement EBL EBR NBL NBT SBT SBR Y Lane Configurations ŧ Ъ 19 270 Traffic Vol, veh/h 19 0 563 18 Future Vol, veh/h 19 19 0 563 270 18 Conflicting Peds, #/hr 0 0 0 0 0 0 Sign Control Stop Stop Free Free Free Free **RT** Channelized -None -None -None Storage Length 0 -----Veh in Median Storage, # 0 --0 0 -Grade, % 0 0 0 ---Peak Hour Factor 92 92 92 92 92 92 Heavy Vehicles, % 2 2 2 2 2 2 Mvmt Flow 21 21 0 612 293 20

Major/Minor	Minor2	Ν	1ajor1	Ma	ajor2			
Conflicting Flow All	915	303	-	0	-	0		
Stage 1	303	-	-	-	-	-		
Stage 2	612	-	-	-	-	-		
Critical Hdwy	6.42	6.22	-	-	-	-		
Critical Hdwy Stg 1	5.42	-	-	-	-	-		
Critical Hdwy Stg 2	5.42	-	-	-	-	-		
Follow-up Hdwy	3.518	3.318	-	-	-	-		
Pot Cap-1 Maneuver	303	737	0	-	-	-		
Stage 1	749	-	0	-	-	-		
Stage 2	541	-	0	-	-	-		
Platoon blocked, %				-	-	-		
Mov Cap-1 Maneuver	303	737	-	-	-	-		
Mov Cap-2 Maneuver	303	-	-	-	-	-		
Stage 1	749	-	-	-	-	-		
Stage 2	541	-	-	-	-	-		
Approach	EB		NB		SB		 	
HCM Control Delay, s	14.3		0		0			

HCM LOS B

Minor Lane/Major Mvmt	NBT EBLn1	SBT	SBR
Capacity (veh/h)	- 429	-	-
HCM Lane V/C Ratio	- 0.096	-	-
HCM Control Delay (s)	- 14.3	-	-
HCM Lane LOS	- B	-	-
HCM 95th %tile Q(veh)	- 0.3	-	-

Intersection						
Int Delay, s/veh	2.6					
Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		र्स	¢,		Y	
Traffic Vol, veh/h	0	5	5	40	20	1
Future Vol, veh/h	0	5	5	40	20	1
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	0	-
Veh in Median Storage,	# -	0	0	-	0	-
Grade, %	-	0	0	-	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	0	5	5	43	22	1

Major/Minor	Major1	Ν	/lajor2	[Minor2		
Conflicting Flow All	48	0	-	0	32	27	
Stage 1	-	-	-	-	27	-	
Stage 2	-	-	-	-	5	-	
Critical Hdwy	4.12	-	-	-	6.42	6.22	
Critical Hdwy Stg 1	-	-	-	-	5.42	-	
Critical Hdwy Stg 2	-	-	-	-	5.42	-	
Follow-up Hdwy	2.218	-	-	-	3.518	3.318	
Pot Cap-1 Maneuver	1559	-	-	-	982	1048	
Stage 1	-	-	-	-	996	-	
Stage 2	-	-	-	-	1018	-	
Platoon blocked, %		-	-	-			
Mov Cap-1 Maneuver	r 1559	-	-	-	982	1048	
Mov Cap-2 Maneuver	r -	-	-	-	982	-	
Stage 1	-	-	-	-	996	-	
Stage 2	-	-	-	-	1018	-	
Approach	EB		WB		SB		
HCM Control Delay, s	s 0		0		8.7		
HCM LOS					А		
Minor Lane/Major Mv	mt	EBL	EBT	WBT	WBR	SBLn1	
Capacity (veh/h)		1559	-	-	-	985	
HCM Lane V/C Ratio		-	-	-	-	0.023	
HCM Control Delay (s	5)	0	-	-	-	8.7	
HCM Lane LOS		А	-	-	-	А	
HCM 95th %tile Q(ve	h)	0	-	-	-	0.1	

Intersection

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		\$			\$		1	et 👘			\$	
Traffic Vol, veh/h	16	2	156	0	0	5	181	426	6	3	548	49
Future Vol, veh/h	16	2	156	0	0	5	181	426	6	3	548	49
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None									
Storage Length	-	-	-	-	-	-	280	-	-	-	-	-
Veh in Median Storage,	# -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	100	100	100	100	100	100	100	100	100	100	100	100
Heavy Vehicles, %	0	0	1	0	0	20	1	2	0	0	2	0
Mvmt Flow	16	2	156	0	0	5	181	426	6	3	548	49

Major/Minor	Minor2		ľ	Minor1			Major1			Μ	ajor2			
Conflicting Flow All	1373	1373	573	1449	1394	429	597	0	()	432	0	0	
Stage 1	579	579	-	791	791	-	-	-		-	-	-	-	
Stage 2	794	794	-	658	603	-	-	-		-	-	-	-	
Critical Hdwy	7.1	6.5	6.21	7.1	6.5	6.4	4.11	-		-	4.1	-	-	
Critical Hdwy Stg 1	6.1	5.5	-	6.1	5.5	-	-	-		-	-	-	-	
Critical Hdwy Stg 2	6.1	5.5	-	6.1	5.5	-	-	-		-	-	-	-	
Follow-up Hdwy	3.5	4	3.309	3.5	4	3.48	2.209	-		-	2.2	-	-	
Pot Cap-1 Maneuver	124	147	521	110	143	589	985	-		-	1138	-	-	
Stage 1	504	504	-	386	404	-	-	-		-	-	-	-	
Stage 2	384	403	-	457	492	-	-	-		-	-	-	-	
Platoon blocked, %								-		-		-	-	
Mov Cap-1 Maneuver	105	120	521	65	116	589	985	-		-	1138	-	-	
Mov Cap-2 Maneuver	105	120	-	65	116	-	-	-		-	-	-	-	
Stage 1	411	502	-	315	330	-	-	-		-	-	-	-	
Stage 2	311	329	-	318	490	-	-	-		-	-	-	-	

Approach	EB	WB	NB	SB	
HCM Control Delay, s	23	11.2	2.8	0	
HCM LOS	С	В			

Minor Lane/Major Mvmt	NBL	NBT	NBR	EBLn1V	VBLn1	SBL	SBT	SBR
Capacity (veh/h)	985	-	-	371	589	1138	-	-
HCM Lane V/C Ratio	0.184	-	-	0.469	0.008	0.003	-	-
HCM Control Delay (s)	9.5	-	-	23	11.2	8.2	0	-
HCM Lane LOS	А	-	-	С	В	А	А	-
HCM 95th %tile Q(veh)	0.7	-	-	2.4	0	0	-	-

Queues	
2: Fairview Ave & Manassas PD Driveway/Signal Hill Rd	

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Lane Group	EBT	WBT	WBR	NBL	NBT	SBL	SBT
Lane Group Flow (vph)	22	50	217	2	446	275	463
v/c Ratio	0.10	0.21	0.54	0.00	0.72	0.50	0.42
Control Delay	36.9	34.9	11.0	9.0	29.5	10.9	12.9
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	36.9	34.9	11.0	9.0	29.5	10.9	12.9
Queue Length 50th (ft)	8	17	0	0	141	34	65
Queue Length 95th (ft)	36	61	63	3	330	123	312
Internal Link Dist (ft)	120	611			159		588
Turn Bay Length (ft)			110			260	
Base Capacity (vph)	634	613	703	670	900	559	1092
Starvation Cap Reductn	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0
Reduced v/c Ratio	0.03	0.08	0.31	0.00	0.50	0.49	0.42
Intersection Summary							

HCM Signalized Intersection Capacity Analysis 2: Fairview Ave & Manassas PD Driveway/Signal Hill Rd

Timing Plan: EX PM

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			र्स	1	7	ĥ		7	ĥ	
Traffic Volume (vph)	14	7	0	46	2	206	2	384	40	261	435	5
Future Volume (vph)	14	7	0	46	2	206	2	384	40	261	435	5
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)		6.3			6.1	6.1	6.5	6.5		6.5	6.5	
Lane Util. Factor		1.00			1.00	1.00	1.00	1.00		1.00	1.00	
Frt		1.00			1.00	0.85	1.00	0.99		1.00	1.00	
Flt Protected		0.97			0.95	1.00	0.95	1.00		0.95	1.00	
Satd. Flow (prot)		1837			1762	1615	1805	1840		1770	1860	
Flt Permitted		0.97			0.95	1.00	0.50	1.00		0.27	1.00	
Satd. Flow (perm)		1837			1762	1615	944	1840		502	1860	
Peak-hour factor, PHF	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Adj. Flow (vph)	15	7	0	48	2	217	2	404	42	275	458	5
RTOR Reduction (vph)	0	0	0	0	0	191	0	3	0	0	0	0
Lane Group Flow (vph)	0	22	0	0	50	26	2	443	0	275	463	0
Heavy Vehicles (%)	0%	0%	0%	3%	0%	0%	0%	2%	0%	2%	2%	0%
Turn Type	Split	NA		Split	NA	Perm	pm+pt	NA		pm+pt	NA	
Protected Phases	4	4		3	3		5	2		1	6	
Permitted Phases						3	2			6		
Actuated Green, G (s)		2.6			9.6	9.6	31.0	29.8		50.0	42.3	
Effective Green, g (s)		2.6			9.6	9.6	31.0	29.8		50.0	42.3	
Actuated g/C Ratio		0.03			0.12	0.12	0.38	0.37		0.62	0.52	
Clearance Time (s)		6.3			6.1	6.1	6.5	6.5		6.5	6.5	
Vehicle Extension (s)		3.0			4.0	4.0	3.0	5.0		3.0	5.0	
Lane Grp Cap (vph)		58			208	191	373	676		523	970	
v/s Ratio Prot		c0.01			c0.03		0.00	c0.24		c0.09	0.25	
v/s Ratio Perm						0.02	0.00			0.23		
v/c Ratio		0.38			0.24	0.13	0.01	0.66		0.53	0.48	
Uniform Delay, d1		38.5			32.4	32.0	15.5	21.4		9.5	12.4	
Progression Factor		1.00			1.00	1.00	1.00	1.00		1.00	1.00	
Incremental Delay, d2		4.1			0.8	0.4	0.0	3.1		1.0	0.8	
Delay (s)		42.6			33.3	32.5	15.5	24.5		10.5	13.1	
Level of Service		D			С	С	В	С		В	В	
Approach Delay (s)		42.6			32.6			24.4			12.2	
Approach LOS		D			С			С			В	
Intersection Summary												
HCM 2000 Control Delay			20.0	Н	CM 2000	Level of	Service		С			
HCM 2000 Volume to Capacity	ratio		0.55									
Actuated Cycle Length (s)			81.1	S	um of los	t time (s)			25.4			
Intersection Capacity Utilization	۱		61.0%	IC	CU Level	of Service	e		В			
Analysis Period (min)			15									
c Critical Lane Group												

HCM 2010 analysis expects strict NEMA phasing.

Intersection

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		÷			¢			÷			¢	
Traffic Vol, veh/h	6	0	15	1	0	0	24	399	0	1	488	14
Future Vol, veh/h	6	0	15	1	0	0	24	399	0	1	488	14
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control S	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage, #	# -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	96	96	96	96	96	96	96	96	96	96	96	96
Heavy Vehicles, %	0	0	0	0	0	0	0	1	0	0	2	0
Mvmt Flow	6	0	16	1	0	0	25	416	0	1	508	15

Major/Minor	Minor2		Ν	1inor1		ſ	Major1		Ν	/lajor2			
Conflicting Flow All	984	984	516	992	991	416	523	0	0	416	0	0	
Stage 1	518	518	-	466	466	-	-	-	-	-	-	-	
Stage 2	466	466	-	526	525	-	-	-	-	-	-	-	
Critical Hdwy	7.1	6.5	6.2	7.1	6.5	6.2	4.1	-	-	4.1	-	-	
Critical Hdwy Stg 1	6.1	5.5	-	6.1	5.5	-	-	-	-	-	-	-	
Critical Hdwy Stg 2	6.1	5.5	-	6.1	5.5	-	-	-	-	-	-	-	
Follow-up Hdwy	3.5	4	3.3	3.5	4	3.3	2.2	-	-	2.2	-	-	
Pot Cap-1 Maneuver	229	250	563	227	248	641	1054	-	-	1154	-	-	
Stage 1	544	536	-	581	566	-	-	-	-	-	-	-	
Stage 2	581	566	-	539	533	-	-	-	-	-	-	-	
Platoon blocked, %								-	-		-	-	
Mov Cap-1 Maneuver	r 224	242	563	215	240	641	1054	-	-	1154	-	-	
Mov Cap-2 Maneuver	r 224	242	-	215	240	-	-	-	-	-	-	-	
Stage 1	527	535	-	563	548	-	-	-	-	-	-	-	
Stage 2	563	548	-	524	532	-	-	-	-	-	-	-	

Approach	EB	WB	NB	SB	
HCM Control Delay, s	14.7	21.8	0.5	0	
HCM LOS	В	С			

Minor Lane/Major Mvmt	NBL	NBT	NBR	EBLn1\	WBLn1	SBL	SBT	SBR	
Capacity (veh/h)	1054	-	-	393	215	1154	-	-	
HCM Lane V/C Ratio	0.024	-	-	0.056	0.005	0.001	-	-	
HCM Control Delay (s)	8.5	0	-	14.7	21.8	8.1	0	-	
HCM Lane LOS	А	А	-	В	С	А	А	-	
HCM 95th %tile Q(veh)	0.1	-	-	0.2	0	0	-	-	

Intersection

		EDT			WDT		NIDI	NDT			ODT	
Movement E	BL	FRI	EBR	WBL	WBI	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			- 44			4	
Traffic Vol, veh/h	3	0	3	150	2	11	7	413	159	22	480	4
Future Vol, veh/h	3	0	3	150	2	11	7	413	159	22	480	4
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control St	top	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	96	96	96	96	96	96	96	96	96	96	96	96
Heavy Vehicles, %	0	0	0	3	0	0	0	1	3	0	2	0
Mvmt Flow	3	0	3	156	2	11	7	430	166	23	500	4

Major/Minor	Minor2		[Vinor1		N	Major1		Ν	lajor2			
Conflicting Flow All	1082	1158	502	1077	1077	513	504	0	0	596	0	0	
Stage 1	548	548	-	527	527	-	-	-	-	-	-	-	
Stage 2	534	610	-	550	550	-	-	-	-	-	-	-	
Critical Hdwy	7.1	6.5	6.2	7.13	6.5	6.2	4.1	-	-	4.1	-	-	
Critical Hdwy Stg 1	6.1	5.5	-	6.13	5.5	-	-	-	-	-	-	-	
Critical Hdwy Stg 2	6.1	5.5	-	6.13	5.5	-	-	-	-	-	-	-	
Follow-up Hdwy	3.5	4	3.3	3.527	4	3.3	2.2	-	-	2.2	-	-	
Pot Cap-1 Maneuver	197	198	573	196	221	565	1071	-	-	990	-	-	
Stage 1	524	520	-	533	532	-	-	-	-	-	-	-	
Stage 2	534	488	-	518	519	-	-	-	-	-	-	-	
Platoon blocked, %								-	-		-	-	
Mov Cap-1 Maneuver	⁻ 185	190	573	189	212	565	1071	-	-	990	-	-	
Mov Cap-2 Maneuver	⁻ 185	190	-	189	212	-	-	-	-	-	-	-	
Stage 1	519	503	-	528	527	-	-	-	-	-	-	-	
Stage 2	516	483	-	499	502	-	-	-	-	-	-	-	

Approach	EB	WB	NB	SB	
HCM Control Delay, s	18.2	80.8	0.1	0.4	
HCM LOS	С	F			

Minor Lane/Major Mvmt	NBL	NBT	NBR	EBLn1\	VBLn1	SBL	SBT	SBR	
Capacity (veh/h)	1071	-	-	280	198	990	-	-	
HCM Lane V/C Ratio	0.007	-	-	0.022	0.858	0.023	-	-	
HCM Control Delay (s)	8.4	0	-	18.2	80.8	8.7	0	-	
HCM Lane LOS	А	А	-	С	F	А	А	-	
HCM 95th %tile Q(veh)	0	-	-	0.1	6.4	0.1	-	-	

Queues 5: Wellington Rd & Fairview Ave

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Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
Lane Group Flow (vph)	288	570	491	306	342	313
v/c Ratio	0.49	0.25	0.35	0.26	0.53	0.20
Control Delay	9.9	7.0	17.8	1.7	30.2	0.3
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	9.9	7.0	17.8	1.7	30.2	0.3
Queue Length 50th (ft)	53	54	83	7	74	0
Queue Length 95th (ft)	105	93	134	29	105	0
Internal Link Dist (ft)		1061	854		222	
Turn Bay Length (ft)	225			235		
Base Capacity (vph)	593	2245	1388	1413	1298	1568
Starvation Cap Reductn	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0
Reduced v/c Ratio	0.49	0.25	0.35	0.22	0.26	0.20
Intersection Summary						

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Movement	EBL	EBT	WBT	WBR	SBL	SBR		
Lane Configurations	5	^	44	1	ሻሻ	1		
Traffic Volume (vph)	279	553	476	297	332	304		
Future Volume (vph)	279	553	476	297	332	304		
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900		
Total Lost time (s)	7.1	7.1	7.1	6.9	6.9	4.0		
Lane Util. Factor	1.00	0.95	0.95	1.00	0.97	1.00		
Frt	1.00	1.00	1.00	0.85	1.00	0.85		
Flt Protected	0.95	1.00	1.00	1.00	0.95	1.00		
Satd. Flow (prot)	1770	3574	3539	1599	3467	1568		
Flt Permitted	0.37	1.00	1.00	1.00	0.95	1.00		
Satd. Flow (perm)	696	3574	3539	1599	3467	1568		
Peak-hour factor, PHF	0.97	0.97	0.97	0.97	0.97	0.97		
Adj. Flow (vph)	288	570	491	306	342	313		
RTOR Reduction (vph)	0	0	0	109	0	0		
Lane Group Flow (vph)	288	570	491	197	342	313		
Heavy Vehicles (%)	2%	1%	2%	1%	1%	3%		
Turn Type	pm+pt	NA	NA	pm+ov	Prot	Free		
Protected Phases	5	2	6	3	3			
Permitted Phases	2			6		Free		
Actuated Green, G (s)	47.1	47.1	29.4	43.3	13.9	75.0		
Effective Green, g (s)	47.1	47.1	29.4	43.3	13.9	75.0		
Actuated g/C Ratio	0.63	0.63	0.39	0.58	0.19	1.00		
Clearance Time (s)	7.1	7.1	7.1	6.9	6.9			
Vehicle Extension (s)	3.5	4.0	4.0	3.5	3.5			
Lane Grp Cap (vph)	588	2244	1387	923	642	1568		
v/s Ratio Prot	c0.07	0.16	0.14	0.04	c0.10			
v/s Ratio Perm	c0.24			0.08		0.20		
v/c Ratio	0.49	0.25	0.35	0.21	0.53	0.20		
Uniform Delay, d1	6.8	6.2	16.1	7.6	27.6	0.0		
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00		
Incremental Delay, d2	0.8	0.3	0.7	0.1	1.0	0.3		
Delay (s)	7.5	6.4	16.8	7.8	28.6	0.3		
Level of Service	А	А	В	А	С	A		
Approach Delay (s)		6.8	13.3		15.1			
Approach LOS		А	В		В			
Intersection Summary								
HCM 2000 Control Delay			11.4	Н	CM 2000	Level of Servic	e	
HCM 2000 Volume to Capa	city ratio		0.54					
Actuated Cycle Length (s)			75.0	S	um of lost	t time (s)		
Intersection Capacity Utiliza	tion		55.7%	IC	CU Level of	of Service		
Analysis Period (min)			15					
c Critical Lane Group								

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Movement	EBL	EBT	WBT	WBR	SBL	SBR	
Lane Configurations	5	**	**	1	ካካ	1	
Traffic Volume (veh/h)	279	553	476	297	332	304	
Future Volume (veh/h)	279	553	476	297	332	304	
Number	5	2	6	16	3	18	
Initial O (Ob), veh	0	0	0	0	0	0	
Ped-Bike Adi(A pbT)	1.00			1.00	1.00	1.00	
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	
Adj Sat Flow, veh/h/ln	1863	1881	1863	1881	1881	1845	
Adj Flow Rate, veh/h	288	570	491	306	342	0	
Adj No. of Lanes	1	2	2	1	2	1	
Peak Hour Factor	0.97	0.97	0.97	0.97	0.97	0.97	
Percent Heavy Veh, %	2	1	2	1	1	3	
Cap, veh/h	574	2407	1577	936	487	220	
Arrive On Green	0.13	0.67	0.45	0.45	0.14	0.00	
Sat Flow, veh/h	1774	3668	3632	1599	3476	1568	
Grp Volume(v), veh/h	288	570	491	306	342	0	
Grp Sat Flow(s),veh/h/ln	1774	1787	1770	1599	1738	1568	
Q Serve(g_s), s	5.7	4.6	6.7	7.4	7.0	0.0	
Cycle Q Clear(g_c), s	5.7	4.6	6.7	7.4	7.0	0.0	
Prop In Lane	1.00			1.00	1.00	1.00	
Lane Grp Cap(c), veh/h	574	2407	1577	936	487	220	
V/C Ratio(X)	0.50	0.24	0.31	0.33	0.70	0.00	
Avail Cap(c_a), veh/h	596	2407	1577	936	1302	587	
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	
Upstream Filter(I)	1.00	1.00	1.00	1.00	1.00	0.00	
Uniform Delay (d), s/veh	7.8	4.8	13.4	8.0	30.8	0.0	
Incr Delay (d2), s/veh	0.8	0.2	0.5	0.9	2.2	0.0	
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	
%ile BackOfQ(50%),veh/In	2.9	2.3	3.4	3.5	3.5	0.0	
LnGrp Delay(d),s/veh	8.7	5.0	13.9	8.9	33.0	0.0	
LnGrp LOS	Α	Α	В	Α	С		
Approach Vol, veh/h		858	797		342		
Approach Delay, s/veh		6.2	12.0		33.0		
Approach LOS		А	В		С		
Timer	1	2	3	4	5	6	7 8
Assigned Phs		2			5	6	8
Phs Duration (G+Y+Rc), s		57.6			17.1	40.5	17.4
Change Period (Y+Rc), s		7.1			7.1	7.1	6.9
Max Green Setting (Gmax), s		32.9			10.9	14.9	28.1
Max Q Clear Time (g_c+l1), s		6.6			7.7	9.4	9.0
Green Ext Time (p_c), s		6.0			0.3	2.8	1.5
Intersection Summary							
HCM 2010 Ctrl Delay			13.1				
HCM 2010 LOS			В				

lutana a stian						
Intersection						
Int Delay, s/veh	0.3					
N /				NDT	CDT	
Novement	FRL	FRK	NRL	NRT	SRI	SBK
Lane Configurations	۰¥			-4↑	- Þ	
Traffic Vol, veh/h	12	4	8	413	492	13
Future Vol, veh/h	12	4	8	413	492	13
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	50	-	-	-
Veh in Median Storage,	# 0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mymt Flow	13	4	9	449	535	14
	10				000	

Major/Minor	Minor2	l	Major1	Ma	jor2		
Conflicting Flow All	785	542	549	0	-	0	
Stage 1	542	-	-	-	-	-	
Stage 2	243	-	-	-	-	-	
Critical Hdwy	6.63	6.23	4.13	-	-	-	
Critical Hdwy Stg 1	5.43	-	-	-	-	-	
Critical Hdwy Stg 2	5.83	-	-	-	-	-	
Follow-up Hdwy	3.519	3.319	2.219	-	-	-	
Pot Cap-1 Maneuver	345	539	1019	-	-	-	
Stage 1	582	-	-	-	-	-	
Stage 2	775	-	-	-	-	-	
Platoon blocked, %				-	-	-	
Mov Cap-1 Maneuver	341	539	1019	-	-	-	
Mov Cap-2 Maneuver	341	-	-	-	-	-	
Stage 1	575	-	-	-	-	-	
Stage 2	775	-	-	-	-	-	

Approach	EB	NB	SB
HCM Control Delay, s	15.1	0.2	0
HCM LOS	С		

Minor Lane/Major Mvmt	NBL	NBT EBLn1	SBT	SBR
Capacity (veh/h)	1019	- 375	-	-
HCM Lane V/C Ratio	0.009	- 0.046	-	-
HCM Control Delay (s)	8.6	0 15.1	-	-
HCM Lane LOS	А	A C	-	-
HCM 95th %tile Q(veh)	0	- 0.1	-	-

Intersection

Int Delay, s/veh	0.5						
Movement	EBL	EBR	NBL	NBT	SBT	SBR	
Lane Configurations	Y			↑	- î÷		
Traffic Vol, veh/h	17	15	0	404	488	9	
Future Vol, veh/h	17	15	0	404	488	9	
Conflicting Peds, #/hr	0	0	0	0	0	0	
Sign Control	Stop	Stop	Free	Free	Free	Free	
RT Channelized	-	None	-	None	-	None	
Storage Length	0	-	-	-	-	-	
Veh in Median Storage,	# 0	-	-	0	0	-	
Grade, %	0	-	-	0	0	-	
Peak Hour Factor	92	92	92	92	92	92	
Heavy Vehicles, %	2	2	2	2	2	2	
Mvmt Flow	18	16	0	439	530	10	

Minor2	N	1ajor1	Ма	jor2		
974	535	-	0	-	0	
535	-	-	-	-	-	
439	-	-	-	-	-	
6.42	6.22	-	-	-	-	
5.42	-	-	-	-	-	
5.42	-	-	-	-	-	
3.518	3.318	-	-	-	-	
279	545	0	-	-	-	
587	-	0	-	-	-	
650	-	0	-	-	-	
			-	-	-	
279	545	-	-	-	-	
279	-	-	-	-	-	
587	-	-	-	-	-	
650	-	-	-	-	-	
	Minor2 974 535 439 6.42 5.42 5.42 3.518 279 587 650 279 279 587 650	Minor2 M 974 535 535 - 439 - 6.42 6.22 5.42 - 5.42 - 3.518 3.318 279 545 587 - 650 - 279 545 279 545 650 - 587 - 650 - 587 - 650 -	Minor2 Major1 974 535 - 535 - - 439 - - 6.42 6.22 - 5.42 - - 5.42 - - 3.518 3.318 - 279 545 0 587 - 0 650 - 0 587 - - 279 545 - 650 - 0	Minor2 Major1 Ma 974 535 - 0 535 - - - 439 - - - 6.42 6.22 - - 5.42 - - - 5.42 - - - 3.518 3.318 - - 3.518 3.318 - - 587 - 0 - 650 - 0 - 279 545 0 - 279 545 - - 279 545 - - 279 545 - - 279 545 - - 279 545 - - 587 - - - 587 - - - 650 - - -	Minor2 Major1 Major2 974 535 - 0 - 535 - - - - 439 - - - - 6.42 6.22 - - - 5.42 - - - - 5.42 - - - - 3.518 3.318 - - - 3.518 3.318 - - - 587 - 0 - - 587 0 - - - 279 545 0 - - 279 545 - 0 - - 279 545 - - - - 279 545 - - - - 587 - - - - - 587 - - - -	Minor2Major1Major2 974 535 -0-0 535 439 6.42 6.22 5.42 5.42 5.42 5.42 5.42 5.42 5.42 5.42 5.42 5.42 5.42 5.42 5.42 5.42 5.42 5.47 -0 5.77 279 545 587 587 587 587 587 <

Approach	EB	NB	SB
HCM Control Delay, s	16	0	0
HCM LOS	С		

Minor Lane/Major Mvmt	NBT	EBLn1	SBT	SBR		
Capacity (veh/h)	-	362	-	-		
HCM Lane V/C Ratio	-	0.096	-	-		
HCM Control Delay (s)	-	16	-	-		
HCM Lane LOS	-	С	-	-		
HCM 95th %tile Q(veh)	-	0.3	-	-		
Intersection						
------------------------	------	------	------	------	------	------
Int Delay, s/veh	2.4					
-						
Movement	EBL	FRI	WBI	WBR	SBL	SBR
Lane Configurations		- सी	- î>		۰¥	
Traffic Vol, veh/h	0	5	10	28	15	1
Future Vol, veh/h	0	5	10	28	15	1
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	0	-
Veh in Median Storage,	# -	0	0	-	0	-
Grade. %	-	0	0	-	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles %	2	2	2	2	2	2
Mymt Flow	0	5	11	30	16	1
	0	5		50	10	

Major/Minor	Major1	Ν	1ajor2	1	Minor2	
Conflicting Flow All	41	0	-	0	31	26
Stage 1	-	-	-	-	26	-
Stage 2	-	-	-	-	5	-
Critical Hdwy	4.12	-	-	-	6.42	6.22
Critical Hdwy Stg 1	-	-	-	-	5.42	-
Critical Hdwy Stg 2	-	-	-	-	5.42	-
Follow-up Hdwy	2.218	-	-	-	3.518	3.318
Pot Cap-1 Maneuver	1568	-	-	-	983	1050
Stage 1	-	-	-	-	997	-
Stage 2	-	-	-	-	1018	-
Platoon blocked, %		-	-	-		
Mov Cap-1 Maneuver	1568	-	-	-	983	1050
Mov Cap-2 Maneuver	· -	-	-	-	983	-
Stage 1	-	-	-	-	997	-
Stage 2	-	-	-	-	1018	-
Approach	EB		WB		SB	
HCM Control Delay, s	0		0		8.7	
HCM LOS					А	
Minor Lane/Major Mvr	nt	EBL	EBT	WBT	WBR 3	SBLn1
Capacity (veh/h)		1568	-	-	-	987
HCM Lane V/C Ratio		-	-	-	-	0.018
HCM Control Delay (s	5)	0	-	-	-	8.7
HCM Lane LOS		А	-	-	-	А
HCM 95th %tile Q(vel	n)	0	-	-	-	0.1

H. Peer Child Care Center Site Data

10405 DUMFRIES RD

Location	10405 DUMFRIES RD	ΤΑΧ ΜΑΡ	090/ 01 00/ 57A/ /
Acct#	37760	Owner	ENHANCED REALTY LLC
Assessment	\$2,308,900	Building Count	1
Model		Legal Description	LENGTHY LEGAL
Legal Description	SEE DEED	Legal Description	

Current Value

Assessment				
Valuation Year	Improvements	Land	Total	
2022	\$1,409,900	\$899,000	\$2,308,900	

Owner of Record

Owner	ENHANCED REALTY LLC	Sale Price	\$899,000
Co-Owner		Book & Page	201810100074093/
Address	18403 FABLE DR BOYDS, MD 20841-4371	Sale Date	10/10/2018

Ownership History

Ownership History				
Owner	Sale Price	Book & Page	Sale Date	
10405 LLC	\$0	201810100074091/	10/10/2018	

Building Information

Year Built:	2019
Living Area:	10,371
Building Percent Good:	87

Building Attributes			
Field	Description		
Style:	Day Care Ctr		
Model	Commercial		
Grade	Good		

Stories:	1
Occupancy	
Exterior Wall 1	Single Siding
Exterior Wall 2	Brick
Roof Structure	Gable
Roof Cover	Asphalt
Interior Wall 1	Sheet Rock
Interior Wall 2	
Interior Floor 1	Vinyl
Interior Floor 2	Carpet
Heating Fuel	Gas
Heating Type	Typical
АС Туре	Central
Struct Class	
Bldg Use	DAY CARE MDL-94
Total Rooms	
Total Bedrms	
Total Baths	
I and E Filed?	
Location	
1st Floor Use:	
Heat/AC	HEAT/AC SPLIT
Frame Type	MASONRY
Baths/Plumbing	AVERAGE
Ceiling/Wall	SUS-CEIL/MN WL
Rooms/Prtns	AVERAGE
Wall Height	
% Comn Wall	



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Building Layout



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Extra Features

Extra Featu	ires <u>Legend</u>
No Data for E	xtra Features

Land

Land Use

 Use Code
 210

 Description
 DAY CARE MDL-94

 Zone
 B4

 Neighborhood
 DUMFRIES, SO.

 Alt Land Appr
 No

Land Line Valuation

Size (Sqr Feet) 103372 Frontage Depth Assessed Value \$899,000

Outbuildings

Outbuildings				
Code	Description	Sub Code	Sub Description	Size
PARK	PARKING SPACE			47.00 UNITS
PRKH	HANDICAP SPACE			3.00 UNITS
PLAY	PLAYGROUND			1.00

Valuation History

Assessment					
Valuation Year	Improvements	Land	Total		
2021	\$1,409,900	\$899,000	\$2,308,900		
2020	\$1,651,100	\$899,000	\$2,550,100		
2019	\$0	\$899,000	\$899,000		

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Search for Child Day Care

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The Goddard School	Facility Type:	<u>Child Day C</u>	<u>enter</u>	
10405 Dumfries Road	License Type:	<u>Two Year</u>		
MANASSAS, VA 20110	Expiration Date:	May 24, 202	24	
(703) 420-7333	Administrator:	Layla Miran	da	
🕵 Directions	Business Hours	7:00 am - 6	:00 pm, Monday - Friday	/
	Capacity:	158		
	Ages:	1 month - 1	2 years 11 months	
	Inspector:	Donna Libe	rman: (540) 359-5244	
	Current Subsidy	Provider No		
	License/Facility	ID# 1108694		
Inspection Date	<u>SHSI</u>	Complaint Related	Violations	
May 12, 2022	No	No	Yes	
Dec. 14, 2021 and Jan. 4, 2022	No	No	Yes	
June 11, 2021 and June 15, 2021	No	No	No	
Feb. 11, 2021 and Feb. 26, 2021	No	Yes	Yes	
Nov. 23, 2020	No	No	Yes	

No

No

No

No

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Thank you for visiting. How was your experience?

Disclaimer:

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Nov. 12, 2020 and Dec. 22, 2020

June 9, 2020

Jan. 28, 2020

Nov. 14, 2019

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No

No

No

No

No

No

Yes

Yes

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10023 DUMFRIES RD

Location	10023 DUMFRIES RD	ΤΑΧ ΜΑΡ	090/01 00/ 36A/ /
Acct#	34504	Owner	RUBINOFF JOANNE T TRUSTEE
Assessment	\$1,099,800	Building Count	1
Model	DAY CARE CENTER	Legal Description	COMMERCIAL
Legal Description		Legal Description	

Current Value

	Assessment		
Valuation Year	Improvements	Land	Total
2022	\$233,300	\$866,500	\$1,099,800

Owner of Record

Owner	RUBINOFF JOANNE T TRUSTEE	Sale Price	\$895,907
Co-Owner		Book & Page	1736/1793
Address	915 W FRANCIS ST	Sale Date	05/22/1990

ASPEN, CO 81611-3103

Ownership History

	Ownership History			
Owner	Sale Price Book & Page Sale Date			
LA PETITE ACADEMY INC	\$0	1606/1604	10/13/1988	

Building Information

Year Built:	19	88
Living Area:	6,8	374
Building Percent Good:	58	
	Buildi	ng Attributes
Field		Description
Style:		Day Care Ctr
Model		Commercial

Grade	Average
Stories:	1
Occupancy	
Exterior Wall 1	Brick
Exterior Wall 2	
Roof Structure	Gable
Roof Cover	Asphalt
Interior Wall 1	Sheet Rock
Interior Wall 2	
Interior Floor 1	Carpet
Interior Floor 2	Vinyl
Heating Fuel	Gas
Heating Type	FA/HW/ST
АС Туре	Central
Struct Class	
Bldg Use	DAY CARE MDL-94
Total Rooms	
Total Bedrms	00
Total Baths	0
I and E Filed?	
Location	SOUTH
1st Floor Use:	210
Heat/AC	HEAT/AC PKGS
Frame Type	MASONRY
Baths/Plumbing	TYPICAL
Ceiling/Wall	CEIL & WALLS
Rooms/Prtns	AVERAGE
Wall Height	10.00
% Comn Wall	



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Building Layout



Extra Features

Extra Features	<u>Legend</u>
No Data for Extra Features	

Land

 Use Code
 210

 Description
 DAY CARE MDL-94

 Zone
 B4

 Neighborhood
 SO. DUMFRIES

 Alt Land Appr
 No

 Category
 Son

Size (Sqr Feet) 52154 Frontage Depth Assessed Value \$866,500

Outbuildings

		Outbuildings		<u>Legend</u>
Code	Description	Sub Code	Sub Description	Size
LT5	MERC VAP/FLU			1.00 UNITS
PLAY	PLAYGROUND			1.00
PRKH	HANDICAP SPACE			3.00 UNITS
PARK	PARKING SPACE			25.00 UNITS

Valuation History

	Assessment		
Valuation Year	Improvements	Land	Total
2021	\$233,300	\$866,500	\$1,099,800
2020	\$351,100	\$866,500	\$1,217,600
2019	\$318,700	\$866,500	\$1,185,200

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Search for Child Day Care

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La Petite Academy - Dumfries Road	Facility Type:	Child Day Center
10023 Dumfries Road	License Type:	Two Year
MANASSAS, VA 20110-7950	Expiration Date:	April 23, 2023
(703) 361-6356	Administrator:	Michelle Custer
Market Street St	Business Hours:	6:30 a.m 6:30 p.m., Monday - Friday
	Capacity:	160
	Ages:	1 month - 12 years 11 months
	Inspector:	Keesha Minor: (540) 340-2672
	Current Subsidy Provider	Yes
	License/Facility ID#	524563

	Violations	Complaint Related	SHSI	Inspection Date
	Yes	Yes	No	May 20, 2022
	No	No	Yes	Feb. 14, 2022
	Yes	No	No	Feb. 14, 2022
)	No	No	No	Aug. 24, 2021
1	No	No	Yes	Feb. 26, 2021
	No	No	No	Feb. 26, 2021
Thank you for visiting.	No	No	No	Dec. 28, 2020
now was your experience	No	No	Yes	June 2, 2020
	No	No	No	June 2, 2020
	No	No	Yes	Dec. 26, 2019
_	Yes	No	No	Dec. 26, 2019
G	No	Yes	No	June 19, 2019 and June 21, 2019
	No	No	Yes	April 9, 2019
	No	No	No	April 9, 2019
	No	No	Yes	Sept. 25, 2018
	No	No	No	Sept. 25, 2018
	No	No	Yes	Feb. 8, 2018
	Yes	No	No	Feb. 8, 2018
	Yes	No	No	Aug. 28, 2017
	No	No	Yes	Aug. 28, 2017

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9511 TECHNOLOGY DR

Location	9511 TECHNOLOGY DR	ΤΑΧ ΜΑΡ	101/51 00/ 2/ /
Acct#	30034	Owner	9511 TECHNOLOGY LLC
Assessment	\$1,147,800	Building Count	1
Model	DAY CARE CENTER	Legal Description	LOT 2 DAY CARE CTR
Legal Description	MANASSAS OFFICE AND	Legal Description	RESEARCH PARK

Current Value

Assessment			
Valuation Year	Improvements	Land	Total
2022	\$510,800	\$637,000	\$1,147,800

Owner of Record

Owner	9511 TECHNOLOGY LLC	Sale Price	\$0
Co-Owner		Book & Page	2785/989
Address	4300 PRINCE WILLIAM PARKWAY WOODBRIDGE, VA 22192-5361	Sale Date	08/17/1999

Ownership History

Ownership History				
Owner	Sale Price	Book & Page	Sale Date	
LEOPOLD CHARLES W AND JACQUELINE M	\$700,000	2096/370	01/04/1994	
MCMAHON JOHN F TRUSTEE	\$0	2094/1616	12/30/1993	

Building Information

Year Built:	1988
Living Area:	6,300
Building Percent Good:	58

Building Attributes			
Field Description			
Style:	Day Care Ctr		
Model Commercial			

Grade	Average
Stories:	1
Occupancy	
Exterior Wall 1	Brick
Exterior Wall 2	
Roof Structure	Gable
Roof Cover	Asphalt
Interior Wall 1	Sheet Rock
Interior Wall 2	
Interior Floor 1	Carpet
Interior Floor 2	Vinyl
Heating Fuel	Gas
Heating Type	FA/HW/ST
АС Туре	Central
Struct Class	
Bldg Use	DAY CARE MDL-94
Total Rooms	
Total Bedrms	00
Total Baths	0
I and E Filed?	OWNER OCCUPIED
Location	
1st Floor Use:	210
Heat/AC	HEAT/AC PKGS
Frame Type	MASONRY
Baths/Plumbing	AVERAGE
Ceiling/Wall	CEIL & WALLS
Rooms/Prtns	AVERAGE
Wall Height	10.00
% Comn Wall	0.00



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Building Layout



Extra Features

Extra Features	Legend
No Data for Extra Features	

Land

Land Use		Land Line Valuation	
Use Code	210	Size (Sqr Feet) 48352	
Description	DAY CARE MDL-94	Frontage	
Zone	11	Depth	
Neighborhood	IND SCHOOL	Assessed Value \$637,000	

Alt Land Appr No Category

Outbuildings

Outbuildings				<u>Legend</u>
Code	Description	Sub Code	Sub Description	Size
FN	FENCE			300.00 L.F.
PLAY	PLAYGROUND			1.00
PARK	PARKING SPACE			19.00 UNITS
PRKH	HANDICAP SPACE			1.00 UNITS

Valuation History

Assessment				
Valuation Year	Improvements	Land	Total	
2021	\$510,800	\$637,000	\$1,147,800	
2020	\$633,900	\$637,000	\$1,270,900	
2019	\$600,100	\$637,000	\$1,237,100	

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Search for Child Day Care

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Minnieland Academy at Technology (#10)	Facility Type:	Child Day Center
9511 Technology Drive	License Type:	<u>Two Year</u>
MANASSAS, VA 20110-4150	Expiration Date:	July 2. 2022
(703) 330-9507	Administrator:	Samantha Updyke
Markov Directions	Business Hours:	7:00 a.m 5: 30 p.m., Monday - Friday
	Capacity:	130
	Ages:	Birth - 12 years 11 months
	Inspector:	Stephanie Reed: (540) 272-6558
	Current Subsidy Provider	Yes
	License/Facility ID#	533236

Inspection Date	<u>SHSI</u>	Complaint Related	Violations
June 7, 2022	No	No	No
June 7, 2022	Yes	No	No
May 20, 2021 and May 24, 2021	Yes	No	No
May 20, 2021 and May 24, 2021	No	No	No
Dec. 4, 2020 , Dec. 7, 2020 and Dec. 9, 2020	Yes	No	No
Dec. 4, 2020 , Dec. 7, 2020 and Dec. 9, 2020	No	No	No
June 22, 2020 and June 23, 2020	Yes	No	No
June 22, 2020 and June 23, 2020	No	No	No
May 19, 2020 and May 20, 2020	No	Yes	No
Aug. 23, 2019	No	No	Yes
Aug. 23, 2019	Yes	No	No
Feb. 21, 2019	No	Yes	No
Jan. 31, 2019	Yes	No	No
Jan. 31, 2019	No	No	Yes
July 11, 2018	Yes	No	No
July 11, 2018	No	No	Yes
Feb. 28, 2018	Yes	No	No
July 28, 2017	Yes	No	No
July 28, 2017	No	No	Yes
Feb. 28, 2017	No	No	Yes

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9811 GODWIN DR

Location	9811 GODWIN DR	ΤΑΧ ΜΑΡ	102/01 00/ 37A/ /
Acct#	22162	Owner	STORE MASTER FUNDING II LLC
Assessment	\$3,053,200	Building Count	1
Model		Legal Description	LENGTHY LEGAL
Legal Description	SEE DEED	Legal Description	

Current Value

Assessment			
Valuation Year	Improvements	Land	Total
2022	\$1,533,200	\$1,520,000	\$3,053,200

Owner of Record

Owner Co-Owner Address	STORE MASTER FUNDING II LLC 3520-A MAYLAND CT	Sale Price Book & Page Sale Date	\$3,500,000 201212120119675/ 12/12/2012
	RICHMOND, VA 23233-1421	Sale Date	12/12/2012

Ownership History

Ownership History			
Owner Sale Price		Book & Page	Sale Date
CHILDCARE HOLDINGS OF MANASSAS LLC	\$0	200412150212154/	12/15/2004

Building Information

Year Built:	2005	
Living Area:	14,589	
Building Percent Good:	73	
	Building Attributes	
Field	Description	
Style:	Day Care Ctr	
Model	Commercial	

Grade	Good
Stories:	1
Occupancy	
Exterior Wall 1	Brick
Exterior Wall 2	Concr/Cinder
Roof Structure	Gable
Roof Cover	Asphalt
Interior Wall 1	Sheet Rock
Interior Wall 2	
Interior Floor 1	Vinyl
Interior Floor 2	Carpet
Heating Fuel	Gas
Heating Type	FA/HW/ST
АС Туре	Central
Struct Class	
Bldg Use	DAY CARE MDL-94
Total Rooms	
Total Bedrms	
Total Baths	
I and E Filed?	I AND E FILED
Location	SOUTH
1st Floor Use:	
Heat/AC	HEAT/AC SPLIT
Frame Type	MASONRY
Baths/Plumbing	AVERAGE
Ceiling/Wall	SUS-CEIL & WL
Rooms/Prtns	AVERAGE
Wall Height	13.00
% Comn Wall	



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Building Layout



Extra Features

Extra Features	Legend
No Data for Extra Features	

Land

Land Use		Land Line Valuation	
Use Code	210	Size (Sqr Feet) 108185	
Description	DAY CARE MDL-94	Frontage	
Zone	l1	Depth	
Neighborhood	L0C7	Assessed Value \$1,520,000	

Alt Land Appr No Category

Outbuildings

Outbuildings			Legend	
Code	Description	Sub Code	Sub Description	Size
LT11	W/TRIPLE LIGHT			3.00 UNITS
FN	FENCE			900.00 L.F.
PLAY	PLAYGROUND			1.00
PARK	PARKING SPACE			49.00 UNITS
PRKH	HANDICAP SPACE			2.00 UNITS

Valuation History

Assessment			
Valuation Year	Improvements	Land	Total
2021	\$1,533,200	\$1,520,000	\$3,053,200
2020	\$1,861,300	\$1,520,000	\$3,381,300
2019	\$1,771,300	\$1,520,000	\$3,291,300

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Search for Child Day Care

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The Compass School of Manassas, LLC	Facility Type:	Child Day Center
9811 Godwin Drive	License Type:	<u>Two Year</u>
MANASSAS, VA 20110-4156	Expiration Date:	June 4, 2022
(703) 331-1303	Administrator:	Bailey Brenton
Mirections 201	Business Hours:	7:00 am - 6:00 pm, Monday - Friday
	Capacity:	248
	Ages:	1 month - 12 years 11 months
	Inspector:	Stephanie Reed: (540) 272-6558
	Current Subsidy Provider	No
	License/Facility ID#	1105640

June 2, 2022NoNoYesFeb. 23, 2022NoNoNoMay 18, 2021NoNoYesDec. 9, 2020NoNoYesJune 18, 2020NoNoNoNov. 25, 2019NoNoYesMay 31, 2019NoNoYesFeb. 21, 2019NoNoYesMay 10, 2018NoNoYes		Violations	Complaint Related	<u>SHSI</u>	Inspection Date
Feb. 23, 2022NoNoNoMay 18, 2021NoNoYesDec. 9, 2020NoNoYesJune 18, 2020NoNoNoNov. 25, 2019NoNoYesMay 31, 2019NoNoYesFeb. 21, 2019NoNoYesMay 10, 2018NoNoYes		Yes	No	No	June 2, 2022
May 18, 2021NoNoYesDec. 9, 2020NoNoYes)June 18, 2020NoNoNoINov. 25, 2019NoNoYesIMay 31, 2019NoNoYesThank you for visiting. How was your experience?Feb. 21, 2019NoNoYesMay 10, 2018NoNoYes		No	No	No	Feb. 23, 2022
Dec. 9, 2020NoNoYes>June 18, 2020NoNoNoNoNov. 25, 2019NoNoYesMay 31, 2019NoNoYesFeb. 21, 2019NoNoYesMay 10, 2018NoNoYes		Yes	No	No	May 18, 2021
June 18, 2020NoNoNoNov. 25, 2019NoNoYesMay 31, 2019NoNoYesFeb. 21, 2019NoNoYesMay 10, 2018NoNoYes	_ ×	Yes	No	No	Dec. 9, 2020
Nov. 25, 2019NoNoYesThank you for visiting. How was your experience?May 31, 2019NoNoYesHow was your experience?Feb. 21, 2019NoNoYesHow was your experience?May 10, 2018NoNoYesHow was your experience?		No	No	No	June 18, 2020
May 31, 2019NoNoYesThank you for visiting. How was your experienceFeb. 21, 2019NoNoYesMay 10, 2018NoNoYes		Yes	No	No	Nov. 25, 2019
Feb. 21, 2019 No No Yes May 10, 2018 No No Yes	Thank you for visiting. How was your experience?	Yes	No	No	May 31, 2019
May 10, 2018 No No Yes	,	Yes	No	No	Feb. 21, 2019
		Yes	No	No	May 10, 2018
Dec. 18, 2017 No No Yes		Yes	No	No	Dec. 18, 2017

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10249 HENDLEY RD

Location	10249 HENDLEY RD	ΤΑΧ ΜΑΡ	090/77 00/ A1/ /
Acct#	25121	Owner	LEOPOLD CHARLES W
Assessment	\$2,641,300	Building Count	1
Model	DAYCARE	Legal Description	PARCEL A1
Legal Description	WELLINGTON	Legal Description	COMMERCIAL

Current Value

Assessment			
Valuation Year	Improvements	Land	Total
2022	\$1,078,300	\$1,563,000	\$2,641,300

Owner of Record

Owner	LEOPOLD CHARLES W	Sale Price	\$250,000
Co-Owner		Book & Page	2838/728
Address	4300 PRINCE WILLIAM PARKWAY	Sale Date	12/29/1999
	WOODBRIDGE, VA 22192-5361		

Ownership History

Ownership History				
Owner	Sale Price	Book & Page	Sale Date	
WELLINGTON LAND COMPANY INC	\$0	2836/572	12/23/1999	

Building Information

Year Built:	2003
Living Area:	9,866
Building Percent Good:	71

Building Attributes			
Field Description			
Style:	Day Care Ctr		
Model	Commercial		
Grade	Average		

Stories:	1
Occupancy	
Exterior Wall 1	Concr/Cinder
Exterior Wall 2	
Roof Structure	Gable
Roof Cover	Asphalt
Interior Wall 1	Sheet Rock
Interior Wall 2	
Interior Floor 1	Carpet
Interior Floor 2	Vinyl
Heating Fuel	Typical
Heating Type	Heat pump
АС Туре	Central
Struct Class	
Bldg Use	DAY CARE MDL-94
Total Rooms	
Total Bedrms	00
Total Baths	0
I and E Filed?	OWNER OCCUPIED
Location	
1st Floor Use:	210
Heat/AC	HEAT/AC PKGS
Frame Type	STEEL
Baths/Plumbing	AVERAGE
Ceiling/Wall	CEIL & WALLS
Rooms/Prtns	AVERAGE
Wall Height	14.00
% Comn Wall	



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Building Layout



Extra Features

Extra Features	Legend
No Data for Extra Features	

Land

Land Use		Land Line Valua	ation	
Use Code	210	Size (Sqr Feet)	113763	
Description	DAY CARE MDL-94	Frontage		
Zone	B4	Depth		
Neighborhood	RETAIL	Assessed Value	\$1,563,000	
Alt Land Appr	No			
Category				

Outbuildings

Outbuildings				Legend
Code	Description	Sub Code	Sub Description	Size
LT5	MERC VAP/FLU			8.00 UNITS
LT6	W/DOUBLE LIGHT			1.00 UNITS
PLAY	PLAYGROUND			3.00
FN11	FENCE			84.00 L.F.
FN11	FENCE			218.00 L.F.
РККН	HANDICAP SPACE			4.00 UNITS
PART	TRUCK SPACES			2.00 UNITS
PARK	PARKING SPACE			53.00 UNITS

Valuation History

Assessment				
Valuation Year	Improvements	Land	Total	
2021	\$1,078,300	\$1,563,000	\$2,641,300	
2020	\$1,364,700	\$1,563,000	\$2,927,700	
2019	\$1,286,800	\$1,563,000	\$2,849,800	

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The Merit School of Wellington (#20)

10249 Hendley Road

(703) 369-7929 💓 Directions

MANASSAS, VA 20110-7913

Facility Type:	Child Day Center
License Type:	<u>Two Year</u>
Expiration Date:	June 3, 2024
Administrator:	Ms. Nancy Stewart
Business Hours:	6:00 a.m 6:00 p.m., Monday - Friday
Capacity:	300
Ages:	1 month - 12 years 11 months
Inspector:	Cathy Aylor: (540) 222-6352
Current Subsidy Provider	Yes
License/Facility ID#	1105285

Inspection Date	SHSI	Complaint Related	Violations	
June 8, 2022	Yes	No	No	
June 8, 2022	No	No	No	
Feb. 14, 2022	Yes	No	No	
Feb. 14, 2022	No	No	No	
May 20, 2021 and May 24, 2021	Yes	No	No	
May 20, 2021 and May 24, 2021	No	No	No	
Dec. 4, 2020 , Dec. 7, 2020 and Dec. 9, 2020	No	No	No	Th
Dec. 4, 2020 , Dec. 7, 2020 and Dec. 9, 2020	Yes	No	No	110 11
May 5, 2020 and May 6, 2020	Yes	No	No	
May 5, 2020 and May 6, 2020	No	No	No	
Dec. 16, 2019	Yes	No	No	
Dec. 16, 2019	No	No	No	
June 4, 2019	Yes	No	Yes	
June 4, 2019	No	No	No	
Nov. 29, 2018	Yes	No	No	
Nov. 29, 2018	No	No	No	
May 31, 2018	Yes	No	No	
May 31, 2018	No	No	No	
Dec. 12, 2017	Yes	No	No	
Dec. 12, 2017	No	No	No	

Disclaimen

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8485 PLANTATION LN

Location	8485 PLANTATION LN	ΤΑΧ ΜΑΡ	112/01 00/ 21A/ /
Acct#	33817	Owner	8485 PLANTATION LANE LLC
Assessment	\$2,543,300	Building Count	1
Model	DAY CARE CTR(AP	Legal Description	LENGTHY LEGAL
Legal Description	SEE DEED	Legal Description	

Current Value

Assessment					
Valuation Year	Improvements	Land	Total		
2022	\$1,313,300	\$1,230,000	\$2,543,300		

Owner of Record

Owner	8485 PLANTATION LANE LLC	Sale Price	\$2,425,000
Co-Owner		Book & Page	201108290071078/
Address	4300 PRINCE WILLIAM PKWY	Sale Date	08/29/2011
	WOODBRIDGE, VA 22192-5361		

Ownership History

Ownership History					
Owner Sale Price Book & Page Sale Date					
APPLE TREE PRE-SCHOOL INC	\$40,000	1128/845	10/06/1980		

Building Information

Year Built:	1982
Living Area:	14,304
Building Percent Good:	57

Building Attributes				
Field Description				
Style:	Day Care Ctr			
Model	Commercial			
Grade	Good			

Stories:	1
Occupancy	
Exterior Wall 1	Brick
Exterior Wall 2	Concr/Cinder
Roof Structure	Gable
Roof Cover	Asphalt
Interior Wall 1	Sheet Rock
Interior Wall 2	
Interior Floor 1	Carpet
Interior Floor 2	Vinyl
Heating Fuel	Gas
Heating Type	FA/HW/ST
АС Туре	Central
Struct Class	
Bldg Use	DAY CARE MDL-94
Total Rooms	
Total Bedrms	00
Total Baths	0
I and E Filed?	OWNER OCCUPIED
Location	
1st Floor Use:	210
Heat/AC	HEAT/AC PKGS
Frame Type	MASONRY
Baths/Plumbing	AVERAGE
Ceiling/Wall	CEIL & WALLS
Rooms/Prtns	AVERAGE
Wall Height	10.00
% Comn Wall	



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Building Layout



Extra Features

Use Code	210
Description	DAY CARE MDL-94
Zone	A1
Neighborhood	OTH RT 234
Alt Land Appr	No
Category	

Outbuildings

Outbuildings					
Code	Description	Sub Code	Sub Description	Size	
FN	FENCE			1.00 L.F.	
PLAY	PLAYGROUND			2.00	
EQMT	EQUIP SHED			192.00 S.F.	
GAZ	GAZEBO			1200.00 S.F.	
GAZ	GAZEBO			336.00 S.F.	
GAZ	GAZEBO			224.00 S.F.	
GAZ	GAZEBO			224.00 S.F.	
PARK	PARKING SPACE			47.00 UNITS	

Valuation History

Assessment					
Valuation Year	Improvements	Land	Total		
2021	\$1,313,300	\$1,230,000	\$2,543,300		
2020	\$1,585,200	\$1,230,000	\$2,815,200		
2019	\$1,510,300	\$1,230,000	\$2,740,300		

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ABUSE & NEGLECT	ASSISTANCE	CHILD SUPPORT	COMMUNITY SUPPORT	FOSTER CARE & ADOPTION	LICENSING	

Search for Child Day Care

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The Merit School of Manassas (#47)

8485 Plantation Lane MANASSAS, VA 20110

(703) 369-7942

Facility Type:	Child Day Center
License Type:	<u>Two Year</u>
Expiration Date:	Feb. 24, 2024
Administrator:	Tina Favara-Mullins
Business Hours:	6:30 a.m 6:00 p.m., Monday - Friday
Capacity:	341
Ages:	Birth - 12 years 11 months
Inspector:	Stephanie Reed: (540) 272-6558
Current Subsidy Provider	Yes
License/Facility ID#	1107032

Inspection Date	SHSI	Complaint Related	Violations
Feb. 18, 2022	Yes	No	No
Feb. 18, 2022	No	No	No
Aug. 17, 2021 and Aug. 18, 2021	Yes	No	No
Aug. 17, 2021 and Aug. 18, 2021	No	No	No
Jan. 28, 2021	Yes	No	No
Jan. 28, 2021	No	No	No
July 28, 2020 and July 29, 2020	Yes	No	No
July 28, 2020 and July 29, 2020	No	No	No
Jan. 30, 2020	Yes	No	No
Jan. 30, 2020	No	No	No
Dec. 2, 2019	No	Yes	No
Sept. 13, 2019	Yes	No	No
Sept. 13, 2019	No	No	No
June 3, 2019	Yes	No	No
June 3, 2019	No	No	No
Feb. 8, 2019	No	Yes	No
Aug. 16, 2018	Yes	No	No
Aug. 16, 2018	No	No	No
Feb. 21, 2018	No	No	Yes
Feb. 21, 2018	Yes	No	No
Aug. 29, 2017	Yes	No	No
Aug. 29, 2017	No	No	Yes



Disclaimen

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8757 SIGNAL HILL RD

Location	8757 SIGNAL HILL RD	ΤΑΧ ΜΑΡ	100/01 00/ 196A/ /
Acct#	32619	Owner	C AND J LEOPOLD FAMILY PARTNERSHIP LP
Assessment	\$1,683,500	Building Count	3
Model		Legal Description	LENGTHY LEGAL
Legal Description	SEE DEED	Legal Description	

Current Value

Assessment				
Valuation Year	Improvements	Land	Total	
2022	\$934,000	\$749,500	\$1,683,500	

Owner of Record

Owner	C AND J LEOPOLD FAMILY PARTNERSHIP LP	Sale Price	\$0
Co-Owner		Book & Page	201212270124263/
Address	4300 PRINCE WILLIAM PKWY	Sale Date	12/27/2012
	WOODBRIDGE, VA 22192-5361		

Ownership History

Ownership History			
Owner	Sale Price	Book & Page	Sale Date
8757 SIGNAL HILL LLC	\$0	2785/941	08/17/1999
LEOPOLD CHARLES W AND JACQUELINE M	\$0	2247/638	06/16/1995

Building Information

Year Built:	1900
Living Area:	3,310
Building Percent Good:	49

Building Attributes		
Field Description		
Style:	Inst Classrm	
Model	Commercial	

Grade	Good +15
Stories:	2
Occupancy	
Exterior Wall 1	Aluminum Sidng
Exterior Wall 2	
Roof Structure	Gable
Roof Cover	Slate
Interior Wall 1	Sheet Rock
Interior Wall 2	
Interior Floor 1	Carpet
Interior Floor 2	Vinyl
Heating Fuel	Gas
Heating Type	FA/HW/ST
АС Туре	Unit/AC
Struct Class	
Bldg Use	COM SCH
Total Rooms	
Total Bedrms	00
Total Baths	0
I and E Filed?	OWNER OCCUPIED
Location	SOUTH
1st Floor Use:	260
Heat/AC	HEAT/AC SPLIT
Frame Type	WOOD FRAME
Baths/Plumbing	AVERAGE
Ceiling/Wall	CEIL & WALLS
Rooms/Prtns	AVERAGE
Wall Height	10.00
% Comn Wall	



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Building Layout



Year Built: Living Area: Building Percent Good:	1950 1,232 61	utos - Bida 2 of 3
Field	y Attrib	Description
Style:		Inst Classrm
Model		Commercial
Grade		Average
Stories:		2
Occupancy		
Exterior Wall 1		Aluminum Sidng

Exterior Wall 2	
Roof Structure	Нір
Roof Cover	Metal/Tin
Interior Wall 1	Sheet Rock
Interior Wall 2	
Interior Floor 1	Carpet
Interior Floor 2	Vinyl
Heating Fuel	Gas
Heating Type	FA/HW/ST
АС Туре	Unit/AC
Struct Class	
Bldg Use	COM SCH
Total Rooms	
Total Bedrms	00
Total Baths	0
I and E Filed?	
Location	
1st Floor Use:	260
Heat/AC	HEAT/AC SPLIT
Frame Type	WOOD FRAME
Baths/Plumbing	AVERAGE
Ceiling/Wall	CEIL & WALLS
Rooms/Prtns	AVERAGE
Wall Height	10.00
% Comn Wall	

Building 2 : Section 2

Year Built:	1950	
Living Area:	1,958	
Building Percent Good:	61	
Building Attribut	es : Bl	dg 2 of 3 : Section 2 of 2
Field		Description
Style:		Inst Classrm
Model		Commercial
Grade		Average
Stories:		1
Occupancy		
Exterior Wall 1		Aluminum Sidng
Exterior Wall 2		
Roof Structure		Gable
Roof Cover		Asphalt
Interior Wall 1		Sheet Rock

Building Photo



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Building Layout



Building Photo



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Interior Wall 2	
Interior Floor 1	Carpet
Interior Floor 2	Vinyl
Heating Fuel	Gas
Heating Type	FA/HW/ST
АС Туре	Unit/AC
Struct Class	
Bldg Use	COM SCH
Total Rooms	
Total Bedrms	00
Total Baths	0
I and E Filed?	
Location	
1st Floor Use:	260
Heat/AC	HEAT/AC SPLIT
Frame Type	MASONRY
Baths/Plumbing	AVERAGE
Ceiling/Wall	CEIL & WALLS
Rooms/Prtns	AVERAGE
Wall Height	10.00
% Comn Wall	

Building Layout



Building 3 : Section 1

Year Built:	1950
Living Area:	816
Building Percent Good:	61

Building Attributes : Bldg 3 of 3		
Field	Description	
Style:	Inst Classrm	
Model	Commercial	
Grade	Average	
Stories:	1	
Occupancy		
Exterior Wall 1	Concr/Cinder	
Exterior Wall 2		
Roof Structure	Gable	
Roof Cover	Asphalt	
Interior Wall 1	Sheet Rock	
Interior Wall 2		
Interior Floor 1	Carpet	
Interior Floor 2	Vinyl	
Heating Fuel	Gas	

Building Photo



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Heating Type	FA/HW/ST
АС Туре	Unit/AC
Struct Class	
Bldg Use	COM SCH
Total Rooms	
Total Bedrms	00
Total Baths	0
I and E Filed?	
Location	
1st Floor Use:	260
Heat/AC	HEAT/AC SPLIT
Frame Type	MASONRY
Baths/Plumbing	AVERAGE
Ceiling/Wall	CEIL & WALLS
Rooms/Prtns	AVERAGE
Wall Height	10.00
% Comn Wall	



Building 3 : Section 2

Year Built:	1995
Living Area:	1,530
Building Percent Good:	73

Building Attributes : Bldg 3 of 3 : Section 2 of 2			
Field	Description		
Style:	Inst Classrm		
Model	Commercial		
Grade	Average		
Stories:	1		
Occupancy			
Exterior Wall 1	Concr/Cinder		
Exterior Wall 2			
Roof Structure	Flat		
Roof Cover	T&G/Rubber		
Interior Wall 1	Sheet Rock		
Interior Wall 2			
Interior Floor 1	Carpet		
Interior Floor 2	Vinyl		
Heating Fuel	Gas		
Heating Type	FA/HW/ST		
АС Туре	Unit/AC		
Struct Class			
Bldg Use	COM SCH		

Building Photo



(https://images.vgsi.com/photos/ManassasVAPhotos//default.jpg)

Total Rooms	
Total Bedrms	00
Total Baths	0
I and E Filed?	
Location	
1st Floor Use:	260
Heat/AC	HEAT/AC SPLIT
Frame Type	MASONRY
Baths/Plumbing	AVERAGE
Ceiling/Wall	CEIL & WALLS
Rooms/Prtns	AVERAGE
Wall Height	10.00
% Comn Wall	



Extra Features

Extra Features	Legend
No Data for Extra Features	

Land

Land Use		Land Line Valuation
Use Code	260	Size (Sqr Feet) 42744
Description	COM SCH	Frontage
Zone	B4	Depth
Neighborhood	Wellingtn/Rt28	Assessed Value \$749,500
Alt Land Appr	No	
Category		

Outbuildings

Outbuildings			<u>Legend</u>	
Code	Description	Sub Code	Sub Description	Size
FN5	FENCE-10'			192.00 L.F.
TEN1	TENNIS COURT			1.00 UNITS
LT1	LIGHTS-IN W/PL			4.00 UNITS
PARK	PARKING SPACE			29.00 UNITS
BIN1	BINS			90.00 S.F.
SHD1	SHED FRAME			140.00 S.F.
PLAY	PLAYGROUND			1.00
FN	FENCE			90.00 L.F.
FN1	FENCE-4'		462.00 L.F.	
------	-------------	--	--------------	
PAV2	PAVING-CONC		875.00 S.F.	
OTHR	OTHER		900.00 UNITS	

Valuation History

Assessment				
Valuation Year	Improvements	Land	Total	
2021	\$934,000	\$749,500	\$1,683,500	
2020	\$983,200	\$749,500	\$1,732,700	
2019	\$971,400	\$749,500	\$1,720,900	



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The Merit School of Old Town Manassas	Facility Type:	Child Day Center
8757 Signal Hill Drive	License Type:	Two Year
MANASSAS, VA 20110-5835	Expiration Date:	July 12, 2023
(703) 361-5113	Administrator:	Ms. Carey Michalowski
Interview Service 2017	Business Hours:	6:30am - 6:00pm, Monday - Friday
	Capacity:	142
	Ages:	Birth - 12 years 11 months
	Inspector:	Morgan Bryson: (540) 270-0057
	Current Subsidy Provider	Yes
	License/Facility ID#	1058049

	Violations	Complaint Related	<u>SHSI</u>	Inspection Date
	No	No	Yes	May 19, 2022
	Yes	No	No	May 19, 2022
	No	No	Yes	Nov. 10, 2021
	No	No	No	Nov. 10, 2021
	No	No	Yes	May 20, 2021
	No	No	No	May 20, 2021
Thank you for visit	No	Yes	No	Jan. 27, 2021 and Jan. 28, 2021
	No	No	Yes	Nov. 24, 2020
	No	No	No	Nov. 24, 2020
	No	No	Yes	Jan. 3, 2020
	Yes	No	No	Jan. 3, 2020
	Yes	No	Yes	July 1, 2019
	Yes	No	No	July 1, 2019
	No	Yes	No	April 26, 2019
	No	No	Yes	Jan. 9, 2019
	No	No	No	Jan. 9, 2019
	No	No	Yes	July 18, 2018
	No	No	No	July 18, 2018
	No	No	Yes	Jan. 19, 2018
	No	No	No	Jan. 19, 2018
	Yes	No	No	July 5, 2017
	Yes	No	Yes	July 5, 2017

Disclaimen

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8461 SIGNAL HILL RD

Location	8461 SIGNAL HILL RD	ΤΑΧ ΜΑΡ	100/01 00/ 178A1/ /
Acct#	30993	Owner	SOO MI LLC
Assessment	\$1,656,900	Building Count	1
Model	CHILD CARE	Legal Description	PARCEL A
Legal Description	COMMERCIAL	Legal Description	

Current Value

Assessment				
Valuation Year	Improvements	Land	Total	
2022	\$489,900	\$1,167,000	\$1,656,900	

Owner of Record

Owner	SOO MI LLC	Sale Price	\$1,765,000
Co-Owner		Book & Page	201105090038738/
Address	8534 BAUER CIR	Sale Date	05/09/2011
	SPRINGFIELD, VA 22152-2330		

Ownership History

Ownership History				
Owner	Sale Price	Book & Page	Sale Date	
CC ILVA LP	\$0	200209110117268/	09/11/2002	
CTC VA QBS 11-32 INC	\$238,750	2189/514	10/03/1994	
MCMAHON JOHN F JR TRUSTEE	\$0	2183/1857	09/14/1994	

Building Information

Building Percent Good:	62	
Living Area:	8,880	
Year Built:	1994	

Building Attributes		
Field	Description	
Style:	Day Care Ctr	

Model	Commercial
Grade	Average
Stories:	1
Occupancy	
Exterior Wall 1	Brick
Exterior Wall 2	Concr/Cinder
Roof Structure	Gable
Roof Cover	Asphalt
Interior Wall 1	Sheet Rock
Interior Wall 2	
Interior Floor 1	Carpet
Interior Floor 2	Vinyl
Heating Fuel	Gas
Heating Type	FA/HW/ST
АС Туре	Central
Struct Class	
Bldg Use	DAY CARE MDL-94
Total Rooms	
Total Bedrms	00
Total Baths	0
I and E Filed?	I AND E FILED
Location	SOUTH
1st Floor Use:	210
Heat/AC	HEAT/AC PKGS
Frame Type	WOOD FRAME
Baths/Plumbing	AVERAGE
Ceiling/Wall	CEIL & WALLS
Rooms/Prtns	AVERAGE
Wall Height	10.00
% Comn Wall	



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Building Layout



Extra Features

		Extra Features	Legend
		No Data for Extra Features	
Land			
Land Use		Land Line Valuation	
Use Code	210	Size (Sqr Feet) 67427	
Description	DAY CARE MDL-94	Frontage	

ZoneB1NeighborhoodWellingtn/Rt28Alt Land ApprNoCategoryCategory

Outbuildings

	Legend			
Code	Description	Sub Code	Sub Description	Size
PLAY	PLAYGROUND			1.00
FN11	FENCE			390.00 L.F.
PARK	PARKING SPACE			44.00 UNITS
PRKH	HANDICAP SPACE			2.00 UNITS

Valuation History

Assessment					
Valuation Year Improvements Land Total					
2021	\$489,900	\$1,167,000	\$1,656,900		
2020	\$668,100	\$1,167,000	\$1,835,100		
2019	\$619,200	\$1,167,000	\$1,786,200		



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Childtime Learning Center #1052

8461 Signal Hill Road MANASSAS, VA 20110-7029

(703) 330-4590

Facility Type:	Child Day Center
License Type:	<u>Two Year</u>
Expiration Date:	March 5, 2023
Administrator:	Ms. Samantha Young
Business Hours:	6:00 a.m 6:00 p.m., Monday - Friday
Capacity:	214
Ages:	1 month - 12 years 11 months
Inspector:	Sharon Allen: (540) 272-2941
Current Subsidy Provider	Yes
License/Facility ID#	888728

Violations	Complaint Related	SHSI	Inspection Date
Yes	Yes	No	April 27, 2022
No	No	Yes	April 27, 2022
Yes	No	No	April 27, 2022
No	No	Yes	Dec. 13, 2021
No	No	No	Dec. 13, 2021
No	No	Yes	March 4, 2021
Yes	No	No	March 4, 2021
No	No	Yes	Oct. 14, 2020
Yes	No	No	Oct. 14, 2020
No	No	Yes	May 22, 2020
Yes	No	No	May 22, 2020
No	No	Yes	Sept. 4, 2019
Yes	No	No	Sept. 4, 2019
No	No	Yes	Feb. 25, 2019
No	No	No	Feb. 25, 2019
Yes	No	Yes	Sept. 19, 2018
Yes	No	No	Sept. 18, 2018
No	Yes	No	July 6, 2018
Yes	No	Yes	April 19, 2018
Yes	No	No	April 19, 2018
Yes	No	No	Sept. 7, 2017
Yes	No	Yes	Sept. 7, 2017

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9840 GRANT AVE

Location	9840 GRANT AVE	ΤΑΧ ΜΑΡ	101/01 00/ 113/ /
Acct#	24594	Owner	OHANIAN STEPHEN AND SUZAN
Assessment	\$808,500	Building Count	1
Model	DAY CARE	Legal Description	COMMERCIAL
Legal Description		Legal Description	

Current Value

Assessment					
Valuation Year	Improvements	Land	Total		
2022	\$212,000	\$596,500	\$808,500		

Owner of Record

Owner	OHANIAN STEPHEN AND SUZAN	Sale Price	\$0
Co-Owner		Book & Page	202112160138805/
Address	601 SUFFIELD DR	Sale Date	12/16/2021
	GAITHERSBURG, MD 20878-2656		

Ownership History

Ownership History					
Owner Sale Price Book & Page Sale Date					
OHANIAN STEPHEN AND SUZAN	\$700,000	200303200052033/	03/20/2003		
PLESS SHARON AND MCCAULEY STEPHANIE	\$522,500	200303200052031/	03/20/2003		
MOZINGO AND WILSON	\$31,000	1310/514	04/08/1985		

Building Information

Year Built:	1985
Living Area:	4,408
Building Percent Good:	50
Bu	ilding Attributes
Field	Description
Style:	Day Care Ctr

Model	Commercial
Grade	Fair +10
Stories:	1
Occupancy	
Exterior Wall 1	Vinyl Siding
Exterior Wall 2	
Roof Structure	Gable
Roof Cover	Asphalt
Interior Wall 1	Sheet Rock
Interior Wall 2	
Interior Floor 1	Carpet
Interior Floor 2	Vinyl
Heating Fuel	Electric
Heating Type	FA/HW/ST
АС Туре	Central
Struct Class	
Bldg Use	DAY CARE MDL-94
Total Rooms	
Total Bedrms	00
Total Baths	0
I and E Filed?	I AND E FILED
Location	SOUTH
1st Floor Use:	210
Heat/AC	HEAT/AC PKGS
Frame Type	WOOD FRAME
Baths/Plumbing	AVERAGE
Ceiling/Wall	CEIL & WALLS
Rooms/Prtns	AVERAGE
Wall Height	10.00
% Comn Wall	



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Building Layout



Extra Features

	Extra Features				
No Data for Extra Features					
Land					
Land Use		Land Line Valuation			
Use Code	210	Size (Sqr Feet) 33207			
Description	DAY CARE MDL-94	Frontage			
Zone	B4	Depth			

Outbuildings

	Outbuildings				
Code	Description	Sub Code	Sub Description	Size	
FN	FENCE			1.00 L.F.	
PLAY	PLAYGROUND			1.00	
PARK	PARKING SPACE			15.00 UNITS	

Valuation History

Assessment			
Valuation Year	Improvements	Land	Total
2021	\$162,500	\$646,000	\$808,500
2020	\$196,900	\$646,000	\$842,900
2019	\$196,900	\$646,000	\$842,900



Return to Search Results | New Search |

Learning Tree Academy 9840 Grant Avenue MANASSAS, VA 20110

Facility Type:	Child Day Center
License Type:	Two Year
Expiration Date:	Aug. 8, 2023
Administrator:	Pedro Cardona
Business Hours:	6:00 am - 6:30 pm, Monday - Friday
Capacity:	100
Ages:	1 month - 12 years 11 months
Inspector:	Cathy Aylor: (540) 222-6352
Current Subsidy Provider	Yes
License/Facility ID#	1108126

Inspection Date	<u>SHSI</u>	Complaint Related	Violations
March 22, 2022	Yes	No	No
March 22, 2022	No	No	No
Aug. 17, 2021 and Aug. 20, 2021	No	No	No
Aug. 17, 2021 and Aug. 20, 2021	Yes	No	No
April 30, 2021 and May 3, 2021	Yes	No	No
April 30, 2021 and May 3, 2021	No	No	No
Sept. 17, 2020 , Sept. 22, 2020 and Sept. 23, 2020	Yes	No	No
Sept. 17, 2020 , Sept. 22, 2020 and Sept. 23, 2020	No	No	No
Feb. 6, 2020	No	No	No
Feb. 6, 2020	Yes	No	No
July 22, 2019	Yes	No	No
July 22, 2019	No	No	Yes
May 3, 2019	Yes	No	No
May 3, 2019	No	No	No
Nov. 29, 2018	Yes	No	No
Nov. 29, 2018	No	No	No
April 19, 2018	Yes	No	Yes
April 19, 2018	No	No	Yes
Nov. 6, 2017	No	Yes	Yes
July 24, 2017	Yes	No	Yes
July 24, 2017	No	No	Yes



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9208 CENTREVILLE RD

Location	9208 CENTREVILLE RD	ΤΑΧ ΜΑΡ	100/05 5/ 6/ /
Acct#	22105	Owner	THE ARC OF GREATER PRINCE WILLIAM INSIGHT INC
Assessment	\$624,600	Building Count	1
Model	CUSTOM	Legal Description	LOT 6 BLK 5
Legal Description	ANNABURG	Legal Description	COMMERCIAL

Current Value

Assessment			
Valuation Year Improvements		Land	Total
2022	\$136,100	\$488,500	\$624,600

Owner of Record

Owner	THE ARC OF GREATER PRINCE WILLIAM INSIGHT INC	Sale Price	\$600,000
Co-Owner		Book & Page	200510310188390/
Address		Sale Date	10/31/2005
	WOODBRIDGE, VA 22193-4716		

Ownership History

Ownership History			
Owner	Sale Price	Book & Page	Sale Date
HORN PAUL L AND PRISCILLA L	\$185,000	2432/1558	04/08/1997
BOYER EARL B AND HAZEL B	\$20,250	417/19	01/30/1967

Building Information

Year Built:	1955	
Living Area:	1,692	
Building Percent Good:	37	
Building Attributes		
Field	Description	
Style:	Day Care Ctr	

Model	Commercial
Grade	Fair +10
Stories:	1
Occupancy	
Exterior Wall 1	Pre-cast Concr
Exterior Wall 2	
Roof Structure	Gable
Roof Cover	Asphalt
Interior Wall 1	Sheet Rock
Interior Wall 2	
Interior Floor 1	Carpet
Interior Floor 2	Vinyl
Heating Fuel	Gas
Heating Type	FA/HW/ST
АС Туре	Central
Struct Class	
Bldg Use	DAY CARE MDL-94
Total Rooms	
Total Bedrms	03
Total Baths	1.5
I and E Filed?	OWNER OCCUPIED
Location	NORTH
1st Floor Use:	210
Heat/AC	HEAT/AC PKGS
Frame Type	WOOD FRAME
Baths/Plumbing	LIGHT
Ceiling/Wall	CEIL & WALLS
Rooms/Prtns	AVERAGE
Wall Height	8.00
% Comn Wall	



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Building Layout



Year Built:	1995
Living Area:	808
Building Percent Good:	62

Building Attributes : Section 2 of 2		
Field	Description	
Style:	Day Care Ctr	
Model	Commercial	
Grade	Fair +10	
Stories:	1	
Occupancy		

Exterior Wall 1	Vinyl Siding
Exterior Wall 2	
Roof Structure	Gable
Roof Cover	Asphalt
Interior Wall 1	Sheet Rock
Interior Wall 2	
Interior Floor 1	Carpet
Interior Floor 2	Vinyl
Heating Fuel	Gas
Heating Type	FA/HW/ST
АС Туре	Unit/AC
Struct Class	
Bldg Use	DAY CARE MDL-94
Total Rooms	
Total Bedrms	03
Total Baths	0
I and E Filed?	OWNER OCCUPIED
Location	NORTH
1st Floor Use:	210
Heat/AC	HEAT/AC PKGS
Frame Type	WOOD FRAME
Baths/Plumbing	NONE
Ceiling/Wall	CEIL & WALLS
Rooms/Prtns	AVERAGE
Wall Height	8.00
% Comn Wall	



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Building Layout



Extra Features

Extra Features		
Code	Description	Size
ALM	SECURITY ALARM	2416.00 S.F.

Land

Land Use		Land Line Valua	ation
Use Code	210	Size (Sqr Feet)	17500
Description	DAY CARE MDL-94	Frontage	
Zone	B1	Depth	
Neighborhood	O T 3RD	Assessed Value	\$488,500
Alt Land Appr	No		
Category			

Outbuildings

Outbuildings			Legend	
Code	Description	Sub Code	Sub Description	Size
FN	FENCE			450.00 L.F.
PLAY	PLAYGROUND			1.00
PARK	PARKING SPACE			8.00 UNITS

Valuation History

Assessment				
Valuation Year	Improvements	Land	Total	
2021	\$136,100	\$488,500	\$624,600	
2020	\$137,400	\$488,500	\$625,900	
2019	\$137,400	\$488,500	\$625,900	



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Robert Day Child Care Center 9208 Centreville Road MANASSAS, VA 20110-5131

(703) 361-2443 💓 Directions

Facility Type:	Child Day Center
License Type:	<u>Two Year</u>
Expiration Date:	April 11, 2023
Qualification:	Special Needs Program
Administrator:	Ms. Donna Shipman
Business Hours:	6:00 a.m 6:30 p.m., Monday - Friday
Capacity:	25
Ages:	2 years 6 months - 12 years 11 months
Inspector:	Stephanie Reed: (540) 272-6558
Current Subsidy Provider	No
License/Facility ID#	1033381

	Violations	Complaint Related	<u>SHSI</u>	Inspection Date	
	No	No	Yes	March 1, 2022	
	No	No	No	March 1, 2022	
X	No	No	No	April 8, 2021	
	No	No	No	Aug. 21, 2020	
	No	No	Yes	Nov. 4, 2019	
Thank you for visiting.	No	No	No	Nov. 4, 2019	
now was your experience.	No	No	Yes	April 1, 2019	
	Yes	No	No	April 1, 2019	
	Yes	No	Yes	Oct. 10, 2018	
_	Yes	No	No	Oct. 10, 2018	
G	Yes	No	Yes	May 7, 2018	
	No	No	No	May 7, 2018	
	No	No	No	Oct. 6, 2017	
	Yes	No	Yes	Oct. 6, 2017	

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9151 CENTREVILLE RD

Location	9151 CENTREVILLE RD	ΤΑΧ ΜΑΡ	100/01 00/ C2A/ /
Acct#	34514	Owner	DNA RE HOLDINGS LLC
Assessment	\$1,102,500	Building Count	1
Model	DAY CARE CENTER	Legal Description	LENGTHY LEGAL
Legal Description	SEE DEED	Legal Description	

Current Value

Assessment			
Valuation Year	Improvements	Land	Total
2022	\$379,000	\$723,500	\$1,102,500

Owner of Record

Owner	DNA RE HOLDINGS LLC	Sale Price	\$1,200,000
Co-Owner		Book & Page	201911140084589/
Address	46624 HAMPSHIRE STATION DR	Sale Date	11/14/2019
	STERLING, VA 20165-7394		

Ownership History

Ownership History			
Owner	Sale Price	Book & Page	Sale Date
ADJ LLC	\$1,200,000	201511130094579/	11/13/2015
QMAS GROUP LLC	\$1,300,000	200708140093405/	08/14/2007
CRADLE TO CRAYONS SERVICES LLC	\$157,500	2593/1414	06/29/1998
OAK STREET DEVELOPMENT CORP	\$71,695	2593/1412	06/29/1998

Building Information

Year Built:	1999	
Living Area:	6,000	
Building Percent Good:	67	
Building Attributes		
Field	Description	

Style:	Day Care Ctr
Model	Commercial
Grade	Average
Stories:	1
Occupancy	
Exterior Wall 1	Vinyl Siding
Exterior Wall 2	
Roof Structure	Gable
Roof Cover	Asphalt
Interior Wall 1	Sheet Rock
Interior Wall 2	
Interior Floor 1	Carpet
Interior Floor 2	Vinyl
Heating Fuel	Gas
Heating Type	FA/HW/ST
АС Туре	Central
Struct Class	
Bldg Use	DAY CARE MDL-94
Total Rooms	
Total Bedrms	00
Total Baths	0
I and E Filed?	OWNER OCCUPIED
Location	NORTH
1st Floor Use:	210
Heat/AC	HEAT/AC PKGS
Frame Type	WOOD FRAME
Baths/Plumbing	TYPICAL
Ceiling/Wall	CEIL & WALLS
Rooms/Prtns	AVERAGE
Wall Height	10.00
% Comn Wall	



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Building Layout



Extra Features

	Extra Features	<u>Legend</u>
	No Data for Extra Features	
Land]

Land Use

Land Line Valuation

 Description
 DAY CARE MDL-94

 Zone
 B1

 Neighborhood
 OFF RT 28

 Alt Land Appr
 No

 Category
 Vertication

Outbuildings

Outbuildings				Legend
Code	Description	Sub Code	Sub Description	Size
PLAY	PLAYGROUND			1.00
FN11	FENCE			390.00 L.F.
РККН	HANDICAP SPACE			2.00 UNITS
PARK	PARKING SPACE			18.00 UNITS

Valuation History

Assessment					
Valuation Year	Improvements	Land	Total		
2021	\$379,000	\$723,500	\$1,102,500		
2020	\$489,200	\$723,500	\$1,212,700		
2019	\$489,200	\$723,500	\$1,212,700		



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Carousel Children's Academy 9151 Centreville Road MANASSAS, VA 20110 (703) 365-7622 💓 Directions

Facility Type:	Child Day Center	
License Type:	Provisional	
Expiration Date:	Nov. 17, 2022	
Administrator:	Doreen Hecker	
Business Hours:	6:00 a.m 6:30 p.m., Monday - Friday	
Capacity:	150	
Ages:	Birth - 12 years 11 months	
Inspector:	Laura Brindle: (540) 905-2062	
Current Subsidy Provider	Yes	
License/Facility ID#	1108681	

Inspection Date	<u>SHSI</u>	Complaint Related	Violations	
June 7, 2022	No	No	No	
April 13, 2022	No	No	Yes	
April 13, 2022	Yes	No	No	
March 3, 2022	No	Yes	Yes	>
Nov. 23, 2021	Yes	No	No	•
Nov. 23, 2021	No	No	Yes	
June 7, 2021	Yes	No	Yes	Thank you for visiting.
June 7, 2021	No	No	Yes	
April 21, 2021	No	Yes	Yes	
Dec. 30, 2020	No	No	Yes	
Dec. 30, 2020	Yes	No	No	
Oct. 6, 2020 and Oct. 27, 2020	No	Yes	Yes	G
May 20, 2020	Yes	No	No	
May 20, 2020	No	No	No	
Jan. 14, 2020	Yes	No	No	
Jan. 14, 2020	No	No	Yes	
Oct. 4, 2019	No	No	Yes	

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